

# Traffic Impact Study Proposed Residential Development

Oak Park, Illinois



Prepared For:



February 1, 2021

# 1. Introduction

This report summarizes the methodologies, results, and findings of a traffic impact study conducted by Kenig, Lindgren, O'Hara, Aboona, Inc. (KLOA, Inc.) for a proposed residential development to be located at 203 South Marion Street in Oak Park, Illinois. The site, which currently contains Drechsler Brown and Williams funeral home and its parking lot, is located in the southwest corner of Marion Street with Pleasant Street. As proposed, the site will be redeveloped to provide a seven-story building containing approximately 153 apartment units, six maisonettes, an approximate 1,127 square-foot café, and a 123-space parking garage. The parking garage will be utilized by the residents only. Access to the parking garage will be provided off the public alley.

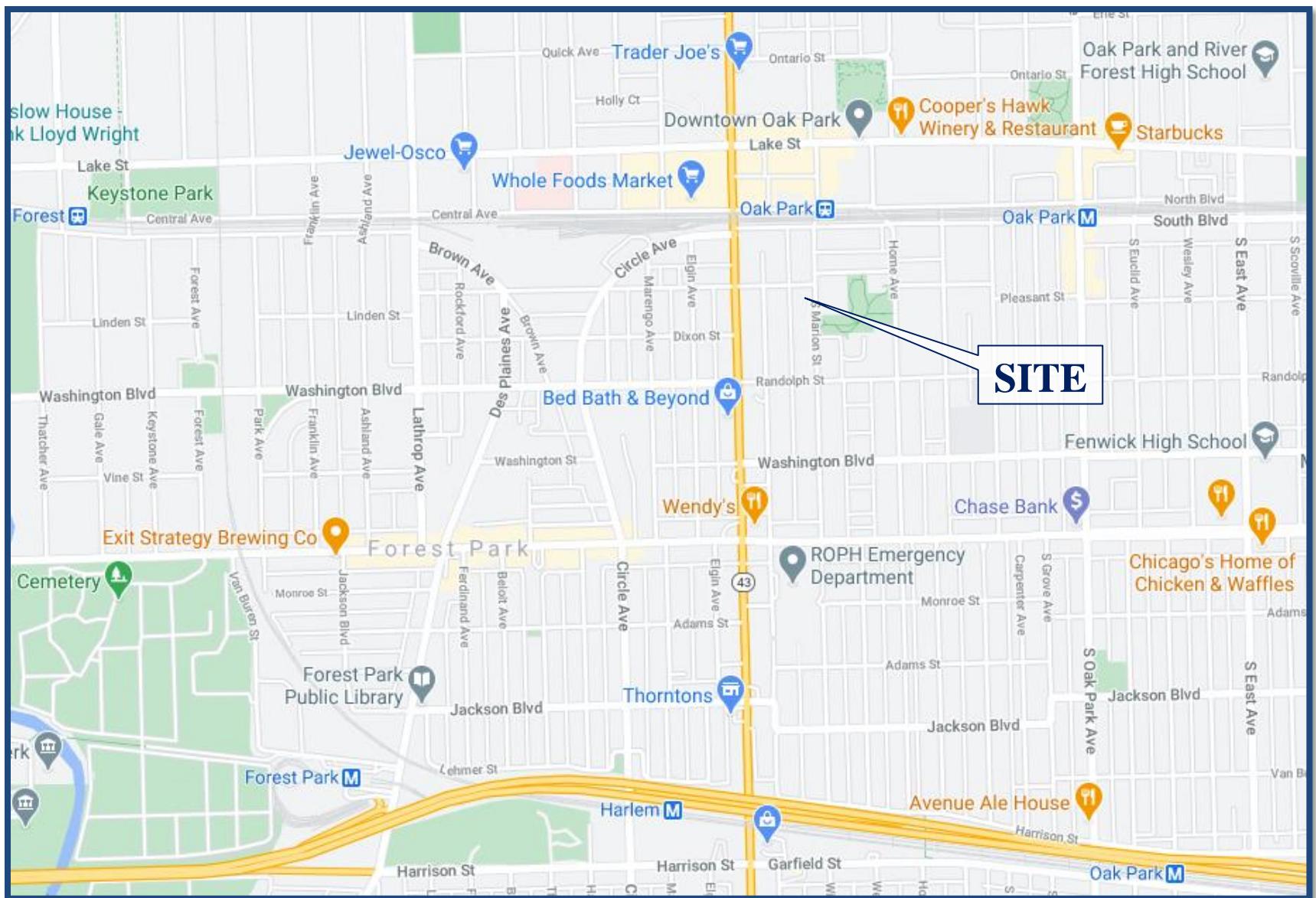
The purpose of this study was to examine background traffic conditions, assess the impact that the proposed development will have on traffic conditions in the area, and determine if any roadway or access improvements are necessary to accommodate traffic generated by the proposed development. **Figure 1** shows the location of the site in relation to the area roadway system. **Figure 2** shows an aerial view of the site.

The sections of this report present the following:

- Existing roadway conditions
- A description of the proposed development
- Directional distribution of the development traffic
- Vehicle trip generation for the development
- Future traffic conditions including access to the development
- Traffic analyses for the weekday morning, weekday evening, and Saturday midday peak hours
- Recommendations with respect to adequacy of the site access and adjacent roadway system
- Evaluation of the adequacy of the parking supply

Traffic capacity analyses were conducted for the weekday morning, weekday evening, and Saturday midday peak hours for the following conditions:

1. Existing Conditions - Analyzes the capacity of the existing roadway system using existing peak hour traffic volumes in the surrounding area.
2. Background Conditions - Analyzes the capacity of the future roadway system using the traffic volumes that include the existing traffic volumes, the ambient area growth not attributable to any particular development, and the traffic to be generated by the future parking garage located within the Rush Hospital Oak Park campus and by the full occupancy of the Eleven33 apartment building located in the southeast corner of South Boulevard with Harlem Avenue.
3. Projected Conditions – Analyzes the capacity of the future roadway system using the traffic volumes that include the existing traffic volumes, ambient area growth not attributable to any particular development, and the traffic estimated to be generated by the proposed development.



**Site Location**

**Figure 1**

*Proposed Residential Development  
Oak Park, Illinois*



Aerial View of Site

Figure 2

## 2. Existing Conditions

The following provides a description of the geographical location of the site, physical characteristics of the area roadway system including lane usage and traffic control devices, and existing peak hour traffic volumes.

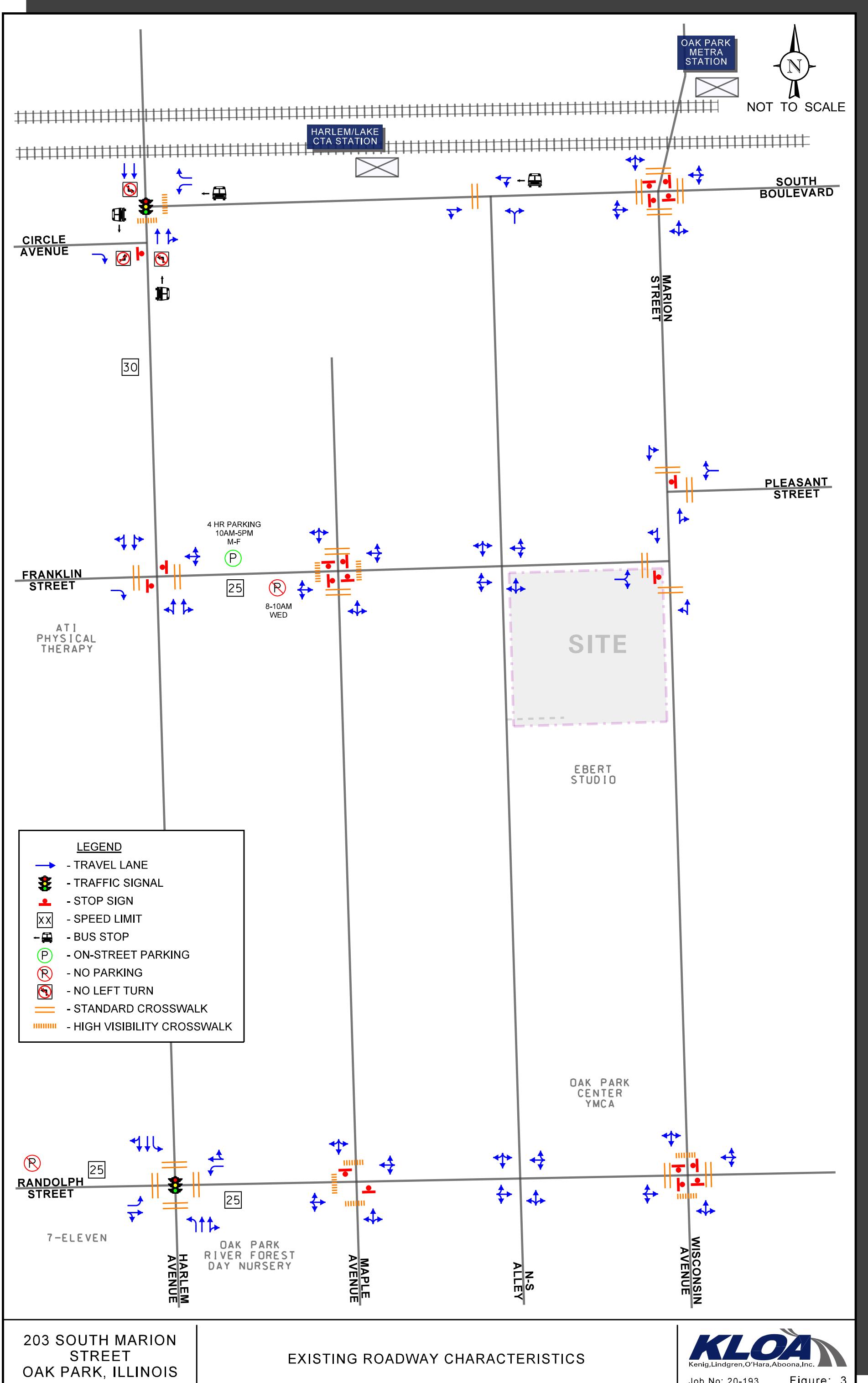
### Site Location

The site, which currently contains Drechsler Brown and Williams funeral homes and its parking lot, is bounded by Pleasant Street to the north, Marion Street to the east, a parking lot and Ebert Studio to the south, and an apartment building to the west.

### Existing Roadway System Characteristics

The characteristics of the existing roadways near the proposed development are described below and illustrated in **Figure 3**.

*Harlem Avenue (IL 43)* is a north-south other principal arterial roadway that generally provides two lanes in each direction within the vicinity of the site. At its signalized intersection with South Boulevard, Harlem Avenue provides a through lane and a combined through/right-turn lane on the northbound approach. The southbound approach provides two through lanes. A high-visibility crosswalk is provided on the south leg of this intersection. At its unsignalized intersection with Circle Avenue, Harlem Avenue provides two through lanes on the northbound approach. Northbound left turn movements onto Circle Avenue are prohibited. The southbound approach provides a through lane and a combined through/right-turn lane. At its unsignalized intersection with Pleasant Street and Franklin Street, Harlem Avenue provides a combined through/right-turn lane and a combined through/left-turn lane on both approaches. At its signalized intersection with Randolph Street, Harlem Avenue provides an exclusive left-turn lane, a through lane and a combined through/right-turn lane on both approaches. Standard style crosswalks are provided on the north and south legs of this intersection. Harlem Avenue is under the jurisdiction of the Illinois Department of Transportation (IDOT) and is classified as a Strategic Regional Arterial (SRA). In addition, Harlem Avenue carries an Annual Average Daily Traffic (AADT) volume of approximately 35,200 vehicles north of South Boulevard, 35,300 vehicles between South Boulevard and Randolph Street and 35,400 vehicles south of Randolph Street (IDOT 2019). Harlem Avenue has a posted speed limit of 30 miles per hour.



*Randolph Street* is an east-west, major collector west of Harlem Avenue and a local road east of Harlem Avenue and generally provides one lane in each direction in the vicinity of the site. At its signalized intersection with Harlem Avenue, Randolph Street provides an exclusive left-turn lane and a combined through/right-turn lane on both approaches. Standard style crosswalks are provided on the east and west legs of this intersection. At its unsignalized intersection with Maple Avenue, Randolph Street provides a combined left/through/right-turn lane on both approaches. A high-visibility crosswalk is provided on the west leg of this intersection. At its all-way stop-sign controlled intersection with Marion Street, Randolph Street provides a combined left/through/right-turn lane on both approaches. Standard style crosswalks are provided on the east and west legs of this intersection. Randolph Street is under the jurisdiction of IDOT west of Harlem Avenue and under the jurisdiction of the Village of Oak Park east of Harlem Avenue, carries an AADT volume of 8,550 vehicles (IDOT 2018), and has a posted speed limit of 25 miles per hour.

*Marion Street* is a north-south local road that generally provides one lane in each direction and continues as Wisconsin Avenue south of Randolph Street. At its all-way stop-sign controlled intersection with South Boulevard, Marion Street provides a combined left/through/right-turn lane on both approaches. Standard style crosswalks are provided on the north and south legs of this intersection. At its offset unsignalized intersection with the east leg of Pleasant Street, Marion Street provides a combined through/right-turn lane on the northbound approach and a combined through/left-turn lane on the southbound approach. At its offset unsignalized intersection with the west leg of Pleasant Street, Marion Street provides a combined through/left-turn lane on the northbound approach and a combined through/right-turn lane on the southbound approach. Standard style crosswalks are provided on the north and south legs of this intersection. At its all-way stop-sign controlled intersection with Randolph Street, Marion Street provides a combined left/through/right-turn lane on both approaches. High-visibility crosswalks are provided on the north and south legs of this intersection. Marion Street is under the jurisdiction of the Village of Oak Park and carries an AADT volume of 2,200 vehicles (IDOT 2018).

*Pleasant Street* is an east-west local road that provides one lane in each direction and continues as Franklin Street west of Harlem Avenue. At its offset unsignalized intersection with Marion Street, Pleasant Street provides a combined left/right-turn lane on both approaches under stop sign control. Standard style crosswalks are provided on the east and west legs of this intersection. At its unsignalized intersection with Harlem Avenue, Franklin Street provides an exclusive right-turn lane on the eastbound approach under stop sign control and Pleasant Street provides a combined left/through/right-turn lane on the westbound approach under stop sign control. Standard style crosswalks are provided on the east and west legs of this intersection. At its all-way stop-sign controlled intersection with Maple Avenue, Pleasant Street provides a combined left/through/right-turn lane on both approaches. High-visibility crosswalks are provided on the east and west legs of this intersection. Pleasant Street is under the jurisdiction of the Village of Oak Park and has a posted speed limit of 25 miles per hour.

*South Boulevard* is an east-west local roadway that generally provides one lane in each direction in the vicinity of the site. At its signalized intersection with Harlem Avenue, South Boulevard provides an exclusive left-turn lane and an exclusive right-turn lane on the westbound approach. A high-visibility crosswalk is provided on the east leg of this intersection. At its all-way stop-sign controlled intersection with Marion Street, South Boulevard provides a combined left/through/right-turn lane on both approaches. Standard style crosswalks are provided on the east and west legs of this intersection. South Boulevard is under the jurisdiction of the Village of Oak Park.

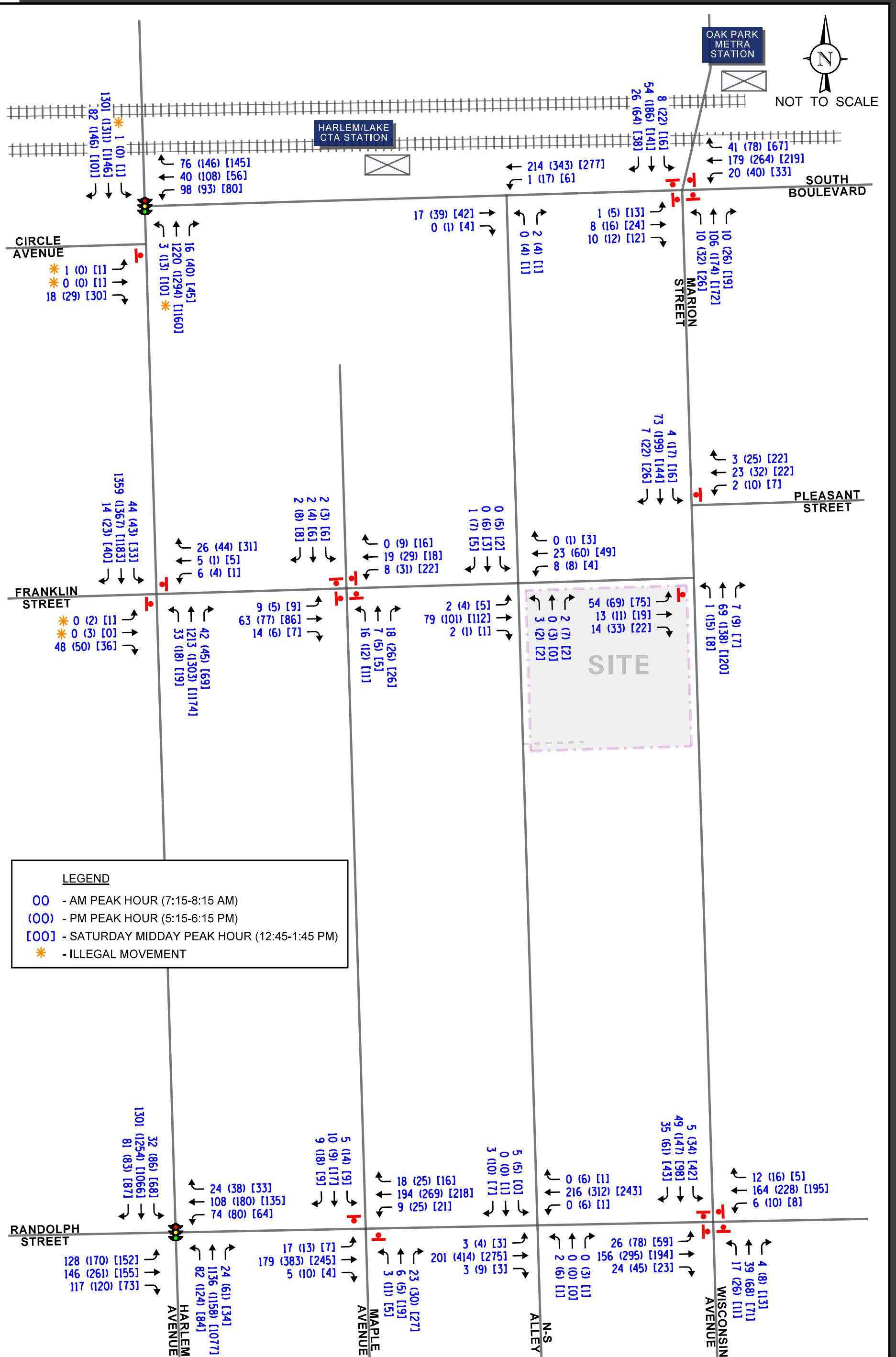
*Maple Avenue* is a north-south local roadway that generally provides one lane in each direction in the vicinity of the site. At its all-way stop-sign controlled intersection with Pleasant Street, Maple Avenue provides a combined left/through/right-turn lane on both approaches. Standard style crosswalks are provided on the north and south legs of this intersection. At its unsignalized intersection with Randolph Street, Maple Avenue provides a combined left/through/right-turn lane on both approaches under stop sign control. High-visibility crosswalks are provided on the north and south legs of this intersection. Maple Avenue is under the jurisdiction of the Village of Oak Park.

## Existing Traffic Volumes

In order to determine current traffic conditions in the vicinity of the site, KLOA, Inc. conducted peak period vehicle, pedestrian, and bicycle traffic counts using Miovision Video Scout Collection Units on Thursday, September 10, 2020 during the weekday morning (7:00 to 9:00 A.M.) and weekday evening (4:00 to 6:00 P.M.) peak periods and on Saturday, September 12, 2020 during the midday peak period (11:30 A.M. to 2:00 P.M.) at the following intersections:

- Harlem Avenue with South Boulevard and Circle Avenue
- Harlem Avenue with Pleasant Street and Franklin Street
- Harlem Avenue with Randolph Street
- Maple Avenue with Pleasant Street
- Maple Avenue with Randolph Street
- Marion Street with South Boulevard
- Marion Street with Pleasant Street
- Marion Street with Randolph Street and Wisconsin Avenue
- South Boulevard with the Public Alley
- Pleasant Street with the Public Alley
- Randolph Street with the Public Alley

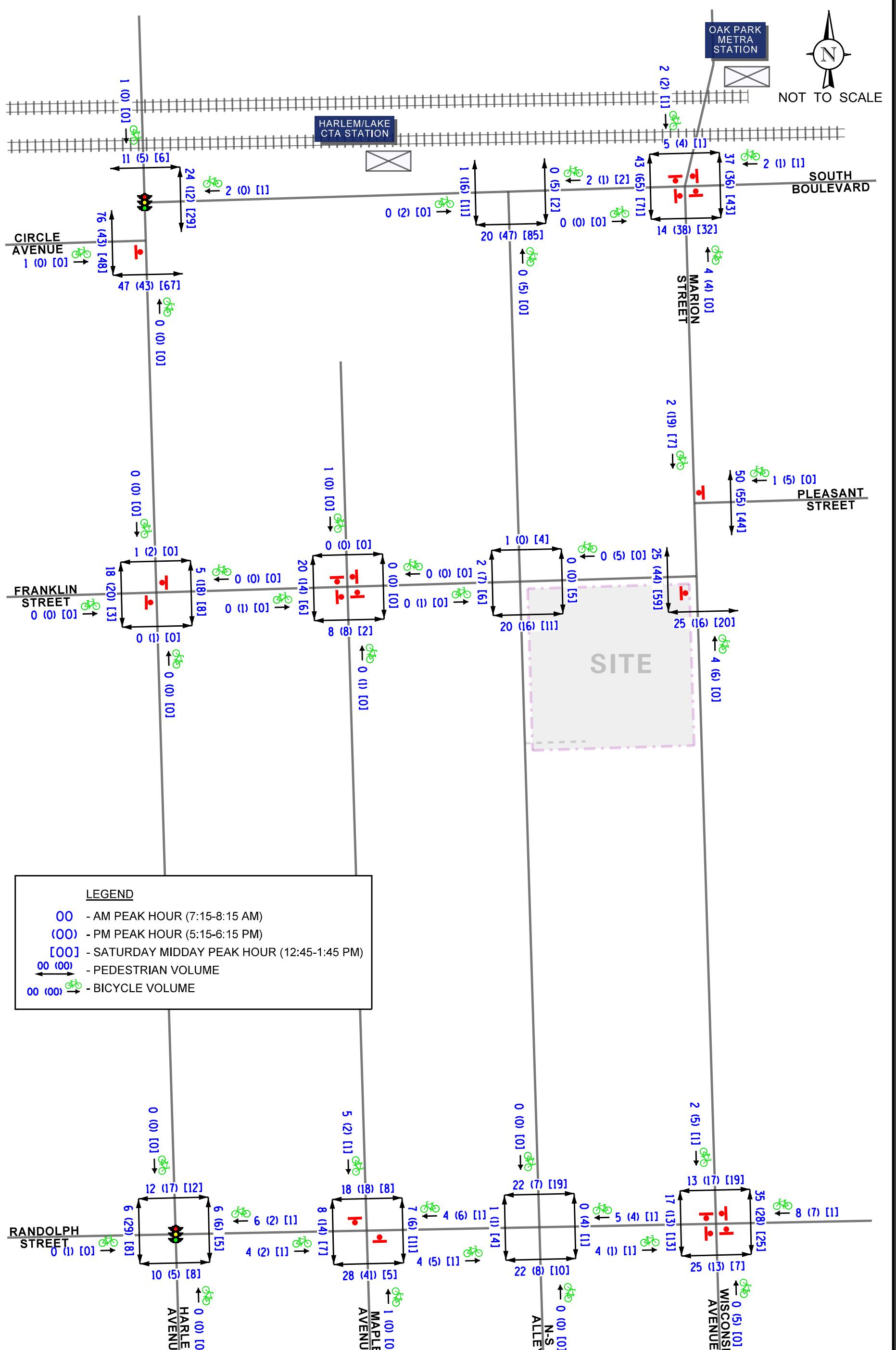
It should be noted that, due to the ongoing Covid-19 pandemic, vehicular traffic counts were adjusted based on previous counts conducted by KLOA, Inc in 2015 at the intersections of Harlem Avenue with South Boulevard and Circle Avenue, Harlem Avenue with Pleasant Street and Franklin Street, Maple Avenue with Pleasant Street, Pleasant Street with Marion Street, and Marion Street with South Boulevard and based on pre-Covid-19 data in other communities. Based on the above, traffic volumes on road segments in the area were increased between 10 and 50 percent during the peak hours (See Appendix for detailed breakdown of the applied adjustments). Based on the turning movement count data, it was determined that the weekday morning peak hour generally occurs between 7:15 and 8:15 A.M., the weekday evening peak hour generally occurs between 4:15 and 5:15 P.M., and the Saturday midday peak hour generally occurs between 12:45 and 1:45 P.M. These three respective peak hours will be used for the traffic capacity analyses presented later in this report. **Figure 4** shows the adjusted Year 2020 peak hour traffic volumes. **Figure 5** illustrates the adjusted Year 2020 pedestrian and bicycle traffic volumes, showing direction of travel. Copies of the traffic count summary sheets are located in the Appendix.



203 SOUTH MARION  
STREET  
OAK PARK ILLINOIS

## YEAR 2020 BASE TRAFFIC VOLUMES

**KLOA**  
Kenig,Lindgren,O'Hara,Aboona,Inc.  
Job No: 20-193      Figure: 4



## Crash Analysis

KLOA, Inc. obtained accident data for the most recent available past five years (2014 to 2018) for the intersections of Harlem Avenue with South Boulevard and Circle Avenue, Harlem Avenue Pleasant Street and Franklin Street, Harlem Avenue with Randolph Street, Maple Avenue with Pleasant Street, Maple Avenue with Randolph Street, Marion Street with South Boulevard, Marion Street with Pleasant Street, Marion Street with Randolph Street and Wisconsin Avenue, South Boulevard with the public alley, Pleasant Street with the public alley and Randolph Street with the public alley. **Tables 1 through 8** show a summary of the crash data for those intersections<sup>1</sup>. It should be noted that one pedestrian crash was reported at the intersection of Pleasant Street with the public alley in 2016 and two crashes were reported at the intersection of Randolph Street with the public alley in 2015. A review of the crash data revealed that no fatalities were reported at any location during the reviewed period.

Table 1  
HARLEM AVENUE WITH SOUTH BOULEVARD - CRASH SUMMARY

Year	Type of Accident Frequency							Total
	Angle	Object	Rear End	Sideswipe	Turning	Other	Total	
2014	0	0	0	0	1	0	1	
2015	0	0	0	2	2	0	4	
2016	0	0	0	1	0	1	2	
2017	1	0	2	1	1	1	6	
2018	1	0	0	1	0	0	2	
<b>Total</b>	<b>2</b>	<b>0</b>	<b>2</b>	<b>5</b>	<b>4</b>	<b>2</b>	<b>15</b>	
<b>Average/Year</b>	<b>&lt;1.0</b>	<b>0</b>	<b>&lt;1.0</b>	<b>1.0</b>	<b>&lt;1.0</b>	<b>&lt;1.0</b>	<b>3.0</b>	

<sup>1</sup> IDOT DISCLAIMER: The motor vehicle crash data referenced herein was provided by the Illinois Department of Transportation. Any conclusions drawn from analysis of the aforementioned data are the sole responsibility of the data recipient(s). Additionally, for coding years 2015 to present, the Bureau of Data Collection uses the exact latitude/longitude supplied by the investigating law enforcement agency to locate crashes. Therefore, location data may vary in previous years since data prior to 2015 was physically located by bureau personnel.

Table 2  
HARLEM AVENUE WITH PLEASANT STREET/FRANKLIN STREET - CRASH SUMMARY

Year	Type of Accident Frequency							Total
	Angle	Object	Rear End	Sideswipe	Turning	Other		
2014	2	0	0	0	0	0	0	2
2015	0	0	2	0	1	0	0	3
2016	1	0	0	0	2	0	0	3
2017	5	0	0	0	0	0	0	5
2018	4	0	2	0	3	0	0	9
<b>Total</b>	<b>12</b>	<b>0</b>	<b>4</b>	<b>0</b>	<b>6</b>	<b>0</b>	<b>0</b>	<b>22</b>
<b>Average/Year</b>	<b>2.4</b>	<b>0</b>	<b>&lt;1.0</b>	<b>0</b>	<b>2.4</b>	<b>0</b>	<b>0</b>	<b>4.4</b>

Table 3  
HARLEM AVENUE WITH RANDOLPH STREET - CRASH SUMMARY

Year	Type of Accident Frequency							Total
	Angle	Object	Rear End	Sideswipe	Turning	Other		
2014	1	0	3	0	1	0	0	5
2015	0	0	4	0	2	0	0	6
2016	0	0	5	0	2	1	0	8
2017	0	0	1	1	1	1	0	4
2018	2	0	1	1	4	0	0	8
<b>Total</b>	<b>3</b>	<b>0</b>	<b>14</b>	<b>2</b>	<b>10</b>	<b>2</b>	<b>0</b>	<b>31</b>
<b>Average/Year</b>	<b>&lt;1.0</b>	<b>0</b>	<b>2.8</b>	<b>&lt;1.0</b>	<b>2.0</b>	<b>&lt;1.0</b>	<b>0</b>	<b>6.2</b>

Table 4  
MAPLE AVENUE WITH PLEASANT STREET - CRASH SUMMARY

Year	Type of Accident Frequency						
	Angle	Object	Rear End	Sideswipe	Turning	Other	Total
2014	2	0	0	0	0	0	2
2015	0	0	0	0	0	0	0
2016	1	0	0	0	0	0	1
2017	0	0	0	0	0	0	0
2018	0	1	0	0	0	0	1
<b>Total</b>	<b>3</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>4</b>
Average/Year	<1.0	<1.0	0	0	0	0	<1.0

Table 5  
MAPLE AVENUE WITH RANDOLPH STREET - CRASH SUMMARY

Year	Type of Accident Frequency						
	Angle	Object	Rear End	Sideswipe	Turning	Other	Total
2014	1	0	0	0	0	0	1
2015	1	0	0	0	0	0	1
2016	2	0	0	0	0	0	2
2017	1	0	0	0	0	0	1
2018	3	0	0	0	0	0	3
<b>Total</b>	<b>8</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>8</b>
Average/Year	1.6	0	0	0	0	0	1.6

Table 6  
MARION STREET WITH RANDOLPH STREET - CRASH SUMMARY

Year	Type of Accident Frequency							Total
	Angle	Object	Rear End	Sideswipe	Turning	Other		
2014	0	1	0	0	0	1		2
2015	0	0	0	0	0	0		0
2016	1	0	0	0	0	0		1
2017	0	0	0	0	1	0		1
2018	0	0	1	0	1	0		2
<b>Total</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>0</b>	<b>2</b>	<b>1</b>		<b>6</b>
Average/Year	<1.0	<1.0	<1.0	0	<1.0	<1.0		1.2

Table 7  
MARION STREET WITH SOUTH BOULEVARD - CRASH SUMMARY

Year	Type of Accident Frequency							Total
	Angle	Object	Rear End	Sideswipe	Turning	Other		
2014	0	0	0	0	0	0		0
2015	0	0	0	0	0	0		0
2016	1	0	0	0	1	0		2
2017	2	0	0	0	0	0		2
2018	0	0	0	0	1	0		1
<b>Total</b>	<b>3</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>0</b>		<b>5</b>
Average/Year	<1.0	0	0	0	<1.0	0		1.0

Table 8

## MARION STREET WITH PLEASANT STREET - CRASH SUMMARY

Year	Type of Accident Frequency							Total
	Angle	Ped	Rear End	Sideswipe	Turning	Parked Vehicle		
2014	0	0	0	0	1	3	4	
2015	0	0	0	0	0	1	1	
2016	1	1 <sup>1</sup>	1	0	0	0	3	
2017	1	0	0	0	0	5	6	
2018	0	0	0	1	0	3	4	
<b>Total</b>	<b>2</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>12</b>	<b>18</b>	
<b>Average/Year</b>	<b>&lt;1.0</b>	<b>&lt;1.0</b>	<b>&lt;1.0</b>	<b>&lt;1.0</b>	<b>&lt;1.0</b>	<b>2.4</b>	<b>3.6</b>	

1 – Accident occurred at the intersection of Pleasant Street and the north-south alley west of Marion Street  
Notes:

- Twelve of the eighteen total accidents involved parked vehicles and, as such, did not occur at or within the intersection.
- , Approximately 58 percent of the parked vehicle accidents were due to driver error (i.e. motorists not properly gaging the available distance when parking)

### 3. Traffic Characteristics of Proposed Development

In order to properly evaluate future traffic conditions in the surrounding area, it was necessary to determine the traffic characteristics of the proposed development, including the directional distribution and volumes of traffic that it will generate.

#### Proposed Site and Development Plan

As proposed, the site will be redeveloped to provide a seven-story building containing approximately 153 apartment units, six maisonettes, an approximate 1,127 square-foot café, and a 123-space parking garage. The café is located at the northeast corner of the building and the six maisonette units are located on the east side of the site facing Marion Street. It should be noted that the parking garage will be utilized by the residents only. Access to the parking garage will be provided off the public alley. A copy of the preliminary site plan depicting the proposed development is included in the Appendix.

#### *Proposed Pedestrian Access*

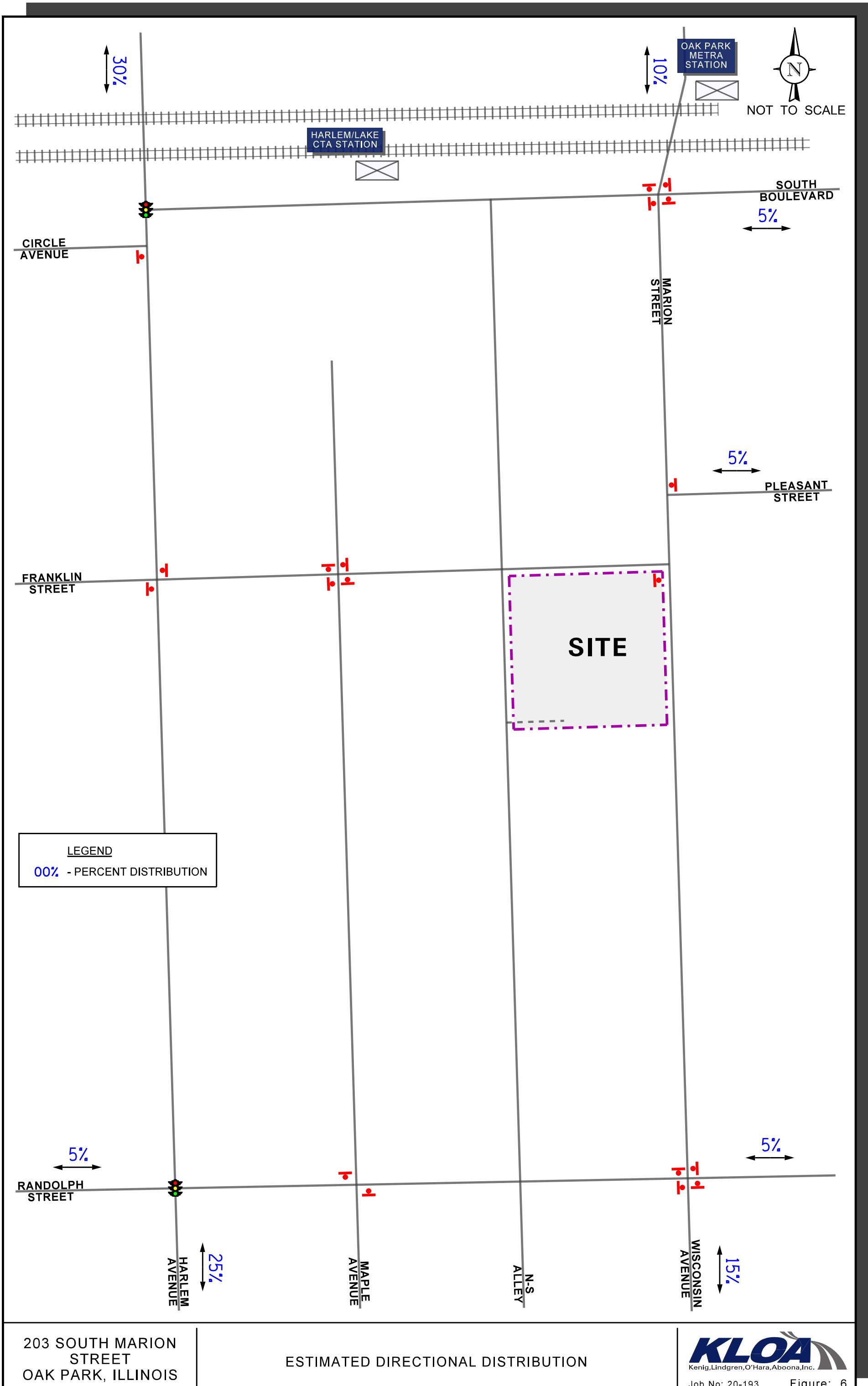
Pedestrian access to the proposed residential building will be provided via a lobby located on the north side of the building facing Pleasant Street, and a loading zone for pick-up/drop-off will be provided on the south side of Pleasant Street. Additionally, the entrance to the café will be provided off Pleasant Street.

#### *Proposed Vehicle Access*

Access to the proposed 123-space parking garage will be provided off the 18-foot wide north-south public alley located approximately 85 feet south of Pleasant Street. This access drive will provide one inbound lane and one outbound lane with outbound movements under stop sign control. Visual warning devices should be provided at both access drives. Turning movement exhibits to and from both access drives are included in the Appendix.

#### Directional Distribution

The directions from which residents and visitors of the development will approach and depart the site were estimated based on existing travel patterns, as determined from the traffic counts. **Figure 6** illustrates the directional distribution of the traffic to be generated by the proposed development.



## Development Traffic Generation

The vehicle trip generation for the overall development was calculated using data published in the Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 10<sup>th</sup> Edition. The location of the site within downtown Oak Park and its proximity to the train stations and PACE/CTA bus routes and other modes of transportation (i.e. car sharing facilities) fit the criterion of a Transit Oriented Development (TOD) that results in less dependence on automobile use. Based on a review of the census data (included in the Appendix), approximately 40 percent of the residents currently use other modes of transportation. As such, a 40 percent reduction factor was applied to the estimated vehicle traffic to be generated by the residential use. For the retail use and in order to reflect the mixed-use nature of the development, its location within downtown Oak Park and proximity to other retail destinations, the estimated trips were reduced by 20 percent. **Table 9** shows the estimated vehicle trip generation for the weekday morning, weekday evening and Saturday midday peak hours. **Table 10** shows the estimated pedestrian trip generation for the weekday morning, weekday evening and Saturday midday peak hours. It should be noted that the pedestrian trip generation shown in Table 10 also assumes that one pedestrian trip will be generated for every vehicular trip reduced in Table 9. Copies of the ITE trip generation worksheets are included in the Appendix.

Table 9  
ESTIMATED PEAK HOUR VEHICLE TRIP GENERATION

ITE Land Use Code	Type/Size	Weekday Morning Peak Hour			Weekday Evening Peak Hour			Saturday Midday Peak Hour		
		In	Out	Total	In	Out	Total	In	Out	Total
221	Apartments (153 units)	14	41	55	41	26	67	35	36	71
	<u>- 40 Percent Reduction</u>	<u>-6</u>	<u>-16</u>	<u>-22</u>	<u>-17</u>	<u>-10</u>	<u>-27</u>	<u>-14</u>	<u>-14</u>	<u>-28</u>
	Subtotal	8	25	33	24	16	40	21	22	43
220	Maisonette (6 units)	1	2	3	3	2	5	2	2	4
	<u>- 40 Percent Reduction</u>	<u>-0</u>	<u>-1</u>	<u>-1</u>	<u>-1</u>	<u>-1</u>	<u>-2</u>	<u>-1</u>	<u>-1</u>	<u>-2</u>
	Subtotal	1	1	2	2	1	3	1	1	2
936	Café (1,127 s.f.)	58	56	114	18	19	37	33	34	67
	<u>- 20 Percent Reduction</u>	<u>-12</u>	<u>-11</u>	<u>-23</u>	<u>-4</u>	<u>-4</u>	<u>-8</u>	<u>-7</u>	<u>-7</u>	<u>-14</u>
	Subtotal	46	45	91	14	15	29	26	27	53
	<b>Total</b>	<b>55</b>	<b>71</b>	<b>126</b>	<b>40</b>	<b>32</b>	<b>72</b>	<b>48</b>	<b>50</b>	<b>98</b>

Table 10  
ESTIMATED PEAK HOUR PEDESTRIAN TRIP GENERATION

ITE Land Use Code	Type/Size	Weekday Morning Peak Hour			Weekday Evening Peak Hour			Saturday Midday Peak Hour		
		In	Out	Total	In	Out	Total	In	Out	Total
221	Apartments (153 units)	18	64	82	73	37	110	70	41	111
220	Maisonette (6 units)	0	3	3	3	2	5	3	2	5
936	Café (1,127 s.f.)	17	15	32	9	8	17	12	11	23
<b>Total</b>		<b>35</b>	<b>82</b>	<b>117</b>	<b>85</b>	<b>47</b>	<b>132</b>	<b>85</b>	<b>54</b>	<b>139</b>
Note: Pedestrian trip generation also includes the assumption that one pedestrian trip will be generated per every vehicular trip reduced in Table 9										

## 4. Projected Traffic Conditions

The total projected traffic volumes include the existing traffic volumes, increase in background traffic due to growth, and the traffic estimated to be generated by the proposed subject development.

### Development Traffic Assignment

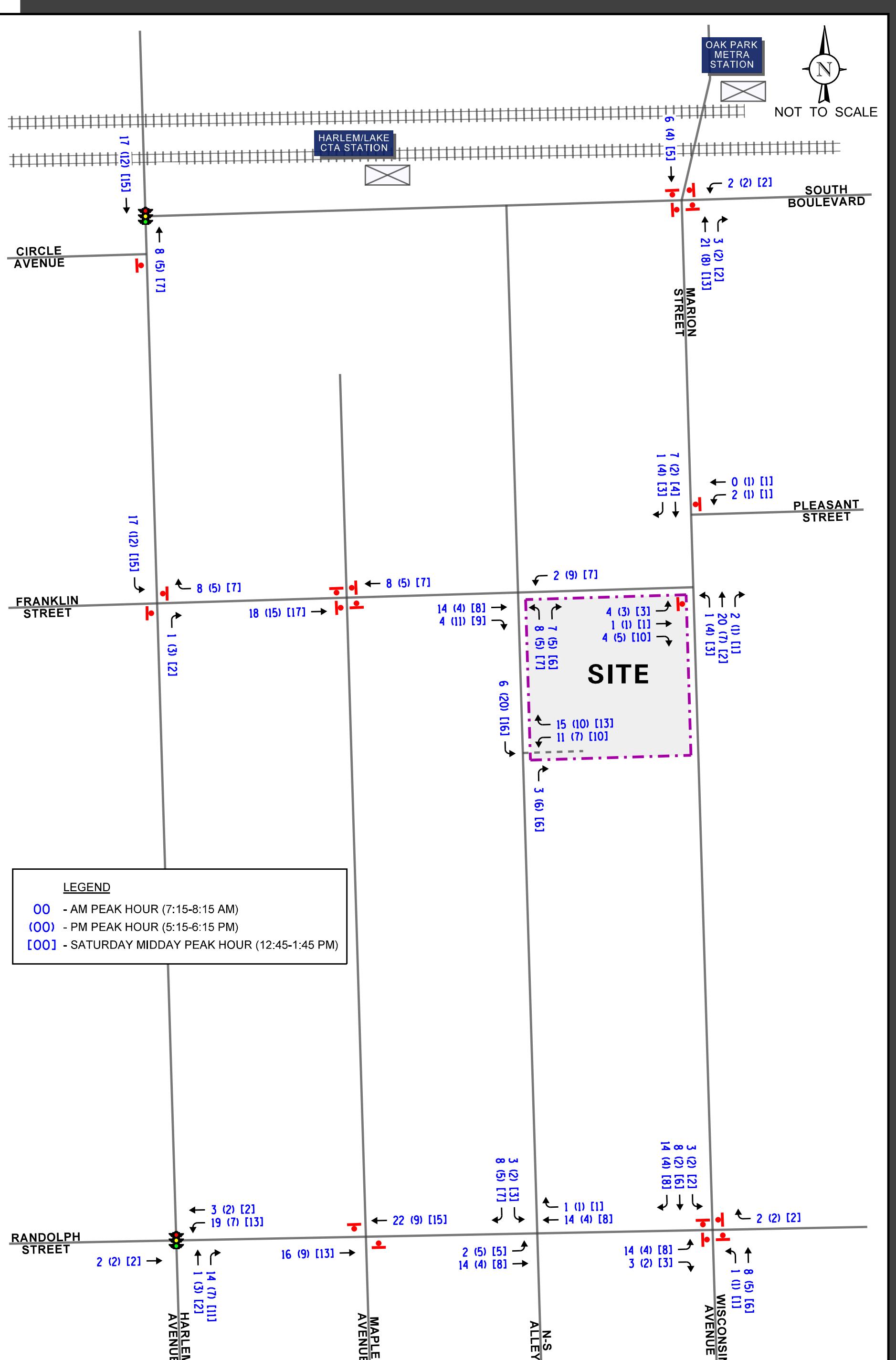
The estimated peak hour traffic volumes that will be generated by the proposed development were assigned to the roadway system in accordance with the previously described directional distribution. It should be noted that the café parking demand will be accommodated by the on-street parking in the area. **Figure 7** illustrates the assignment of the vehicle traffic volumes to be generated by the proposed development. **Figure 8** illustrates the assignment of the pedestrian traffic volumes to be generated by the proposed development.

### Background Traffic Conditions

The existing traffic volumes (Figure 4) were increased by a regional growth factor to account for the increase in existing traffic related to regional growth in the area (i.e., not attributable to any particular planned development). Based on 2050 Average Daily Traffic (ADT) projections provided by the Chicago Metropolitan Agency for Planning (CMAP) in a letter dated October 1, 2020, the existing traffic volumes were increased by an annually compounded growth rate for six years (one-year buildout plus five years) totaling 1.8 percent to represent Year 2026 total projected conditions. Additionally, the Year 2026 background traffic conditions include the traffic to be generated by the future parking garage located within the Rush Hospital Oak Park campus, and by the full occupancy of the Eleven33 apartment building located at the southeast corner of South Boulevard with Harlem Avenue, which based on our understanding it is currently 75 percent occupied. **Figure 9** shows the Year 2026 background traffic conditions. A copy of the CMAP 2050 projections letter is included in the Appendix.

### Total Projected Traffic Volumes

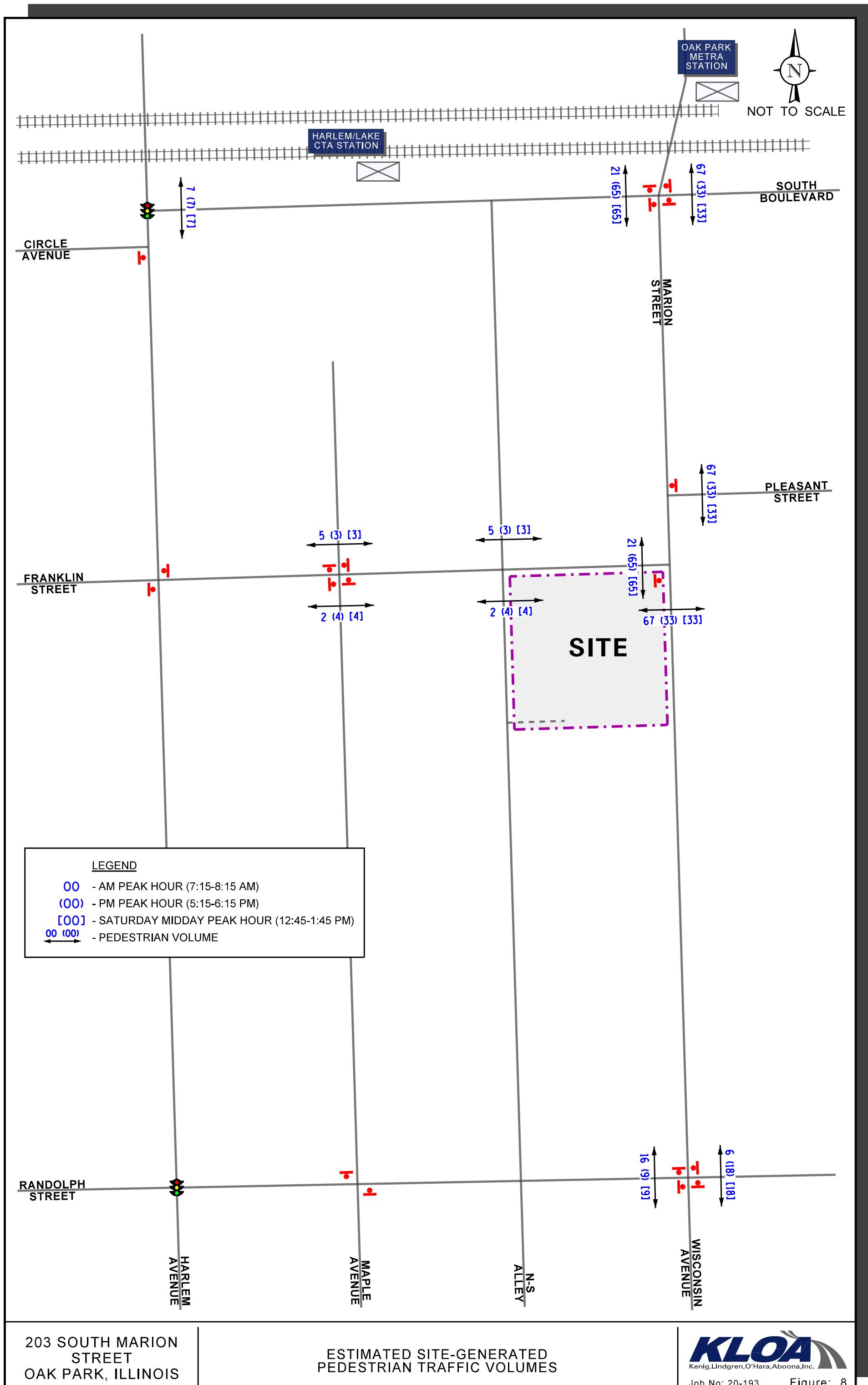
The total projected traffic volumes include the Year 2026 background traffic volumes (Figure 9) and the traffic estimated to be generated by the proposed development (Figure 7). **Figure 10** shows the Year 2026 total projected traffic volumes.

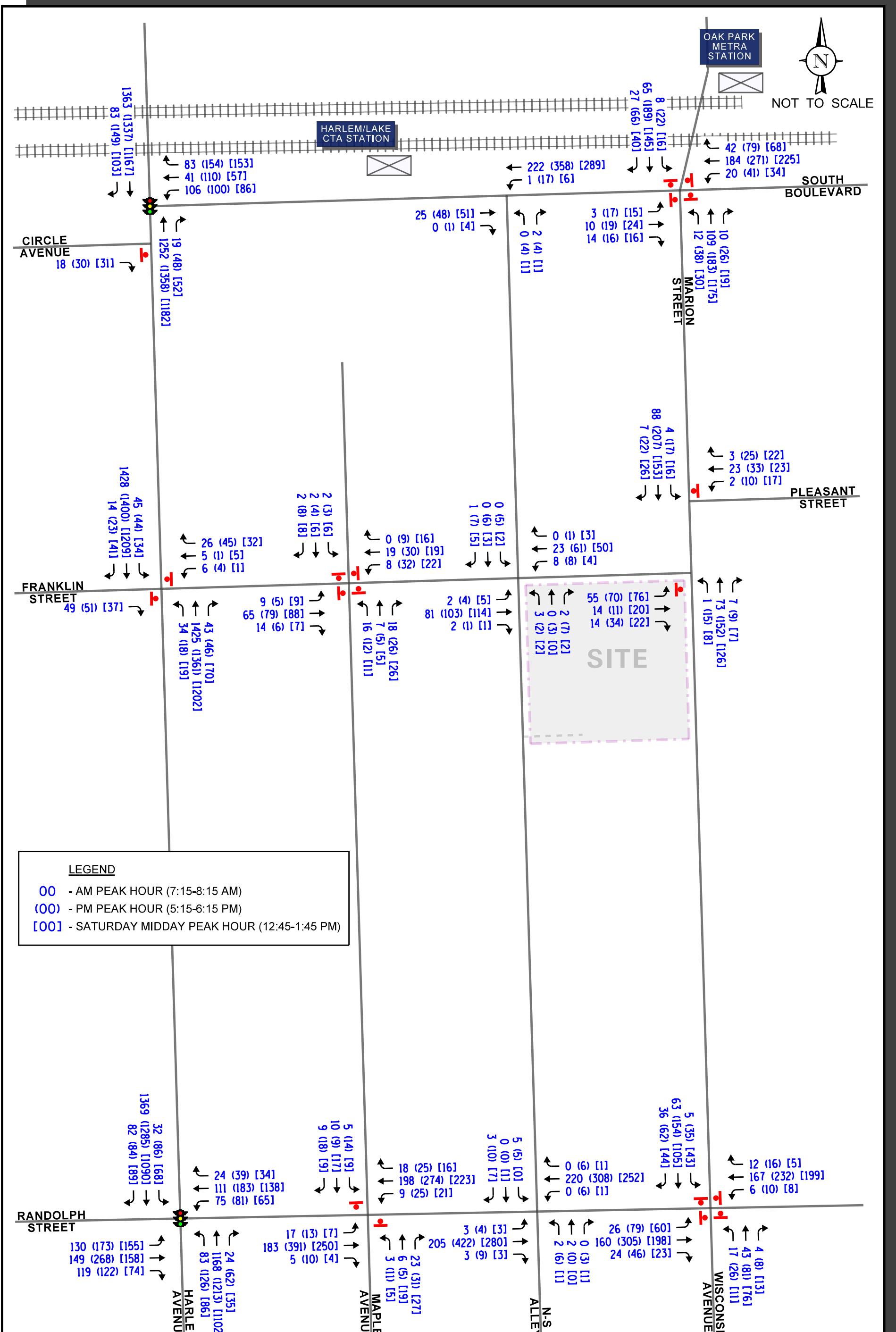


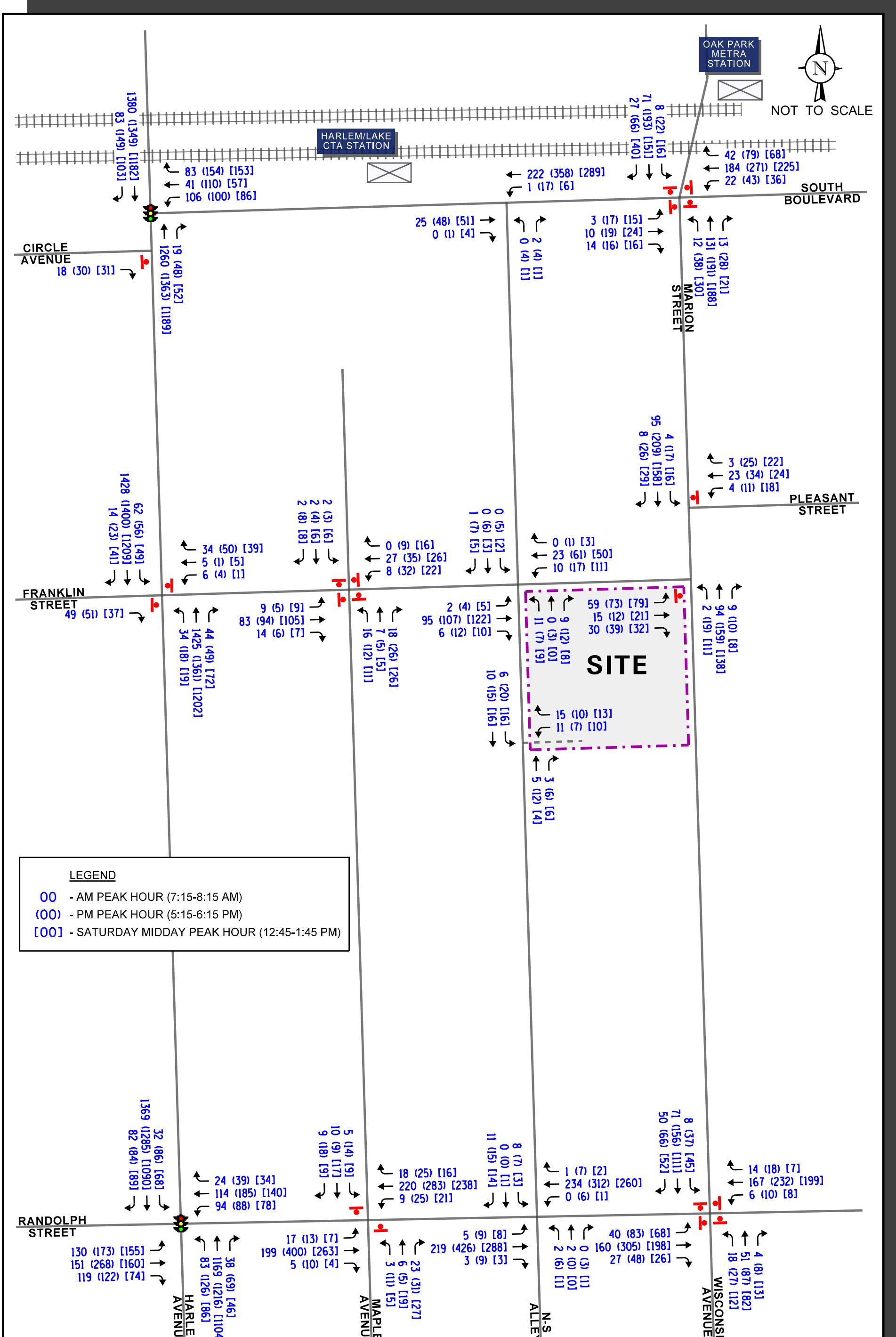
203 SOUTH MARION  
STREET  
OAK PARK ILLINOIS

## ESTIMATED SITE-GENERATED VEHICLE TRAFFIC VOLUMES

**KLOA**  
Kenig,Lindgren,O'Hara,Aboona,Inc.  
Job No: 20-193      Figure: 7







## 5. Traffic Analysis and Recommendations

The following provides an evaluation conducted for the weekday morning, weekday evening and Saturday midday peak hours. The analysis includes conducting capacity analyses to determine how well the roadway system and access drives are projected to operate and whether any roadway improvements or modifications are required.

### Traffic Analyses

Roadway and adjacent or nearby intersection analyses were performed for the weekday morning, weekday evening, and Saturday midday peak hours for the existing (Year 2020), no-build (Year 2026), and future projected (Year 2026) traffic volumes.

The traffic analyses were performed using the methodologies outlined in the Transportation Research Board's *Highway Capacity Manual (HCM)*, 6<sup>th</sup> Edition and analyzed using Synchro/SimTraffic 10 software. The analysis for the traffic-signal controlled intersections were accomplished using actual cycle lengths, phasings, and offsets to determine the average overall vehicle delay and levels of service.

The analyses for the unsignalized intersections determine the average control delay to vehicles at an intersection. Control delay is the elapsed time from a vehicle joining the queue at a stop sign (includes the time required to decelerate to a stop) until its departure from the stop sign and resumption of free flow speed. The methodology analyzes each intersection approach controlled by a stop sign and considers traffic volumes on all approaches and lane characteristics.

The ability of an intersection to accommodate traffic flow is expressed in terms of level of service, which is assigned a letter from A to F based on the average control delay experienced by vehicles passing through the intersection. The *Highway Capacity Manual* definitions for levels of service and the corresponding control delay for signalized intersections and unsignalized intersections are included in the Appendix of this report.

Summaries of the traffic analysis results showing the level of service and overall intersection delay (measured in seconds) for the existing, Year 2026 no-build conditions, and Year 2026 total projected conditions are presented in **Tables 11** through **15**. A discussion of the intersections follows. Summary sheets for the capacity analyses are included in the Appendix.

Table 11

## CAPACITY ANALYSIS RESULTS – HARLEM AVENUE WITH SOUTH BOULEVARD – SIGNALIZED

	Peak Hour	Westbound		Northbound	Southbound	Overall			
		L	R	T/R	T/L				
Year 2020 Existing Traffic Volumes	Weekday Morning Peak Hour	E 62.6	E 60.8	B 12.8	A 7.0	B 13.7			
		E - 61.9							
		E 59.7	E 64.3						
	Weekday Evening Peak Hour	E - 61.6		A 8.9	A 9.3	B 14.9			
		D 50.3	E 62.8	A 8.6	A 7.7	B 13.1			
	Saturday Midday Peak Hour	E - 56.7							
Year 2026 No Build Traffic Volumes	Weekday Morning Peak Hour	E 62.4	E 60.9	B 13.7	A 7.7	B 14.5			
		E - 61.8							
		E 59.1	E 64.2	A 9.8	A 9.8	B 15.6			
	Weekday Evening Peak Hour	E - 61.3							
		D 49.7	E 62.7	A 9.3	A 8.1	B 13.8			
	Saturday Midday Peak Hour	E - 56.4							
Year 2026 Projected Traffic Volumes	Weekday Morning Peak Hour	E 62.4	E 60.9	B 14.5	A 7.8	B 14.8			
		E - 61.8							
		E 59.1	E 64.2	A 9.7	A 9.9	B 15.5			
	Weekday Evening Peak Hour	E - 61.3							
		D 49.7	E 62.7	A 9.2	A 8.2	B 13.7			
	Saturday Midday Peak Hour	E - 56.4							

Table 12

## CAPACITY ANALYSIS RESULTS – HARLEM AVENUE WITH RANDOLPH STREET – SIGNALIZED

	Peak Hour	Eastbound		Westbound		Northbound		Southbound		Overall
		L	T/R	L	T/R	L	T/R	L	T/R	
Year 2020 Existing Traffic Volumes	Weekday Morning Peak Hour	D 36.1	E 70.5	C 34.5	D 51.3	B 16.5	C 20.0	B 12.3	C 33.5	C 31.9
	E – 59.2		D – 45.3		B – 19.8		C – 33.1			
	Weekday Evening Peak Hour	D 41.0	F 96.1	D 37.1	E 56.1	C 34.8	C 25.7	B 18.1	D 38.5	D 40.8
		E – 79.1		D – 51.0		C – 26.5		D – 37.3		
	Saturday Midday Peak Hour	D 39.2	E 58.2	C 31.9	D 53.8	B 12.4	C 20.4	B 15.0	C 30.7	C 29.9
		D – 50.6		D – 47.8		B – 19.9		C – 29.8		
Year 2026 No Build Traffic Volumes	Weekday Morning Peak Hour	D 36.2	E 71.7	C 34.7	D 51.4	C 21.9	C 20.5	B 11.9	C 34.7	C 32.8
	E – 60.1		D – 45.4		C – 20.6		C – 34.2			
	Weekday Evening Peak Hour	D 41.5	F 99+	D 37.0	E 56.7	D 38.8	C 26.7	B 18.0	D 38.8	D 44.3
		F – 98.9		D – 51.5		C – 27.8		D – 37.6		
	Saturday Midday Peak Hour	D 39.7	E 58.5	C 32.0	D 54.0	B 12.9	C 20.9	B 14.8	C 30.9	C 30.2
		D – 51.0		D – 48.0		C – 20.3		C – 30.0		
Year 2026 Projected Traffic Volumes	Weekday Morning Peak Hour	D 35.1	E 76.7	D 36.3	D 49.8	C 24.3	C 21.5	B 11.9	D 36.0	C 34.1
	E – 63.2		D – 44.3		C – 21.7		C – 35.5			
	Weekday Evening Peak Hour	D 41.7	F 99+	D 38.0	E 57.0	D 38.8	C 26.9	B 18.1	D 38.8	D 44.9
		F – 102.4		D – 51.6		C – 28.0		D – 37.5		
	Saturday Midday Peak Hour	D 39.3	E 58.7	C 32.7	D 53.6	B 13.1	C 21.3	B 14.9	C 31.2	C 30.4
		D – 51.0		D – 47.1		C – 20.7		C – 30.3		

Table 13  
CAPACITY ANALYSIS RESULTS – EXISTING CONDITIONS - UNSIGNALIZED

Intersection	Weekday Morning Peak Hour		Weekday Evening Peak Hour		Saturday Midday Peak Hour	
	LOS	Delay	LOS	Delay	LOS	Delay
<b>Harlem Avenue with Circle Avenue</b>						
• Eastbound Approach	C	24.3	C	21.6	C	19.2
<b>South Boulevard with North-South Public Alley</b>						
• Northbound Approach	A	8.5	B	10.9	B	10.8
• Westbound Left Turns	A	7.3	A	7.5	A	7.7
<b>Harlem Avenue with Pleasant Street</b>						
• Northbound Left Turns	B	13.5	B	13.8	B	12.2
• Eastbound Approach	C	17.0	C	16.9	B	14.9
• Westbound Approach	F	99+	F	99+	F	99+
• Southbound Left Turns	B	12.5	B	13.3	B	12.6
<b>Pleasant Street with North-South Public Alley</b>						
• Northbound Approach	A	9.5	A	9.7	A	9.5
• Eastbound Left Turns	A	7.3	A	7.3	A	7.3
• Westbound Left Turns	A	7.5	A	7.5	A	7.5
• Southbound Approach	A	8.5	A	9.9	A	9.5
<b>Marion Street with Pleasant Street</b>						
• Northbound Left Turns	A	7.5	A	8.1	A	7.9
• Eastbound Approach	B	11.0	C	19.0	C	15.3
• Westbound Approach	B	11.1	C	15.3	B	13.0
• Southbound Left Turns	A	8.4	A	7.9	A	7.7
<b>Randolph Street with Maple Avenue</b>						
• Northbound Approach	B	11.2	C	18.5	B	13.1
• Eastbound Left Turns	A	7.8	A	8.2	A	7.9
• Westbound Left Turns	A	7.8	A	8.9	A	7.9
• Southbound Approach	B	13.0	C	20.4	B	14.6
LOS = Level of Service Delay is measured in seconds.						

Table 13, cont'd

## CAPACITY ANALYSIS RESULTS – EXISTING CONDITIONS – UNSIGNALIZED

<b>Intersection</b>	<b>Weekday Morning Peak Hour</b>		<b>Weekday Evening Peak Hour</b>		<b>Saturday Midday Peak Hour</b>	
	<b>LOS</b>	<b>Delay</b>	<b>LOS</b>	<b>Delay</b>	<b>LOS</b>	<b>Delay</b>
<b>Randolph Street with North-South Public Alley</b>						
• Northbound Approach	B	13.0	C	16.4	B	12.4
• Eastbound Left Turns	A	7.8	A	8.0	A	7.9
• Westbound Left Turns	A	0.1	A	8.3	A	8.0
• Southbound Approach	B	12.0	B	13.1	B	10.7
<b>Maple Avenue with Pleasant Street</b>						
• Overall	A	7.6	A	7.6	A	7.6
• Eastbound Approach	A	7.7	A	7.7	A	7.8
• Westbound Approach	A	7.4	A	7.7	A	7.5
• Northbound Approach	A	7.4	A	7.3	A	7.3
• Southbound Approach	A	7.2	A	7.2	A	7.6
<b>Randolph Street with Marion Street</b>						
• Overall	A	9.0	C	17.9	B	11.3
• Eastbound Approach	A	9.3	C	23.4	B	12.2
• Westbound Approach	A	9.1	B	14.3	B	11.0
• Northbound Approach	A	8.6	B	11.7	A	9.8
• Southbound Approach	A	8.5	B	14.6	B	11.0
LOS = Level of Service Delay is measured in seconds.						

Table 8

## CAPACITY ANALYSIS RESULTS – NO BUILD CONDITIONS - UNSIGNALIZED

Intersection	Weekday Morning Peak Hour		Weekday Evening Peak Hour		Saturday Midday Peak Hour	
	LOS	Delay	LOS	Delay	LOS	Delay
<b>Harlem Avenue with Circle Avenue</b>						
• Eastbound Approach	D	26.4	C	23.2	C	20.6
<b>South Boulevard with North-South Public Alley</b>						
• Northbound Approach	A	8.6	B	11.3	B	11.3
• Westbound Left Turns	A	7.3	A	7.6	A	7.8
<b>Harlem Avenue with Pleasant Street</b>						
• Northbound Left Turns	B	14.2	B	14.2	B	12.4
• Eastbound Approach	C	18.0	C	17.5	C	15.2
• Westbound Approach	F	99+	F	99+	F	99+
• Southbound Left Turns	B	12.8	B	13.9	B	12.9
<b>Pleasant Street with North-South Public Alley</b>						
• Northbound Approach	A	9.5	A	9.8	A	9.6
• Eastbound Left Turns	A	7.3	A	7.4	A	7.3
• Westbound Left Turns	A	7.5	A	7.6	A	7.5
• Southbound Approach	A	8.5	A	9.9	A	9.6
<b>Marion Street with Pleasant Street</b>						
• Northbound Left Turns	A	7.5	A	8.1	A	8.0
• Eastbound Approach	B	11.4	C	20.8	C	16.4
• Westbound Approach	B	11.6	C	16.4	B	13.7
• Southbound Left Turns	A	8.4	A	8.0	A	7.8
<b>Randolph Street with Maple Avenue</b>						
• Northbound Approach	B	11.4	C	19.3	B	13.2
• Eastbound Left Turns	A	7.9	A	8.3	A	7.8
• Westbound Left Turns	A	7.8	A	9.1	A	7.9
• Southbound Approach	B	13.2	C	21.3	B	14.6
LOS = Level of Service						
Delay is measured in seconds.						

Table 14, cont'd

## CAPACITY ANALYSIS RESULTS – NO BUILD CONDITIONS – UNSIGNALIZED

<b>Intersection</b>	<b>Weekday Morning Peak Hour</b>		<b>Weekday Evening Peak Hour</b>		<b>Saturday Midday Peak Hour</b>	
	<b>LOS</b>	<b>Delay</b>	<b>LOS</b>	<b>Delay</b>	<b>LOS</b>	<b>Delay</b>
<b>Randolph Street with North-South Public Alley</b>						
• Northbound Approach	B	13.2	C	16.7	B	12.5
• Eastbound Left Turns	A	7.8	A	8.0	A	8.0
• Westbound Left Turns	A	0.1	A	8.4	A	8.0
• Southbound Approach	B	12.2	B	13.2	B	10.8
<b>Maple Avenue with Pleasant Street</b>						
• Overall	A	7.6	A	7.6	A	7.6
• Eastbound Approach	A	7.7	A	7.8	A	7.8
• Westbound Approach	A	7.4	A	7.7	A	7.5
• Northbound Approach	A	7.4	A	7.3	A	7.3
• Southbound Approach	A	7.2	A	7.2	A	7.7
<b>Randolph Street with Marion Street</b>						
• Overall	A	9.2	C	19.3	B	11.6
• Eastbound Approach	A	9.5	D	25.9	B	12.6
• Westbound Approach	A	9.4	C	15.1	B	11.2
• Northbound Approach	A	8.7	B	12.2	A	10.0
• Southbound Approach	A	8.7	C	15.5	B	11.3
LOS = Level of Service Delay is measured in seconds.						

Table 15

## CAPACITY ANALYSIS RESULTS – PROJECTED CONDITIONS - UNSIGNALIZED

Intersection	Weekday Morning Peak Hour		Weekday Evening Peak Hour		Saturday Midday Peak Hour	
	LOS	Delay	LOS	Delay	LOS	Delay
<b>Harlem Avenue with Circle Avenue</b>						
• Eastbound Approach	D	26.7	C	23.3	C	20.8
<b>South Boulevard with North-South Public Alley</b>						
• Northbound Approach	A	8.6	B	11.3	B	11.3
• Westbound Left Turns	A	7.4	A	7.6	A	7.8
<b>Harlem Avenue with Pleasant Street</b>						
• Northbound Left Turns	B	14.2	B	14.2	B	12.4
• Eastbound Approach	C	18.0	C	17.5	C	15.2
• Westbound Approach	F	99+	F	99+	F	99+
• Southbound Left Turns	B	13.1	B	14.3	B	13.2
<b>Pleasant Street with North-South Public Alley</b>						
• Northbound Approach	A	9.8	B	10.2	A	9.9
• Eastbound Left Turns	A	7.3	A	7.4	A	7.3
• Westbound Left Turns	A	7.6	A	7.6	A	7.6
• Southbound Approach	A	8.5	B	10.2	A	9.8
<b>Marion Street with Pleasant Street</b>						
• Northbound Left Turns	A	7.6	A	8.6	A	8.4
• Eastbound Approach	B	12.8	D	31.0	C	21.9
• Westbound Approach	B	13.6	C	21.0	C	16.8
• Southbound Left Turns	A	9.0	A	8.2	A	8.0
<b>Randolph Street with Maple Avenue</b>						
• Northbound Approach	B	11.7	C	19.7	B	13.5
• Eastbound Left Turns	A	7.9	A	8.3	A	7.9
• Westbound Left Turns	A	7.9	A	9.1	A	7.9
• Southbound Approach	B	13.7	C	21.9	C	15.1
LOS = Level of Service						
Delay is measured in seconds.						

Table 15, cont'd

## CAPACITY ANALYSIS RESULTS – PROJECTED CONDITIONS – UNSIGNALIZED

<b>Intersection</b>	<b>Weekday Morning Peak Hour</b>		<b>Weekday Evening Peak Hour</b>		<b>Saturday Midday Peak Hour</b>	
	<b>LOS</b>	<b>Delay</b>	<b>LOS</b>	<b>Delay</b>	<b>LOS</b>	<b>Delay</b>
<b>Randolph Street with North-South Public Alley</b>						
• Northbound Approach	B	13.8	C	17.3	B	12.9
• Eastbound Left Turns	A	7.9	A	8.0	A	8.0
• Westbound Left Turns	A	0.1	A	8.4	A	8.0
• Southbound Approach	B	11.9	B	13.5	B	11.6
<b>Maple Avenue with Pleasant Street</b>						
• Overall	A	7.7	A	7.7	A	7.8
• Eastbound Approach	A	7.9	A	7.9	A	8.0
• Westbound Approach	A	7.5	A	7.7	A	7.6
• Northbound Approach	A	7.5	A	7.4	A	7.4
• Southbound Approach	A	7.3	A	7.3	A	7.7
<b>Randolph Street with Marion Street</b>						
• Overall	A	9.6	C	20.5	B	12.1
• Eastbound Approach	A	10.0	D	28.1	B	13.3
• Westbound Approach	A	9.6	C	15.6	B	11.6
• Northbound Approach	A	9.0	B	12.6	B	10.3
• Southbound Approach	A	9.1	C	16.2	B	11.8
<b>North-South Public Alley with Proposed Garage Access</b>						
• Westbound Approach	A	8.6	A	8.7	A	8.6
• Southbound Left Turns	A	7.2	A	7.3	A	7.2
LOS = Level of Service Delay is measured in seconds.						

## Discussion and Recommendations

The following summarizes how the intersections are projected to operate and identifies any roadway and traffic control improvements necessary to accommodate the development traffic.

### *Harlem Avenue with South Boulevard*

The results of the capacity analysis indicate that overall this intersection currently operates at Level of Service (LOS) B during the weekday morning, weekday evening and Saturday midday peak hours. The westbound approach currently operates at LOS E during all three peak hours. Additionally, the northbound and southbound approaches are operating at LOS B or better during all three peak hours.

Under Year 2026 no-build conditions, overall this intersection will continue to operate at LOS B during all three peak hours with increases in delay of approximately one second. All approaches will continue to operate at the same existing levels of service with increases in delay of approximately one second.

Under Year 2026 total projected conditions, overall this intersection will continue to operate at LOS B during all three peak hours with increases in delay of approximately one second. All approaches will continue to operate at the same levels of service with increases in delay of approximately one second over no-build conditions. Furthermore, the proposed development is projected to increase the volume of traffic traversing this intersection by less than one percent during the three peak hours. As such, this intersection has sufficient reserve capacity to accommodate the traffic projected to be generated by the development and no roadway improvements and/or traffic control modifications are required.

### *Harlem Avenue with Randolph Street*

The results of the capacity analysis indicate that overall this intersection currently operates at LOS C during the weekday morning and Saturday midday peak hours and LOS D during the weekday evening peak hour. The eastbound currently operates at LOS E during the weekday morning and evening peak hours and LOS D during the Saturday midday peak hour. In addition, the westbound, northbound, and southbound approaches are operating at an acceptable LOS D or better during all three peak hours.

Under Year 2026 no-build conditions, overall this intersection will operate at the same existing levels of service with increases in delay of approximately four seconds or less during all three peak hours. The westbound, northbound and southbound approach will operate at an acceptable LOS D or better during all three peak hours with increases in delay of approximately one second. In addition, the eastbound approach will operate at LOS E during the weekday morning peak hour, LOS F during the weekday evening peak hour, and LOS D during the Saturday midday peak hour with increases in delay of approximately one second, 20 seconds, and one second, respectively.

Under Year 2026 total projected conditions, overall this intersection will continue to operate at the same levels of service with increases in delay of approximately one second during all three peak hours. All approaches will continue to operate at the same levels of service with increases in delay of less than four seconds during all three peak hours. Furthermore, the proposed development is projected to increase the volume of traffic traversing this intersection by approximately one percent during the three peak hours. As such, this intersection has sufficient reserve capacity to accommodate the traffic projected to be generated by the development and no roadway improvements and/or traffic control modifications are required.

#### *Harlem Avenue with Circle Avenue*

The results of the capacity analysis indicate that the eastbound approach currently operates at LOS C during the weekday morning, weekday evening and Saturday midday peak hours.

Under Year 2026 no-build conditions, the eastbound approach will operate at LOS D during the weekday morning peak hour and LOS C during the weekday evening and Saturday midday peak hours with increases in delay of approximately two seconds or less.

Under Year 2026 total projected conditions, the eastbound approach will continue to operate at the same levels of service with increases in delay of less than one second during all three peak hours. As such, this intersection has sufficient reserve capacity to accommodate the traffic projected to be generated by the development and no roadway improvements and/or traffic control modifications are required.

#### *South Boulevard with North-South Public Alley*

The results of the capacity analysis indicate that the northbound approach and westbound left turning movements are operating at LOS B or better during the weekday morning, weekday evening, and Saturday midday peak hours.

Under Year 2026 no-build conditions, the northbound approach and westbound left turning movements will continue to operate at LOS B or better during all three peak hours with increases in delay of less one second.

Under Year 2026 total projected conditions, the northbound approach and westbound left turning movements will continue to operate at the same levels of service with increases in delay of less than one second during all three peak hours. As such, this intersection has sufficient reserve capacity to accommodate the traffic projected to be generated by the development and no roadway improvements and/or traffic control modifications are required.

#### *Harlem Avenue with Pleasant Street*

The results of the capacity analysis indicate that the northbound and southbound left turning movements are operating at LOS B during the weekday morning, weekday evening, and Saturday midday peak hours. The eastbound approach currently operates at LOS C or better during all three peak hours. Additionally, the westbound approach is operating at LOS F during all three peak hours. This is normal and expected when a minor approach intersects a major road such as Harlem Avenue.

Under Year 2026 no-build conditions, the eastbound approach and the northbound and southbound left turning movements will continue to operate at LOS C or better during all three peak hours with increases in delay of less than one second. In addition, the westbound approach will continue to operate at LOS F during all three peak hours.

Under Year 2026 projected conditions, all movements will continue to operate at the same levels of service with increases in delay of less than one second. As such, this intersection has sufficient reserve capacity to accommodate the traffic projected to be generated by the development and no roadway improvements and/or traffic control modifications are required.

#### *Pleasant Street with North-South Public Alley*

The results of the capacity analysis indicate that all movements are operating at LOS A during the weekday morning, weekday evening, and Saturday midday peak hours.

Under Year 2026 no-build conditions, all movements will continue to operate at LOS A during all three peak hours with increases in delay of less than one second during all three peak hours.

Under Year 2026 total projected conditions, the northbound and southbound approaches will operate at LOS A during the weekday morning and Saturday midday peak hours and LOS B during the weekday evening peak hour with increases in delay of less than one second with 95<sup>th</sup> percentile queues of one to two vehicles. The eastbound and westbound left turning movements will continue to operate at LOS A during all three peak hours with increases in delay of less than one second. As such, this intersection has sufficient reserve capacity to accommodate the traffic projected to be generated by the development and no roadway improvements and/or traffic control modifications are required.

#### *Marion Street with Pleasant Street*

The results of the capacity analysis indicate that the northbound and southbound left turning movements are operating at LOS A during the weekday morning, weekday evening and Saturday midday peak hours. Additionally, the eastbound and westbound approaches currently operate at LOS C or better during all three peak hours.

Under Year 2026 no-build conditions, all movements will continue to operate at the same existing levels of service with increases in delay of approximately two seconds or less.

Under Year 2026 total projected conditions, the eastbound approach will operate at LOS B during the weekday morning peak hour, LOS D during the weekday evening peak hour, and LOS C during the Saturday midday peak hour with increases in delay of approximately one second, seven seconds and four seconds, respectively, with 95<sup>th</sup> percentile queues of one to three vehicles. The westbound approach will operate at LOS B during the weekday morning peak hour and LOS C during the weekday evening peak hour and Saturday midday peak hour with increases in delay of approximately four seconds or less. In addition, the northbound and southbound left turning movements will continue to operate at LOS A during all three peak hours with increases in delay of approximately one second and will experience 95<sup>th</sup> percentile queues of one to two vehicles during all three peak hours. As such, this intersection has sufficient reserve capacity to accommodate the traffic projected to be generated by the development and no roadway improvements and/or traffic control modifications are required.

#### *Randolph Street with Maple Avenue*

The results of the capacity analysis indicate that the eastbound and westbound left turning movements currently operate at LOS A during the weekday morning, weekday evening, and Saturday midday peak hours. Additionally, the northbound and southbound approaches currently operate at LOS C or better during all three peak hours.

Under Year 2026 no-build conditions, all movements will operate at the same existing levels of service with increases in delay of less than one second during all three peak hours.

Under Year 2026 total projected conditions, the southbound approach will operate at LOS B during the weekday morning peak hour and LOS C during the weekday evening and Saturday midday peak hours with increases in delay of less than one second. All other movements will continue to operate at the same levels of service with increases in delay of less than one second during all three peak hours. As such, this intersection has sufficient reserve capacity to accommodate the traffic projected to be generated by the development and no roadway improvements and/or traffic control modifications are required.

#### *Randolph Street with North-South Public Alley*

The results of the capacity analysis indicate that the eastbound and westbound left turning movements currently operate at LOS A during the weekday morning, weekday evening, and Saturday midday peak hours. Additionally, the northbound and southbound approaches currently operate at LOS C or better during all three peak hours.

Under Year 2026 no-build conditions, all movements will operate at the same existing levels of service with increases in delay of less than one second during all three peak hours.

Under Year 2026 total projected conditions, all movements will operate at the same levels of service with increases in delay of less than one second during all three peak hours. As such, this intersection has sufficient reserve capacity to accommodate the traffic projected to be generated by the development and no roadway improvements and/or traffic control modifications are required.

### *Maple Avenue with Pleasant Street*

The results of the capacity analysis indicate that overall this intersection currently operates at LOS A during the weekday morning, weekday evening and Saturday midday peak hours. In addition, all approaches are operating at LOS A during all three peak hours.

Under Year 2026 no-build conditions, overall this intersection will continue to operate at LOS A during all three peak hours with increases in delay of less than one second. In addition, all approaches will continue to operate at LOS A during all three peak hours with increases in delay of less than one second.

Under Year 2026 total projected conditions, overall this intersection will continue to operate at LOS A during all three peak hours with increases in delay of less than one second. In addition, all approaches will continue to operate at LOS A during all three peak hours with increases in delay of less than one second. As such, this intersection has sufficient reserve capacity to accommodate the traffic projected to be generated by the development and no roadway improvements and/or traffic control modifications are required.

### *Randolph Street with Marion Street*

The results of the capacity analysis indicate that overall this intersection currently operates at LOS C or better during the weekday morning, weekday evening, and Saturday midday peak hours. In addition, all approaches are operating at LOS C or better during all three peak hours.

Under Year 2026 no-build conditions, overall this intersection will operate at the same existing levels of service with increases in delay of approximately one second during all three peak hours. The eastbound approach will operate at LOS A during the weekday morning peak hour, LOS D during the weekday evening peak hour, and LOS B during the Saturday midday peak hour with increases in less than three seconds. All other approaches will operate at LOS C or better during the three peak hours with increases in delay of approximately one second.

Under Year 2026 total projected conditions, overall this intersection will continue to operate at the same levels of service with increases in delay of approximately one second over no-build conditions. All approaches will operate at the same levels of service during all three peak hours with increases in delay of approximately two seconds or less. As such, this intersection has sufficient reserve capacity to accommodate the traffic projected to be generated by the development and no roadway improvements and/or traffic control modifications are required.

### *North-South Public Alley with Proposed Garage Access*

The results of the capacity analysis indicate that the outbound movements from the proposed garage access onto the public alley will operate at LOS A during the weekday morning, weekday evening, and Saturday midday peak hours with 95<sup>th</sup> percentile queues of one to two vehicles. In addition, the southbound left turning movements will operate at LOS A during all three peak hours with 95<sup>th</sup> percentile queues of one to two vehicles. As such, access to and from the parking garage will adequately accommodate the residential development-generated traffic. Visual warning devices should be provided at the garage exit.

## Marion Street and Pleasant Street All-Way Stop Warrant Analysis

The intersection of Marion Street and Pleasant Street was further evaluated to determine if an all-way stop traffic control is warranted.

### *All-Way Stop Sign Warrants*

The *Manual on Uniform Control Devices* (MUTCD) indicates that an all-way stop sign control should be considered if any of the following conditions are met:

1. Where traffic control signals are justified, the multi-way stop is an interim measure that can be installed quickly to control traffic while arrangements are being made for the installation of the traffic control signal.
2. A crash problem, as indicated by five or more reported crashes in a 12-month period that are susceptible to correction by a multi-way stop installation. Such crashes include right- and left-turn collisions as well as right-angle collisions.
3. Minimum volumes warranting a multi-way stop:
  - a. The vehicular volume entering the intersection from the major street approaches (total of both approaches) averages at least 300 vehicles per hour for any 8 hours of an average day, and
  - b. The combined vehicular, pedestrian, and bicycle volume entering the intersection from the minor street approaches (total of both approaches) averages at least 200 units per hour for the same 8 hours, with an average delay to minor-street vehicular traffic of at least 30 seconds per vehicle during the highest hour.
  - c. If the 85<sup>th</sup> percentile approach speed of the major-street traffic exceeds 40 mph, the minimum vehicular volume warrants are 70 percent of the values provided on a and b.
  - d. Where no single criterion is satisfied, but where Criteria 2, 3a and 3b are all satisfied to 80 percent of the minimum values. Criterion 3c is excluded from this condition.

### *All-Way Stop Sign Evaluation*

Based on a review of the Year 2026 total volumes and the MUTCD warrants for all-way stop sign control, the weekday evening and Saturday midday peak hour vehicle, pedestrian, and bicycle volumes just meet the minimum volume thresholds. However, the MUTCD also states that the average hourly volumes over an eight-hour period on an average day must meet the minimum volume thresholds requirements. As such, since the peak hour volumes just meet the thresholds, it is unlikely that the average volumes over an eight-hour period will meet the minimum thresholds.

Further, crash data obtained from the Village of Oak Park shows that 18 accidents over the five-year period occurred within the influence of the intersection. However, 12 of the 18 accidents involved parked vehicle and, as such, did not occur at or within the intersection. As such, it can be seen that only six accidents occurred at the intersection or within the intersection and does not meet the MUTCD crash warrant which requires “five or more reported crashes in a 12-month period that are susceptible to correction by a multi-way stop installation”.

Per the request of the Village, sight distance studies were performed along both legs of Pleasant Street at its intersection with Marion Street and are included in the Appendix. The sight distance studies show that motorists stopped at the stop signs on Pleasant Street have limited sight distance looking north and south along Marion Street. However, appropriate sight distance is available once the motorists pulls closer to the edge of Marion Street after first stopping at the stop signs. This situation is common at other intersections in Oak Park and other similar urban areas. The low number of crashes at the intersection would further indicate that the sight distance at this intersection has not being an issue.

As such, it appears all-way stop sign control is not currently warranted at the intersection of Marion Street with Pleasant Street. Nevertheless, it is recommended that the intersection continue to be monitored in the future, particularly after the completion and occupancy of the subject development, to see if all-way stop sign control will be warranted and/or required.

## 6. Conclusion

Based on the preceding analyses and recommendations, the following conclusions have been made:

- The proposed development will not generate a significant amount of traffic, which will be reduced due to the location of the site within downtown Oak Park and its proximity to the train stations and PACE/CTA bus routes and other modes of transportation.
- The results of the capacity analysis indicate that the proposed development traffic will not have a significant impact on the area roadways.
- The development-generated traffic will only add less than one percent of the traffic projected to be traversing the intersections of Harlem Avenue with South Boulevard and Harlem Avenue with Randolph Street.
- Visual warning devices should be provided at both access drives to the parking garage.
- The proposed access drive will be adequate in accommodating the traffic projected to be generated by the proposed development and will ensure that a flexible access system is provided.

# Appendix

Traffic Count Adjustment Factors  
Traffic Count Summary Sheets  
Site Plan  
AutoTurn Exhibits  
Census Data  
ITE Trip Generation Sheets  
CMAP Projections Letter  
Level of Service Criteria  
Capacity Analysis Summary Sheets  
Sight Distance Analyses

# Traffic Count Adjustment Factors

## **Traffic Adjustment Factors**

### **Harlem Avenue**

- Southbound Approach
  - AM : increased by 15 percent
  - PM: increased by 15 percent
  - Saturday: increased by nothing
- Northbound Approach
  - AM : increased by 15 percent
  - PM: increased by 15 percent
  - Saturday: increased by nothing

### **Circle Avenue**

- Eastbound Approach
  - AM : increased by 30 percent
  - PM: increased by 15 percent
  - Saturday: increased by 30 percent

### **South Boulevard**

- Both directions
  - AM : increased by 40 percent
  - PM: increased by 10 percent
  - Saturday: increased by 10 percent

### **Pleasant**

- Both directions
  - AM : increased by 50 percent
  - PM: increased by 25 percent
  - Saturday: increased by 30 percent

### **Marion**

- Both directions
  - AM : increased by 30 percent
  - PM: increased by 10 percent
  - Saturday: increased by 20 percent

### **Maple**

- Northbound
  - AM : increased by 50 percent
  - PM: increased by 50 percent
  - Saturday: increased by 50 percent

### **Randolph**

- Both directions
  - AM : increased by 50 percent
  - PM: increased by 25 percent
  - Saturday: increased by 30 percent

# Traffic Count Summary Sheets



Kenig Lindgren O'Hara Aboona, Inc.  
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018  
(847)518-9990

Count Name: Harlem Avenue with Pleasant Street  
Site Code:  
Start Date: 09/10/2020  
Page No: 1

### Turning Movement Data

Start Time	Pleasant Street Eastbound							Pleasant Street Westbound							Harlem Avenue Northbound							Harlem Avenue Southbound							Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left			
7:00 AM	0	0	0	9	0	9	0	0	0	8	0	8	0	1	176	3	0	180	0	0	270	3	2	273	470				
7:15 AM	0	0	0	6	3	6	0	0	0	2	3	2	0	6	232	4	0	242	1	4	292	3	0	300	550				
7:30 AM	0	0	0	6	0	6	0	0	0	6	0	6	0	2	246	7	0	255	0	4	299	6	1	309	576				
7:45 AM	0	0	0	11	6	11	0	1	0	7	1	8	0	14	280	5	0	299	0	4	297	1	0	302	620				
Hourly Total	0	0	0	32	9	32	0	1	0	23	4	24	0	23	934	19	0	976	1	12	1158	13	3	1184	2216				
8:00 AM	0	0	0	9	6	9	1	2	0	2	0	5	0	7	266	9	0	282	1	12	294	2	0	309	605				
8:15 AM	0	1	0	5	5	6	0	0	0	2	3	2	0	5	240	7	0	252	0	8	256	2	1	266	526				
8:30 AM	0	0	0	5	3	5	0	0	0	5	1	5	0	2	281	8	0	291	0	5	258	6	0	269	570				
8:45 AM	0	1	0	3	3	4	0	0	0	4	1	4	0	3	271	6	0	280	0	3	248	1	2	252	540				
Hourly Total	0	2	0	22	17	24	1	2	0	13	5	16	0	17	1058	30	0	1105	1	28	1056	11	3	1096	2241				
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-				
4:00 PM	0	0	0	7	5	7	0	0	1	4	2	5	0	9	277	9	0	295	0	4	280	4	1	288	595				
4:15 PM	0	1	1	8	4	10	0	1	0	6	8	7	0	3	302	6	0	311	0	5	319	2	0	326	654				
4:30 PM	0	0	1	12	4	13	0	1	0	10	1	11	0	3	279	10	1	292	0	9	311	5	1	325	641				
4:45 PM	0	1	0	12	3	13	0	1	0	1	3	2	0	6	281	8	0	295	0	8	286	7	0	301	611				
Hourly Total	0	2	2	39	16	43	0	3	1	21	14	25	0	21	1139	33	1	1193	0	26	1196	18	2	1240	2501				
5:00 PM	0	0	1	8	6	9	0	0	1	18	3	19	0	4	271	8	0	283	0	9	273	6	1	288	599				
5:15 PM	0	1	0	6	7	7	1	0	0	7	4	8	0	2	248	12	0	262	0	12	282	8	0	302	579				
5:30 PM	0	0	1	11	1	12	0	0	0	4	1	4	0	6	271	8	0	285	0	6	255	5	0	266	567				
5:45 PM	0	2	0	5	4	7	0	0	0	2	4	2	0	4	258	5	0	267	0	9	314	9	0	332	608				
Hourly Total	0	3	2	30	18	35	1	0	1	31	12	33	0	16	1048	33	0	1097	0	36	1124	28	1	1188	2353				
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-					
11:30 AM	0	1	1	4	3	6	1	0	0	1	2	2	0	3	309	9	0	321	0	5	288	6	0	299	628				
11:45 AM	0	0	1	2	2	3	0	2	0	3	9	5	0	8	302	9	0	319	0	7	310	6	0	323	650				
Hourly Total	0	1	2	6	5	9	1	2	0	4	11	7	0	11	611	18	0	640	0	12	598	12	0	622	1278				
12:00 PM	0	1	0	11	0	12	0	0	0	8	1	8	0	6	277	7	0	290	0	9	280	4	2	293	603				
12:15 PM	0	0	0	15	4	15	0	1	1	4	0	6	0	2	292	14	0	308	1	3	278	8	0	290	619				
12:30 PM	0	0	1	8	2	9	0	1	0	6	2	7	0	3	252	4	0	259	1	7	282	3	1	293	568				
12:45 PM	0	1	0	13	1	14	0	0	0	6	2	6	0	4	276	8	0	288	0	7	288	1	0	296	604				
Hourly Total	0	2	1	47	7	50	0	2	1	24	5	27	0	15	1097	33	0	1145	2	26	1128	16	3	1172	2394				
1:00 PM	0	0	0	7	0	7	0	0	1	1	1	2	0	5	282	12	0	299	0	8	289	4	0	301	609				
1:15 PM	0	0	0	5	1	5	0	0	2	8	1	10	0	7	275	14	0	296	0	9	346	4	0	359	670				
1:30 PM	0	0	0	3	1	3	0	1	1	6	3	8	0	3	266	6	0	275	0	11	269	2	0	282	568				
1:45 PM	0	0	1	6	2	7	0	1	1	5	2	7	0	4	260	8	0	272	0	10	307	2	0	319	605				
Hourly Total	0	0	1	21	4	22	0	2	5	20	7	27	0	19	1083	40	0	1142	0	38	1211	12	0	1261	2452				

Grand Total	0	10	8	197	76	215	3	12	8	136	58	159	0	122	6970	206	1	7298	4	178	7471	110	12	7763	15435
Approach %	0.0	4.7	3.7	91.6	-	-	1.9	7.5	5.0	85.5	-	-	0.0	1.7	95.5	2.8	-	-	0.1	2.3	96.2	1.4	-	-	-
Total %	0.0	0.1	0.1	1.3	-	1.4	0.0	0.1	0.1	0.9	-	1.0	0.0	0.8	45.2	1.3	-	47.3	0.0	1.2	48.4	0.7	-	50.3	-
Lights	0	10	8	192	-	210	3	12	8	131	-	154	0	118	6760	204	-	7082	4	176	7274	109	-	7563	15009
% Lights	-	100.0	100.0	97.5	-	97.7	100.0	100.0	100.0	96.3	-	96.9	-	96.7	97.0	99.0	-	97.0	100.0	98.9	97.4	99.1	-	97.4	97.2
Buses	0	0	0	2	-	2	0	0	0	0	-	0	0	1	54	1	-	56	0	0	54	0	-	54	112
% Buses	-	0.0	0.0	1.0	-	0.9	0.0	0.0	0.0	0.0	-	0.0	-	0.8	0.8	0.5	-	0.8	0.0	0.0	0.7	0.0	-	0.7	0.7
Single-Unit Trucks	0	0	0	1	-	1	0	0	0	5	-	5	0	2	108	1	-	111	0	2	94	0	-	96	213
% Single-Unit Trucks	-	0.0	0.0	0.5	-	0.5	0.0	0.0	0.0	3.7	-	3.1	-	1.6	1.5	0.5	-	1.5	0.0	1.1	1.3	0.0	-	1.2	1.4
Articulated Trucks	0	0	0	2	-	2	0	0	0	0	-	0	0	1	48	0	-	49	0	0	49	1	-	50	101
% Articulated Trucks	-	0.0	0.0	1.0	-	0.9	0.0	0.0	0.0	0.0	-	0.0	-	0.8	0.7	0.0	-	0.7	0.0	0.0	0.7	0.9	-	0.6	0.7
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	0	
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	-	76	-	-	-	-	-	58	-	-	-	-	-	1	-	-	-	-	-	12	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-



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Rosemont, Illinois, United States 60018  
(847)518-9990

Count Name: Harlem Avenue with Pleasant  
Street  
Site Code:  
Start Date: 09/10/2020  
Page No: 3

## Turning Movement Peak Hour Data (7:15 AM)



Kenig Lindgren O'Hara Aboona, Inc.  
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(847)518-9990

Count Name: Harlem Avenue with Pleasant Street  
Site Code:  
Start Date: 09/10/2020  
Page No: 4

### Turning Movement Peak Hour Data (4:15 PM)

Start Time	Pleasant Street Eastbound						Pleasant Street Westbound						Harlem Avenue Northbound						Harlem Avenue Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
4:15 PM	0	1	1	8	4	10	0	1	0	6	8	7	0	3	302	6	0	311	0	5	319	2	0	326	654
4:30 PM	0	0	1	12	4	13	0	1	0	10	1	11	0	3	279	10	1	292	0	9	311	5	1	325	641
4:45 PM	0	1	0	12	3	13	0	1	0	1	3	2	0	6	281	8	0	295	0	8	286	7	0	301	611
5:00 PM	0	0	1	8	6	9	0	0	1	18	3	19	0	4	271	8	0	283	0	9	273	6	1	288	599
Total	0	2	3	40	17	45	0	3	1	35	15	39	0	16	1133	32	1	1181	0	31	1189	20	2	1240	2505
Approach %	0.0	4.4	6.7	88.9	-	-	0.0	7.7	2.6	89.7	-	-	0.0	1.4	95.9	2.7	-	-	0.0	2.5	95.9	1.6	-	-	-
Total %	0.0	0.1	0.1	1.6	-	1.8	0.0	0.1	0.0	1.4	-	1.6	0.0	0.6	45.2	1.3	-	47.1	0.0	1.2	47.5	0.8	-	49.5	-
PHF	0.000	0.500	0.750	0.833	-	0.865	0.000	0.750	0.250	0.486	-	0.513	0.000	0.667	0.938	0.800	-	0.949	0.000	0.861	0.932	0.714	-	0.951	0.958
Lights	0	2	3	40	-	45	0	3	1	34	-	38	0	15	1102	32	-	1149	0	31	1156	20	-	1207	2439
% Lights	-	100.0	100.0	100.0	-	100.0	-	100.0	100.0	97.1	-	97.4	-	93.8	97.3	100.0	-	97.3	-	100.0	97.2	100.0	-	97.3	97.4
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	1	8	0	-	9	0	0	6	0	-	6	15
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	6.3	0.7	0.0	-	0.8	-	0.0	0.5	0.0	-	0.5	0.6
Single-Unit Trucks	0	0	0	0	-	0	0	0	0	1	-	1	0	0	14	0	-	14	0	0	18	0	-	18	33
% Single-Unit Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	2.9	-	2.6	-	0.0	1.2	0.0	-	1.2	-	0.0	1.5	0.0	-	1.5	1.3
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	9	0	-	9	0	0	9	0	-	9	18
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.8	0.0	-	0.8	-	0.0	0.8	0.0	-	0.7	0.7
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	17	-	-	-	-	15	-	-	-	-	-	-	1	-	-	-	-	2	-	-	
% Pedestrians	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	



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Count Name: Harlem Avenue with Pleasant  
Street  
Site Code:  
Start Date: 09/10/2020  
Page No: 5

## Turning Movement Peak Hour Data (12:45 PM)



Kenig Lindgren O'Hara Aboona, Inc.  
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018  
(847)518-9990

Count Name: Harlem Avenue with Randolph Street  
Site Code:  
Start Date: 09/10/2020  
Page No: 1

### Turning Movement Data

Start Time	Randolph Street Eastbound						Randolph Street Westbound						Harlem Avenue Northbound						Harlem Avenue Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:00 AM	0	17	13	10	0	40	0	12	8	5	1	25	0	11	174	6	0	191	0	6	250	10	2	266	522
7:15 AM	0	28	22	23	1	73	1	18	17	4	3	40	0	13	223	2	3	238	0	3	274	15	1	292	643
7:30 AM	0	16	20	20	1	56	0	11	17	3	0	31	0	16	236	4	1	256	0	6	280	23	3	309	652
7:45 AM	0	21	26	19	1	66	0	10	18	7	1	35	0	22	271	9	2	302	0	8	271	16	1	295	698
Hourly Total	0	82	81	72	3	235	1	51	60	19	5	131	0	62	904	21	6	987	0	23	1075	64	7	1162	2515
8:00 AM	0	20	20	16	2	56	0	9	25	2	1	36	1	19	258	6	4	284	0	10	283	16	3	309	685
8:15 AM	0	21	19	15	3	55	0	12	17	6	4	35	0	22	239	6	3	267	0	4	252	5	2	261	618
8:30 AM	0	29	22	16	2	67	0	8	15	3	0	26	0	16	252	4	3	272	0	5	243	17	2	265	630
8:45 AM	0	24	25	14	0	63	0	8	17	4	2	29	0	16	237	7	4	260	0	10	237	9	2	256	608
Hourly Total	0	94	86	61	7	241	0	37	74	15	7	126	1	73	986	23	14	1083	0	29	1015	47	9	1091	2541
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
4:00 PM	0	37	52	25	4	114	0	14	35	4	5	53	0	35	244	10	7	289	0	11	253	19	5	283	739
4:15 PM	0	34	53	22	8	109	0	15	24	6	2	45	0	20	243	13	2	276	0	16	283	17	5	316	746
4:30 PM	0	37	40	29	3	106	0	13	35	7	2	55	0	25	272	21	0	318	0	18	294	18	3	330	809
4:45 PM	0	32	59	23	4	114	0	15	45	9	1	69	0	31	244	11	1	286	0	17	260	22	2	299	768
Hourly Total	0	140	204	99	19	443	0	57	139	26	10	222	0	111	1003	55	10	1169	0	62	1090	76	15	1228	3062
5:00 PM	0	33	58	22	9	113	0	21	42	8	0	71	0	32	246	8	1	286	0	22	244	15	4	281	751
5:15 PM	0	34	57	29	3	120	0	14	36	7	3	57	0	29	218	15	1	262	0	20	239	22	3	281	720
5:30 PM	0	24	64	15	1	103	0	18	37	2	0	57	0	38	238	6	2	282	0	14	243	14	0	271	713
5:45 PM	0	31	40	22	2	93	0	14	23	6	2	43	0	30	247	15	1	292	0	13	287	21	2	321	749
Hourly Total	0	122	219	88	15	429	0	67	138	23	5	228	0	129	949	44	5	1122	0	69	1013	72	9	1154	2933
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
11:30 AM	0	36	22	18	1	76	0	15	20	6	0	41	0	9	276	13	2	298	0	11	275	7	1	293	708
11:45 AM	0	31	31	12	0	74	0	17	20	8	0	45	0	13	277	13	0	303	0	18	285	13	0	316	738
Hourly Total	0	67	53	30	1	150	0	32	40	14	0	86	0	22	553	26	2	601	0	29	560	20	1	609	1446
12:00 PM	0	31	24	13	2	68	0	27	30	4	2	61	0	14	279	9	6	302	0	11	253	14	3	278	709
12:15 PM	0	28	25	15	1	68	0	13	31	8	1	52	0	15	251	13	0	279	0	15	263	23	2	301	700
12:30 PM	0	19	23	11	2	53	0	8	19	7	1	34	0	20	230	7	2	257	0	17	272	18	1	307	651
12:45 PM	0	24	31	8	1	63	0	16	35	7	0	58	0	21	276	10	0	307	0	17	254	22	1	293	721
Hourly Total	0	102	103	47	6	252	0	64	115	26	4	205	0	70	1036	39	8	1145	0	60	1042	77	7	1179	2781
1:00 PM	0	33	28	18	1	79	0	13	17	4	1	34	0	19	285	8	2	312	0	16	269	19	3	304	729
1:15 PM	0	31	28	15	1	74	0	13	32	3	1	48	0	15	269	10	2	294	0	21	294	23	0	338	754
1:30 PM	0	29	32	15	4	76	0	7	21	11	2	39	0	29	247	6	3	282	0	13	239	23	6	275	672
1:45 PM	0	22	32	18	3	72	0	11	24	7	1	42	0	16	253	7	5	276	0	12	291	11	6	314	704
Hourly Total	0	115	120	66	9	301	0	44	94	25	5	163	0	79	1054	31	12	1164	0	62	1093	76	15	1231	2859

Grand Total	0	722	866	463	60	2051	1	352	660	148	36	1161	1	546	6485	239	57	7271	0	334	6888	432	63	7654	18137
Approach %	0.0	35.2	42.2	22.6	-	-	0.1	30.3	56.8	12.7	-	-	0.0	7.5	89.2	3.3	-	-	0.0	4.4	90.0	5.6	-	-	-
Total %	0.0	4.0	4.8	2.6	-	11.3	0.0	1.9	3.6	0.8	-	6.4	0.0	3.0	35.8	1.3	-	40.1	0.0	1.8	38.0	2.4	-	42.2	-
Lights	0	713	856	458	-	2027	1	350	645	146	-	1142	0	540	6272	237	-	7049	0	330	6697	424	-	7451	17669
% Lights	-	98.8	98.8	98.9	-	98.8	100.0	99.4	97.7	98.6	-	98.4	0.0	98.9	96.7	99.2	-	96.9	-	98.8	97.2	98.1	-	97.3	97.4
Buses	0	1	1	1	-	3	0	0	0	0	-	0	0	2	57	0	-	59	0	0	55	0	-	55	117
% Buses	-	0.1	0.1	0.2	-	0.1	0.0	0.0	0.0	0.0	-	0.0	0.0	0.4	0.9	0.0	-	0.8	-	0.0	0.8	0.0	-	0.7	0.6
Single-Unit Trucks	0	6	4	3	-	13	0	1	4	2	-	7	0	4	105	2	-	111	0	4	91	8	-	103	234
% Single-Unit Trucks	-	0.8	0.5	0.6	-	0.6	0.0	0.3	0.6	1.4	-	0.6	0.0	0.7	1.6	0.8	-	1.5	-	1.2	1.3	1.9	-	1.3	1.3
Articulated Trucks	0	2	0	1	-	3	0	0	0	0	-	0	1	0	51	0	-	52	0	0	45	0	-	45	100
% Articulated Trucks	-	0.3	0.0	0.2	-	0.1	0.0	0.0	0.0	0.0	-	0.0	100.0	0.0	0.8	0.0	-	0.7	-	0.0	0.7	0.0	-	0.6	0.6
Bicycles on Road	0	0	5	0	-	5	0	1	11	0	-	12	0	0	0	0	-	0	0	0	0	-	0	0	17
% Bicycles on Road	-	0.0	0.6	0.0	-	0.2	0.0	0.3	1.7	0.0	-	1.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.1
Pedestrians	-	-	-	-	-	60	-	-	-	-	-	36	-	-	-	-	-	57	-	-	-	-	-	63	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-



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Count Name: Harlem Avenue with Randolph Street  
Site Code:  
Start Date: 09/10/2020  
Page No: 3

### Turning Movement Peak Hour Data (7:15 AM)

Start Time	Randolph Street Eastbound						Randolph Street Westbound						Harlem Avenue Northbound						Harlem Avenue Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:15 AM	0	28	22	23	1	73	1	18	17	4	3	40	0	13	223	2	3	238	0	3	274	15	1	292	643
7:30 AM	0	16	20	20	1	56	0	11	17	3	0	31	0	16	236	4	1	256	0	6	280	23	3	309	652
7:45 AM	0	21	26	19	1	66	0	10	18	7	1	35	0	22	271	9	2	302	0	8	271	16	1	295	698
8:00 AM	0	20	20	16	2	56	0	9	25	2	1	36	1	19	258	6	4	284	0	10	283	16	3	309	685
Total	0	85	88	78	5	251	1	48	77	16	5	142	1	70	988	21	10	1080	0	27	1108	70	8	1205	2678
Approach %	0.0	33.9	35.1	31.1	-	-	0.7	33.8	54.2	11.3	-	-	0.1	6.5	91.5	1.9	-	-	0.0	2.2	92.0	5.8	-	-	-
Total %	0.0	3.2	3.3	2.9	-	9.4	0.0	1.8	2.9	0.6	-	5.3	0.0	2.6	36.9	0.8	-	40.3	0.0	1.0	41.4	2.6	-	45.0	-
PHF	0.000	0.759	0.846	0.848	-	0.860	0.250	0.667	0.770	0.571	-	0.888	0.250	0.795	0.911	0.583	-	0.894	0.000	0.675	0.979	0.761	-	0.975	0.959
Lights	0	84	86	77	-	247	1	48	71	16	-	136	0	67	923	20	-	1010	0	26	1070	66	-	1162	2555
% Lights	-	98.8	97.7	98.7	-	98.4	100.0	100.0	92.2	100.0	-	95.8	0.0	95.7	93.4	95.2	-	93.5	-	96.3	96.6	94.3	-	96.4	95.4
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	2	9	0	-	11	0	0	9	0	-	9	20
% Buses	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	2.9	0.9	0.0	-	1.0	-	0.0	0.8	0.0	-	0.7	0.7
Single-Unit Trucks	0	1	2	1	-	4	0	0	1	0	-	1	0	1	39	1	-	41	0	1	21	4	-	26	72
% Single-Unit Trucks	-	1.2	2.3	1.3	-	1.6	0.0	0.0	1.3	0.0	-	0.7	0.0	1.4	3.9	4.8	-	3.8	-	3.7	1.9	5.7	-	2.2	2.7
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	1	0	17	0	-	18	0	0	8	0	-	8	26
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	100.0	0.0	1.7	0.0	-	1.7	-	0.0	0.7	0.0	-	0.7	1.0
Bicycles on Road	0	0	0	0	-	0	0	0	5	0	-	5	0	0	0	0	-	0	0	0	0	0	-	0	5
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	0.0	0.0	6.5	0.0	-	3.5	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.2
Pedestrians	-	-	-	-	5	-	-	-	-	-	5	-	-	-	-	-	10	-	-	-	-	8	-	-	
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	



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Count Name: Harlem Avenue with Randolph Street  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Peak Hour Data (4:15 PM)

Start Time	Randolph Street Eastbound						Randolph Street Westbound						Harlem Avenue Northbound						Harlem Avenue Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
4:15 PM	0	34	53	22	8	109	0	15	24	6	2	45	0	20	243	13	2	276	0	16	283	17	5	316	746
4:30 PM	0	37	40	29	3	106	0	13	35	7	2	55	0	25	272	21	0	318	0	18	294	18	3	330	809
4:45 PM	0	32	59	23	4	114	0	15	45	9	1	69	0	31	244	11	1	286	0	17	260	22	2	299	768
5:00 PM	0	33	58	22	9	113	0	21	42	8	0	71	0	32	246	8	1	286	0	22	244	15	4	281	751
Total	0	136	210	96	24	442	0	64	146	30	5	240	0	108	1005	53	4	1166	0	73	1081	72	14	1226	3074
Approach %	0.0	30.8	47.5	21.7	-	-	0.0	26.7	60.8	12.5	-	-	0.0	9.3	86.2	4.5	-	-	0.0	6.0	88.2	5.9	-	-	-
Total %	0.0	4.4	6.8	3.1	-	14.4	0.0	2.1	4.7	1.0	-	7.8	0.0	3.5	32.7	1.7	-	37.9	0.0	2.4	35.2	2.3	-	39.9	-
PHF	0.000	0.919	0.890	0.828	-	0.969	0.000	0.762	0.811	0.833	-	0.845	0.000	0.844	0.924	0.631	-	0.917	0.000	0.830	0.919	0.818	-	0.929	0.950
Lights	0	133	208	96	-	437	0	64	144	28	-	236	0	107	978	52	-	1137	0	72	1051	72	-	1195	3005
% Lights	-	97.8	99.0	100.0	-	98.9	-	100.0	98.6	93.3	-	98.3	-	99.1	97.3	98.1	-	97.5	-	98.6	97.2	100.0	-	97.5	97.8
Buses	0	0	1	0	-	1	0	0	0	0	-	0	0	0	11	0	-	11	0	0	7	0	-	7	19
% Buses	-	0.0	0.5	0.0	-	0.2	-	0.0	0.0	0.0	-	0.0	-	0.0	1.1	0.0	-	0.9	-	0.0	0.6	0.0	-	0.6	0.6
Single-Unit Trucks	0	3	0	0	-	3	0	0	0	2	-	2	0	1	7	1	-	9	0	1	15	0	-	16	30
% Single-Unit Trucks	-	2.2	0.0	0.0	-	0.7	-	0.0	0.0	6.7	-	0.8	-	0.9	0.7	1.9	-	0.8	-	1.4	1.4	0.0	-	1.3	1.0
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	9	0	-	9	0	0	8	0	-	8	17
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.9	0.0	-	0.8	-	0.0	0.7	0.0	-	0.7	0.6
Bicycles on Road	0	0	1	0	-	1	0	0	2	0	-	2	0	0	0	0	-	0	0	0	0	0	-	0	3
% Bicycles on Road	-	0.0	0.5	0.0	-	0.2	-	0.0	1.4	0.0	-	0.8	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.1
Pedestrians	-	-	-	-	24	-	-	-	-	5	-	-	-	-	-	-	4	-	-	-	-	-	14	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



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Count Name: Harlem Avenue with Randolph Street  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Peak Hour Data (12:45 PM)

Start Time	Randolph Street Eastbound						Randolph Street Westbound						Harlem Avenue Northbound						Harlem Avenue Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
12:45 PM	0	24	31	8	1	63	0	16	35	7	0	58	0	21	276	10	0	307	0	17	254	22	1	293	721
1:00 PM	0	33	28	18	1	79	0	13	17	4	1	34	0	19	285	8	2	312	0	16	269	19	3	304	729
1:15 PM	0	31	28	15	1	74	0	13	32	3	1	48	0	15	269	10	2	294	0	21	294	23	0	338	754
1:30 PM	0	29	32	15	4	76	0	7	21	11	2	39	0	29	247	6	3	282	0	13	239	23	6	275	672
Total	0	117	119	56	7	292	0	49	105	25	4	179	0	84	1077	34	7	1195	0	67	1056	87	10	1210	2876
Approach %	0.0	40.1	40.8	19.2	-	-	0.0	27.4	58.7	14.0	-	-	0.0	7.0	90.1	2.8	-	-	0.0	5.5	87.3	7.2	-	-	-
Total %	0.0	4.1	4.1	1.9	-	10.2	0.0	1.7	3.7	0.9	-	6.2	0.0	2.9	37.4	1.2	-	41.6	0.0	2.3	36.7	3.0	-	42.1	-
PHF	0.000	0.886	0.930	0.778	-	0.924	0.000	0.766	0.750	0.568	-	0.772	0.000	0.724	0.945	0.850	-	0.958	0.000	0.798	0.898	0.946	-	0.895	0.954
Lights	0	116	119	54	-	289	0	49	104	25	-	178	0	84	1060	34	-	1178	0	67	1041	87	-	1195	2840
% Lights	-	99.1	100.0	96.4	-	99.0	-	100.0	99.0	100.0	-	99.4	-	100.0	98.4	100.0	-	98.6	-	100.0	98.6	100.0	-	98.8	98.7
Buses	0	1	0	1	-	2	0	0	0	0	-	0	0	0	6	0	-	6	0	0	5	0	-	5	13
% Buses	-	0.9	0.0	1.8	-	0.7	-	0.0	0.0	0.0	-	0.0	-	0.0	0.6	0.0	-	0.5	-	0.0	0.5	0.0	-	0.4	0.5
Single-Unit Trucks	0	0	0	1	-	1	0	0	0	0	-	0	0	0	9	0	-	9	0	0	9	0	-	9	19
% Single-Unit Trucks	-	0.0	0.0	1.8	-	0.3	-	0.0	0.0	0.0	-	0.0	-	0.0	0.8	0.0	-	0.8	-	0.0	0.9	0.0	-	0.7	0.7
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	2	0	-	2	0	0	1	0	-	1	3
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.2	0.0	-	0.2	-	0.0	0.1	0.0	-	0.1	0.1
Bicycles on Road	0	0	0	0	-	0	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	1
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	1.0	0.0	-	0.6	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	-	7	-	-	-	-	-	4	-	-	-	-	-	7	-	-	-	-	-	10	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-



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Count Name: Harlem Avenue with South Boulevard  
Site Code:  
Start Date: 09/10/2020  
Page No: 1

### Turning Movement Data

Start Time	South Boulevard Eastbound							South Boulevard Westbound							Harlem Avenue Northbound							Harlem Avenue Southbound							Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left			
7:00 AM	0	1	0	3	8	4	0	15	7	11	3	33	0	0	181	4	9	185	0	0	255	8	2	263	485				
7:15 AM	0	0	0	5	23	5	0	25	14	10	6	49	0	0	228	1	13	229	0	0	265	16	3	281	564				
7:30 AM	0	0	0	1	11	1	0	21	8	18	5	47	0	1	239	3	6	243	0	0	272	20	5	292	583				
7:45 AM	0	0	1	4	20	5	0	14	12	17	5	43	0	0	301	1	14	302	0	0	293	16	1	309	659				
Hourly Total	0	1	1	13	62	15	0	75	41	56	19	172	0	1	949	9	42	959	0	0	1085	60	11	1145	2291				
8:00 AM	0	1	0	4	9	5	0	16	6	18	4	40	0	2	262	9	6	273	0	1	281	20	0	302	620				
8:15 AM	0	0	1	3	16	4	0	10	9	15	8	34	0	0	229	6	13	235	0	0	260	10	0	270	543				
8:30 AM	0	0	1	7	11	8	0	14	5	22	2	41	0	0	265	0	13	265	0	0	244	12	0	256	570				
8:45 AM	0	0	0	6	14	6	0	13	10	16	7	39	0	0	258	5	7	263	0	0	232	7	1	239	547				
Hourly Total	0	1	2	20	50	23	0	53	30	71	21	154	0	2	1014	20	39	1036	0	1	1017	49	1	1067	2280				
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-				
4:00 PM	0	0	0	6	16	6	0	16	23	29	5	68	0	0	254	5	16	259	0	0	264	27	2	291	624				
4:15 PM	0	0	0	3	7	3	0	18	25	33	1	76	0	0	271	7	13	278	0	0	291	31	1	322	679				
4:30 PM	0	0	0	8	13	8	0	17	26	26	0	69	0	1	293	9	10	303	0	0	289	19	1	308	688				
4:45 PM	0	0	0	4	7	4	0	23	24	30	3	77	0	0	262	8	5	270	0	0	260	39	2	299	650				
Hourly Total	0	0	0	21	43	21	0	74	98	118	9	290	0	1	1080	29	44	1110	0	0	1104	116	6	1220	2641				
5:00 PM	0	0	0	10	9	10	0	15	23	35	6	73	0	0	249	11	8	260	0	0	266	38	0	304	647				
5:15 PM	0	0	0	9	15	9	0	15	30	25	6	70	0	0	262	6	16	268	0	1	267	33	0	301	648				
5:30 PM	0	0	0	5	9	5	0	20	22	25	7	67	0	0	260	6	9	266	0	0	245	26	0	271	609				
5:45 PM	0	0	0	5	12	5	0	23	22	25	10	70	0	0	269	7	13	276	0	1	295	20	1	316	667				
Hourly Total	0	0	0	29	45	29	0	73	97	110	29	280	0	0	1040	30	46	1070	0	2	1073	117	1	1192	2571				
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-					
11:30 AM	0	2	0	3	7	5	0	15	17	23	1	55	0	0	310	9	5	319	0	0	274	19	0	293	672				
11:45 AM	0	0	0	6	7	6	0	20	17	34	2	71	0	0	268	8	13	276	0	0	284	14	0	298	651				
Hourly Total	0	2	0	9	14	11	0	35	34	57	3	126	0	0	578	17	18	595	0	0	558	33	0	591	1323				
12:00 PM	0	0	0	2	0	2	0	14	13	23	3	50	0	3	283	6	12	292	0	0	289	24	0	313	657				
12:15 PM	0	0	0	7	8	7	0	14	27	24	1	65	0	1	266	6	4	273	0	0	274	25	0	299	644				
12:30 PM	0	0	0	5	5	5	0	16	16	37	10	69	0	0	275	4	15	279	0	0	254	29	0	283	636				
12:45 PM	0	0	0	3	9	3	0	14	10	35	5	59	0	0	268	11	14	279	0	0	278	25	1	303	644				
Hourly Total	0	0	0	17	22	17	0	58	66	119	19	243	0	4	1092	27	45	1123	0	0	1095	103	1	1198	2581				
1:00 PM	0	0	0	6	7	6	0	16	13	25	9	54	0	0	286	12	11	298	0	1	299	25	4	325	683				
1:15 PM	0	0	0	5	12	5	0	19	18	27	5	64	0	0	265	8	11	273	0	0	294	19	0	313	655				
1:30 PM	0	1	1	9	12	11	0	13	11	34	5	58	0	0	280	11	20	291	0	0	275	32	0	307	667				
1:45 PM	0	0	0	3	7	3	0	19	14	33	2	66	0	0	255	5	10	260	0	1	292	23	1	316	645				
Hourly Total	0	1	1	23	38	25	0	67	56	119	21	242	0	0	1086	36	52	1122	0	2	1160	99	5	1261	2650				

Grand Total	0	5	4	132	274	141	0	435	422	650	121	1507	0	8	6839	168	286	7015	0	5	7092	577	25	7674	16337	
Approach %	0.0	3.5	2.8	93.6	-	-	0.0	28.9	28.0	43.1	-	-	0.0	0.1	97.5	2.4	-	-	0.0	0.1	92.4	7.5	-	-	-	
Total %	0.0	0.0	0.0	0.8	-	0.9	0.0	2.7	2.6	4.0	-	9.2	0.0	0.0	41.9	1.0	-	42.9	0.0	0.0	43.4	3.5	-	47.0	-	
Lights	0	4	1	128	-	133	0	430	412	597	-	1439	0	7	6596	167	-	6770	0	5	6905	568	-	7478	15820	
% Lights	-	80.0	25.0	97.0	-	94.3	-	98.9	97.6	91.8	-	95.5	-	87.5	96.4	99.4	-	96.5	-	100.0	97.4	98.4	-	97.4	96.8	
Buses	0	0	0	0	-	0	0	0	0	48	-	48	0	0	57	0	-	57	0	0	55	0	-	55	160	
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	7.4	-	3.2	-	0.0	0.8	0.0	-	0.8	-	0.0	0.8	0.0	-	0.7	1.0	
Single-Unit Trucks	0	0	0	3	-	3	0	5	4	5	-	14	0	1	126	1	-	128	0	0	79	6	-	85	230	
% Single-Unit Trucks	-	0.0	0.0	2.3	-	2.1	-	1.1	0.9	0.8	-	0.9	-	12.5	1.8	0.6	-	1.8	-	0.0	1.1	1.0	-	1.1	1.4	
Articulated Trucks	0	0	0	1	-	1	0	0	0	0	-	0	0	0	60	0	-	60	0	0	53	1	-	54	115	
% Articulated Trucks	-	0.0	0.0	0.8	-	0.7	-	0.0	0.0	0.0	-	0.0	-	0.0	0.9	0.0	-	0.9	-	0.0	0.7	0.2	-	0.7	0.7	
Bicycles on Road	0	1	3	0	-	4	0	0	6	0	-	6	0	0	0	0	-	0	0	0	0	-	2	-	2	12
% Bicycles on Road	-	20.0	75.0	0.0	-	2.8	-	0.0	1.4	0.0	-	0.4	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.3	-	0.0	0.1	
Pedestrians	-	-	-	-	-	274	-	-	-	-	-	121	-	-	-	-	-	286	-	-	-	-	-	25	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



Kenig, Lindgren, O'Hara, Aboona, Inc.  
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(847)518-9990

Count Name: Harlem Avenue with South Boulevard  
Site Code:  
Start Date: 09/10/2020  
Page No: 3

### Turning Movement Peak Hour Data (7:15 AM)

Start Time	South Boulevard Eastbound						South Boulevard Westbound						Harlem Avenue Northbound						Harlem Avenue Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:15 AM	0	0	0	5	23	5	0	25	14	10	6	49	0	0	228	1	13	229	0	0	265	16	3	281	564
7:30 AM	0	0	0	1	11	1	0	21	8	18	5	47	0	1	239	3	6	243	0	0	272	20	5	292	583
7:45 AM	0	0	1	4	20	5	0	14	12	17	5	43	0	0	301	1	14	302	0	0	293	16	1	309	659
8:00 AM	0	1	0	4	9	5	0	16	6	18	4	40	0	2	262	9	6	273	0	1	281	20	0	302	620
Total	0	1	1	14	63	16	0	76	40	63	20	179	0	3	1030	14	39	1047	0	1	1111	72	9	1184	2426
Approach %	0.0	6.3	6.3	87.5	-	-	0.0	42.5	22.3	35.2	-	-	0.0	0.3	98.4	1.3	-	-	0.0	0.1	93.8	6.1	-	-	-
Total %	0.0	0.0	0.0	0.6	-	0.7	0.0	3.1	1.6	2.6	-	7.4	0.0	0.1	42.5	0.6	-	43.2	0.0	0.0	45.8	3.0	-	48.8	-
PHF	0.000	0.250	0.250	0.700	-	0.800	0.000	0.760	0.714	0.875	-	0.913	0.000	0.375	0.855	0.389	-	0.867	0.000	0.250	0.948	0.900	-	0.958	0.920
Lights	0	1	0	12	-	13	0	74	36	53	-	163	0	2	959	13	-	974	0	1	1077	70	-	1148	2298
% Lights	-	100.0	0.0	85.7	-	81.3	-	97.4	90.0	84.1	-	91.1	-	66.7	93.1	92.9	-	93.0	-	100.0	96.9	97.2	-	97.0	94.7
Buses	0	0	0	0	-	0	0	0	0	9	-	9	0	0	10	0	-	10	0	0	8	0	-	8	27
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	14.3	-	5.0	-	0.0	1.0	0.0	-	1.0	-	0.0	0.7	0.0	-	0.7	1.1
Single-Unit Trucks	0	0	0	1	-	1	0	2	2	1	-	5	0	1	45	1	-	47	0	0	15	1	-	16	69
% Single-Unit Trucks	-	0.0	0.0	7.1	-	6.3	-	2.6	5.0	1.6	-	2.8	-	33.3	4.4	7.1	-	4.5	-	0.0	1.4	1.4	-	1.4	2.8
Articulated Trucks	0	0	0	1	-	1	0	0	0	0	-	0	0	0	16	0	-	16	0	0	11	0	-	11	28
% Articulated Trucks	-	0.0	0.0	7.1	-	6.3	-	0.0	0.0	0.0	-	0.0	-	0.0	1.6	0.0	-	1.5	-	0.0	1.0	0.0	-	0.9	1.2
Bicycles on Road	0	0	1	0	-	1	0	0	2	0	-	2	0	0	0	0	-	0	0	0	1	-	1	4	
% Bicycles on Road	-	0.0	100.0	0.0	-	6.3	-	0.0	5.0	0.0	-	1.1	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	1.4	-	0.1	0.2
Pedestrians	-	-	-	-	-	63	-	-	-	-	-	20	-	-	-	-	-	39	-	-	-	-	-	9	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-



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Count Name: Harlem Avenue with South Boulevard  
Site Code:  
Start Date: 09/10/2020  
Page No: 4

### Turning Movement Peak Hour Data (4:15 PM)

Start Time	South Boulevard Eastbound						South Boulevard Westbound						Harlem Avenue Northbound						Harlem Avenue Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
4:15 PM	0	0	0	3	7	3	0	18	25	33	1	76	0	0	271	7	13	278	0	0	291	31	1	322	679
4:30 PM	0	0	0	8	13	8	0	17	26	26	0	69	0	1	293	9	10	303	0	0	289	19	1	308	688
4:45 PM	0	0	0	4	7	4	0	23	24	30	3	77	0	0	262	8	5	270	0	0	260	39	2	299	650
5:00 PM	0	0	0	10	9	10	0	15	23	35	6	73	0	0	249	11	8	260	0	0	266	38	0	304	647
Total	0	0	0	25	36	25	0	73	98	124	10	295	0	1	1075	35	36	1111	0	0	1106	127	4	1233	2664
Approach %	0.0	0.0	0.0	100.0	-	-	0.0	24.7	33.2	42.0	-	-	0.0	0.1	96.8	3.2	-	-	0.0	0.0	89.7	10.3	-	-	-
Total %	0.0	0.0	0.0	0.9	-	0.9	0.0	2.7	3.7	4.7	-	11.1	0.0	0.0	40.4	1.3	-	41.7	0.0	0.0	41.5	4.8	-	46.3	-
PHF	0.000	0.000	0.000	0.625	-	0.625	0.000	0.793	0.942	0.886	-	0.958	0.000	0.250	0.917	0.795	-	0.917	0.000	0.000	0.950	0.814	-	0.957	0.968
Lights	0	0	0	24	-	24	0	73	97	117	-	287	0	1	1040	35	-	1076	0	0	1074	126	-	1200	2587
% Lights	-	-	-	96.0	-	96.0	-	100.0	99.0	94.4	-	97.3	-	100.0	96.7	100.0	-	96.8	-	-	97.1	99.2	-	97.3	97.1
Buses	0	0	0	0	-	0	0	0	0	7	-	7	0	0	8	0	-	8	0	0	9	0	-	9	24
% Buses	-	-	-	0.0	-	0.0	-	0.0	0.0	5.6	-	2.4	-	0.0	0.7	0.0	-	0.7	-	-	0.8	0.0	-	0.7	0.9
Single-Unit Trucks	0	0	0	1	-	1	0	0	1	0	-	1	0	0	17	0	-	17	0	0	11	0	-	11	30
% Single-Unit Trucks	-	-	-	4.0	-	4.0	-	0.0	1.0	0.0	-	0.3	-	0.0	1.6	0.0	-	1.5	-	-	1.0	0.0	-	0.9	1.1
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	10	0	-	10	0	0	12	1	-	13	23
% Articulated Trucks	-	-	-	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.9	0.0	-	0.9	-	-	1.1	0.8	-	1.1	0.9
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	-	-	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	36	-	-	-	-	10	-	-	-	-	-	-	36	-	-	-	-	-	4	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



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Count Name: Harlem Avenue with South Boulevard  
Site Code:  
Start Date: 09/10/2020  
Page No: 5

### Turning Movement Peak Hour Data (12:45 PM)

Start Time	South Boulevard Eastbound						South Boulevard Westbound						Harlem Avenue Northbound						Harlem Avenue Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
12:45 PM	0	0	0	3	9	3	0	14	10	35	5	59	0	0	268	11	14	279	0	0	278	25	1	303	644
1:00 PM	0	0	0	6	7	6	0	16	13	25	9	54	0	0	286	12	11	298	0	1	299	25	4	325	683
1:15 PM	0	0	0	5	12	5	0	19	18	27	5	64	0	0	265	8	11	273	0	0	294	19	0	313	655
1:30 PM	0	1	1	9	12	11	0	13	11	34	5	58	0	0	280	11	20	291	0	0	275	32	0	307	667
Total	0	1	1	23	40	25	0	62	52	121	24	235	0	0	1099	42	56	1141	0	1	1146	101	5	1248	2649
Approach %	0.0	4.0	4.0	92.0	-	-	0.0	26.4	22.1	51.5	-	-	0.0	0.0	96.3	3.7	-	-	0.0	0.1	91.8	8.1	-	-	-
Total %	0.0	0.0	0.0	0.9	-	0.9	0.0	2.3	2.0	4.6	-	8.9	0.0	0.0	41.5	1.6	-	43.1	0.0	0.0	43.3	3.8	-	47.1	-
PHF	0.000	0.250	0.250	0.639	-	0.568	0.000	0.816	0.722	0.864	-	0.918	0.000	0.000	0.961	0.875	-	0.957	0.000	0.250	0.958	0.789	-	0.960	0.970
Lights	0	1	1	23	-	25	0	61	51	116	-	228	0	0	1082	42	-	1124	0	1	1135	101	-	1237	2614
% Lights	-	100.0	100.0	100.0	-	100.0	-	98.4	98.1	95.9	-	97.0	-	-	98.5	100.0	-	98.5	-	100.0	99.0	100.0	-	99.1	98.7
Buses	0	0	0	0	-	0	0	0	0	5	-	5	0	0	6	0	-	6	0	0	6	0	-	6	17
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	4.1	-	2.1	-	-	0.5	0.0	-	0.5	-	0.0	0.5	0.0	-	0.5	0.6
Single-Unit Trucks	0	0	0	0	-	0	0	1	0	0	-	1	0	0	7	0	-	7	0	0	4	0	-	4	12
% Single-Unit Trucks	-	0.0	0.0	0.0	-	0.0	-	1.6	0.0	0.0	-	0.4	-	-	0.6	0.0	-	0.6	-	0.0	0.3	0.0	-	0.3	0.5
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	4	0	-	4	0	0	1	0	-	1	5
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	-	0.4	0.0	-	0.4	-	0.0	0.1	0.0	-	0.1	0.2
Bicycles on Road	0	0	0	0	-	0	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	-	0	-	1
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	1.9	0.0	-	0.4	-	-	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	-	40	-	-	-	-	-	24	-	-	-	-	-	56	-	-	-	-	-	5	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-



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Count Name: Maple Street with Pleasant Street  
Site Code:  
Start Date: 09/10/2020  
Page No: 1

### Turning Movement Data

Start Time	Pleasant Street Eastbound						Pleasant Street Westbound						Maple Street Northbound						Maple Street Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:00 AM	0	1	3	2	0	6	0	1	4	1	6	0	0	4	0	1	0	5	0	0	0	2	0	2	19
7:15 AM	0	0	10	1	0	11	0	1	3	0	4	0	0	0	1	0	1	1	0	0	0	0	0	0	16
7:30 AM	0	0	9	2	8	11	0	2	2	0	4	0	2	2	3	1	7	0	1	1	0	2	0	2	24
7:45 AM	0	1	8	2	3	11	0	0	3	0	3	0	3	1	3	4	7	0	0	0	1	1	1	2	23
Hourly Total	0	2	30	7	11	39	0	4	12	1	17	0	9	4	7	6	20	0	1	4	1	6	1	6	82
8:00 AM	1	4	15	4	6	24	0	2	3	0	5	0	2	0	2	1	4	0	1	1	1	1	3	3	36
8:15 AM	0	4	6	3	1	13	0	3	0	1	4	0	0	0	1	2	1	0	1	2	1	1	4	22	
8:30 AM	0	0	14	3	4	17	0	0	4	0	4	0	0	0	2	2	2	0	0	1	0	1	1	24	
8:45 AM	0	0	7	1	3	8	0	3	4	0	7	0	2	1	3	4	6	0	0	0	0	0	0	21	
Hourly Total	1	8	42	11	14	62	0	8	11	1	20	0	4	1	8	9	13	0	2	4	2	8	8	103	
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
4:00 PM	0	0	12	2	5	14	0	6	2	3	11	0	1	1	3	4	5	0	0	1	2	3	3	33	
4:15 PM	2	2	10	0	3	14	0	2	4	1	7	1	1	0	1	2	3	0	1	1	0	2	2	26	
4:30 PM	0	0	19	1	0	20	0	7	7	1	15	0	2	1	4	2	7	0	0	0	3	3	3	45	
4:45 PM	0	0	17	1	4	18	0	6	2	0	8	0	0	1	6	2	7	0	0	0	1	1	2	35	
Hourly Total	2	2	58	4	12	66	0	21	15	5	41	1	4	3	14	10	22	0	1	3	6	10	10	139	
5:00 PM	0	0	12	3	5	15	0	3	10	5	18	0	4	0	6	1	10	0	2	2	4	8	8	51	
5:15 PM	1	3	19	4	2	27	0	6	3	0	9	0	2	2	12	2	16	0	1	4	1	6	6	58	
5:30 PM	0	1	12	0	1	13	0	8	3	0	11	1	3	1	5	1	10	0	1	2	0	3	3	37	
5:45 PM	0	1	13	1	3	15	0	1	3	0	4	0	1	0	3	1	4	0	0	1	0	1	1	24	
Hourly Total	1	5	56	8	11	70	0	18	19	5	42	1	10	3	26	5	40	0	4	9	5	18	18	170	
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
11:30 AM	0	0	14	3	3	17	0	4	4	0	8	0	1	0	1	1	2	0	1	2	1	4	31		
11:45 AM	0	1	13	2	2	16	0	2	2	1	5	2	2	1	1	1	6	0	0	1	0	1	28		
Hourly Total	0	1	27	5	5	33	0	6	6	1	13	2	3	1	2	2	8	0	1	3	1	5	5	59	
12:00 PM	1	0	11	2	1	14	0	4	5	1	10	0	3	0	3	7	6	0	0	1	0	1	1	31	
12:15 PM	0	1	17	2	0	20	0	0	3	0	3	0	5	0	7	1	12	0	0	1	0	1	1	36	
12:30 PM	1	0	13	1	2	15	0	3	5	1	9	0	2	0	4	0	6	0	1	1	0	2	2	32	
12:45 PM	0	2	13	0	2	15	0	6	4	5	15	0	1	1	5	1	7	0	3	1	3	7	44		
Hourly Total	2	3	54	5	5	64	0	13	17	7	37	0	11	1	19	9	31	0	4	4	3	11	11	143	
1:00 PM	1	0	20	2	1	23	0	2	3	1	6	0	1	0	2	0	3	0	1	4	0	5	37		
1:15 PM	1	1	19	3	0	24	0	4	5	5	14	0	4	1	4	1	9	0	2	1	3	6	53		
1:30 PM	1	1	13	0	2	15	0	5	2	1	8	0	1	1	6	0	8	0	0	0	2	2	33		
1:45 PM	1	1	14	2	5	18	0	1	4	0	5	0	3	1	1	2	5	0	1	0	0	1	29		
Hourly Total	4	3	66	7	8	80	0	12	14	7	33	0	9	3	13	3	25	0	4	5	5	14	152		

Grand Total	10	24	333	47	66	414	0	82	94	27	203	4	50	16	89	44	159	0	17	32	23	72	848
Approach %	2.4	5.8	80.4	11.4	-	-	0.0	40.4	46.3	13.3	-	2.5	31.4	10.1	56.0	-	-	0.0	23.6	44.4	31.9	-	-
Total %	1.2	2.8	39.3	5.5	-	48.8	0.0	9.7	11.1	3.2	23.9	0.5	5.9	1.9	10.5	-	18.8	0.0	2.0	3.8	2.7	8.5	-
Lights	10	24	328	46	-	408	0	82	92	25	199	3	47	13	82	-	145	0	16	29	21	66	818
% Lights	100.0	100.0	98.5	97.9	-	98.6	-	100.0	97.9	92.6	98.0	75.0	94.0	81.3	92.1	-	91.2	-	94.1	90.6	91.3	91.7	96.5
Buses	0	0	1	0	-	1	0	0	0	0	0	0	0	0	1	-	1	0	0	0	0	0	2
% Buses	0.0	0.0	0.3	0.0	-	0.2	-	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.1	-	0.6	-	0.0	0.0	0.0	0.0	0.2
Single-Unit Trucks	0	0	3	0	-	3	0	0	1	2	3	1	2	1	5	-	9	0	1	2	2	5	20
% Single-Unit Trucks	0.0	0.0	0.9	0.0	-	0.7	-	0.0	1.1	7.4	1.5	25.0	4.0	6.3	5.6	-	5.7	-	5.9	6.3	8.7	6.9	2.4
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	0	0
% Articulated Trucks	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0	0.0
Bicycles on Road	0	0	1	1	-	2	0	0	1	0	1	0	1	2	1	-	4	0	0	1	0	1	8
% Bicycles on Road	0.0	0.0	0.3	2.1	-	0.5	-	0.0	1.1	0.0	0.5	0.0	2.0	12.5	1.1	-	2.5	-	0.0	3.1	0.0	1.4	0.9
Pedestrians	-	-	-	-	-	66	-	-	-	-	-	-	-	-	-	-	44	-	-	-	-	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-



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Count Name: Maple Street with Pleasant Street  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Peak Hour Data (7:15 AM)

Start Time	Pleasant Street Eastbound						Pleasant Street Westbound						Maple Street Northbound						Maple Street Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	App. Total	U-Turn	Left	
7:15 AM	0	0	10	1	0	11	0	1	3	0	4	0	0	1	0	1	1	0	0	0	0	0	0	16	
7:30 AM	0	0	9	2	8	11	0	2	2	0	4	0	2	2	3	1	7	0	1	1	0	2	24		
7:45 AM	0	1	8	2	3	11	0	0	3	0	3	0	3	1	3	4	7	0	0	1	1	2	23		
8:00 AM	1	4	15	4	6	24	0	2	3	0	5	0	2	0	2	1	4	0	1	1	1	3	36		
Total	1	5	42	9	17	57	0	5	11	0	16	0	7	4	8	7	19	0	2	3	2	7	99		
Approach %	1.8	8.8	73.7	15.8	-	-	0.0	31.3	68.8	0.0	-	0.0	36.8	21.1	42.1	-	-	0.0	28.6	42.9	28.6	-	-		
Total %	1.0	5.1	42.4	9.1	-	57.6	0.0	5.1	11.1	0.0	16.2	0.0	7.1	4.0	8.1	-	19.2	0.0	2.0	3.0	2.0	7.1	-		
PHF	0.250	0.313	0.700	0.563	-	0.594	0.000	0.625	0.917	0.000	0.800	0.000	0.583	0.500	0.667	-	0.679	0.000	0.500	0.750	0.500	0.583	0.688		
Lights	1	5	42	9	-	57	0	5	11	0	16	0	7	3	7	-	17	0	2	2	1	5	95		
% Lights	100.0	100.0	100.0	100.0	-	100.0	-	100.0	100.0	-	100.0	-	100.0	75.0	87.5	-	89.5	-	100.0	66.7	50.0	71.4	96.0		
Buses	0	0	0	0	-	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	0	0		
% Buses	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0	0.0		
Single-Unit Trucks	0	0	0	0	-	0	0	0	0	0	0	0	0	1	1	-	2	0	0	0	1	1	3		
% Single-Unit Trucks	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	25.0	12.5	-	10.5	-	0.0	0.0	50.0	14.3	3.0		
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	0	0		
% Articulated Trucks	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0	0.0		
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	0	0	0	0	0	-	0	0	0	1	0	1	1		
% Bicycles on Road	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	33.3	0.0	14.3	1.0		
Pedestrians	-	-	-	-	-	17	-	-	-	-	-	-	-	-	-	-	7	-	-	-	-	-	-		
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-		



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Count Name: Maple Street with Pleasant Street  
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Start Date: 09/10/2020  
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### Turning Movement Peak Hour Data (4:15 PM)

Start Time	Pleasant Street Eastbound						Pleasant Street Westbound						Maple Street Northbound						Maple Street Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	App. Total	U-Turn	Left	
4:15 PM	2	2	10	0	3	14	0	2	4	1	7	1	1	0	1	2	3	0	1	1	0	2	26		
4:30 PM	0	0	19	1	0	20	0	7	7	1	15	0	2	1	4	2	7	0	0	0	0	3	3	45	
4:45 PM	0	0	17	1	4	18	0	6	2	0	8	0	0	1	6	2	7	0	0	0	1	1	2	35	
5:00 PM	0	0	12	3	5	15	0	3	10	5	18	0	4	0	6	1	10	0	2	2	4	8	51		
Total	2	2	58	5	12	67	0	18	23	7	48	1	7	2	17	7	27	0	3	4	8	15	157		
Approach %	3.0	3.0	86.6	7.5	-	-	0.0	37.5	47.9	14.6	-	3.7	25.9	7.4	63.0	-	-	0.0	20.0	26.7	53.3	-	-		
Total %	1.3	1.3	36.9	3.2	-	42.7	0.0	11.5	14.6	4.5	30.6	0.6	4.5	1.3	10.8	-	17.2	0.0	1.9	2.5	5.1	9.6	-		
PHF	0.250	0.250	0.763	0.417	-	0.838	0.000	0.643	0.575	0.350	0.667	0.250	0.438	0.500	0.708	-	0.675	0.000	0.375	0.500	0.500	0.469	0.770		
Lights	2	2	57	5	-	66	0	18	23	7	48	0	7	1	15	-	23	0	3	4	7	14	151		
% Lights	100.0	100.0	98.3	100.0	-	98.5	-	100.0	100.0	100.0	100.0	0.0	100.0	50.0	88.2	-	85.2	-	100.0	100.0	87.5	93.3	96.2		
Buses	0	0	0	0	-	0	0	0	0	0	0	0	0	0	1	-	1	0	0	0	0	0	1		
% Buses	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0	0.0	0.0	0.0	5.9	-	3.7	-	0.0	0.0	0.0	0.0	0.0	0.6	
Single-Unit Trucks	0	0	0	0	-	0	0	0	0	0	0	1	0	0	1	-	2	0	0	0	1	1	3		
% Single-Unit Trucks	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0	100.0	0.0	0.0	5.9	-	7.4	-	0.0	0.0	0.0	12.5	6.7	1.9	
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	0	0		
% Articulated Trucks	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0	0.0		
Bicycles on Road	0	0	1	0	-	1	0	0	0	0	0	0	0	1	0	-	1	0	0	0	0	0	2		
% Bicycles on Road	0.0	0.0	1.7	0.0	-	1.5	-	0.0	0.0	0.0	0.0	0.0	0.0	50.0	0.0	-	3.7	-	0.0	0.0	0.0	0.0	0.0	1.3	
Pedestrians	-	-	-	-	-	12	-	-	-	-	-	-	-	-	-	-	7	-	-	-	-	-	-		
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-		



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Count Name: Maple Street with Pleasant Street  
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Start Date: 09/10/2020  
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## Turning Movement Peak Hour Data (12:45 PM)

Start Time	Pleasant Street Eastbound						Pleasant Street Westbound					Maple Street Northbound					Maple Street Southbound					Int. Total	
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	App. Total	
12:45 PM	0	2	13	0	2	15	0	6	4	5	15	0	1	1	5	1	7	0	3	1	3	7	44
1:00 PM	1	0	20	2	1	23	0	2	3	1	6	0	1	0	2	0	3	0	1	4	0	5	37
1:15 PM	1	1	19	3	0	24	0	4	5	5	14	0	4	1	4	1	9	0	2	1	3	6	53
1:30 PM	1	1	13	0	2	15	0	5	2	1	8	0	1	1	6	0	8	0	0	0	2	2	33
Total	3	4	65	5	5	77	0	17	14	12	43	0	7	3	17	2	27	0	6	6	8	20	167
Approach %	3.9	5.2	84.4	6.5	-	-	0.0	39.5	32.6	27.9	-	0.0	25.9	11.1	63.0	-	-	0.0	30.0	30.0	40.0	-	-
Total %	1.8	2.4	38.9	3.0	-	46.1	0.0	10.2	8.4	7.2	25.7	0.0	4.2	1.8	10.2	-	16.2	0.0	3.6	3.6	4.8	12.0	-
PHF	0.750	0.500	0.813	0.417	-	0.802	0.000	0.708	0.700	0.600	0.717	0.000	0.438	0.750	0.708	-	0.750	0.000	0.500	0.375	0.667	0.714	0.788
Lights	3	4	64	5	-	76	0	17	13	10	40	0	7	3	17	-	27	0	5	5	8	18	161
% Lights	100.0	100.0	98.5	100.0	-	98.7	-	100.0	92.9	83.3	93.0	-	100.0	100.0	100.0	-	100.0	-	83.3	83.3	100.0	90.0	96.4
Buses	0	0	1	0	-	1	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	0	1
% Buses	0.0	0.0	1.5	0.0	-	1.3	-	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0	0.6
Single-Unit Trucks	0	0	0	0	-	0	0	0	1	2	3	0	0	0	0	-	0	0	1	1	0	2	5
% Single-Unit Trucks	0.0	0.0	0.0	0.0	-	0.0	-	0.0	7.1	16.7	7.0	-	0.0	0.0	0.0	-	0.0	-	16.7	16.7	0.0	10.0	3.0
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	0	0
% Articulated Trucks	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	0	0
% Bicycles on Road	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0	0.0
Pedestrians	-	-	-	-	-	5	-	-	-	-	-	-	-	-	-	-	2	-	-	-	-	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-



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Count Name: Marion Street with Pleasant Street  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Data

Start Time	Pleasant Street Eastbound						Pleasant Street Westbound						Marion Street Northbound						Marion Street Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:00 AM	0	3	0	1	7	4	0	1	0	0	6	1	0	1	9	1	1	11	0	0	10	1	11	27	
7:15 AM	0	9	0	0	3	9	0	1	0	0	7	1	0	0	4	2	7	6	0	0	13	1	14	30	
7:30 AM	0	8	3	4	3	15	0	0	2	0	5	2	0	0	9	0	3	9	0	1	18	3	22	48	
7:45 AM	0	9	0	2	9	11	0	0	1	2	13	3	0	0	16	0	8	16	0	2	14	1	17	47	
Hourly Total	0	29	3	7	22	39	0	2	3	2	31	7	0	1	38	3	19	42	0	3	55	6	64	152	
8:00 AM	0	10	2	3	6	15	0	0	1	1	17	2	0	1	17	3	3	21	0	0	13	0	13	51	
8:15 AM	0	3	0	5	3	8	0	1	0	0	12	1	0	2	14	0	4	16	0	3	17	1	21	46	
8:30 AM	0	12	3	2	6	17	0	1	0	3	7	4	0	2	8	1	4	11	0	1	15	0	16	48	
8:45 AM	0	5	1	1	6	7	0	2	1	4	5	7	0	1	16	2	2	19	0	2	15	2	19	52	
Hourly Total	0	30	6	11	21	47	0	4	2	8	41	14	0	6	55	6	13	67	0	6	60	3	69	197	
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
4:00 PM	0	9	1	3	8	13	0	1	2	1	12	4	0	2	24	2	6	28	0	5	36	11	52	97	
4:15 PM	0	7	1	6	11	14	0	3	0	3	9	6	0	3	24	3	2	30	0	4	28	2	34	84	
4:30 PM	0	19	1	2	6	22	0	2	2	5	12	9	0	4	30	2	4	36	0	3	42	7	52	119	
4:45 PM	0	16	4	9	10	29	0	3	2	4	16	9	0	3	33	1	2	37	0	5	34	8	47	122	
Hourly Total	0	51	7	20	35	78	0	9	6	13	49	28	0	12	111	8	14	131	0	17	140	28	185	422	
5:00 PM	0	13	2	9	10	24	0	0	2	11	9	13	0	4	39	5	5	48	0	4	51	7	62	147	
5:15 PM	0	20	2	5	8	27	0	0	0	6	10	6	0	1	31	0	7	32	0	4	48	6	58	123	
5:30 PM	0	12	1	5	9	18	0	2	2	5	4	9	0	0	30	0	4	30	0	6	46	6	58	115	
5:45 PM	0	9	2	4	7	15	0	3	1	9	10	13	0	0	24	3	3	27	0	6	46	2	54	109	
Hourly Total	0	54	7	23	34	84	0	5	5	31	33	41	0	5	124	8	19	137	0	20	191	21	232	494	
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
11:30 AM	0	8	4	2	6	14	0	1	1	4	6	6	0	2	26	6	4	34	1	4	35	3	43	97	
11:45 AM	0	10	3	2	7	15	0	7	2	10	13	19	0	0	32	2	2	34	0	5	31	1	37	105	
Hourly Total	0	18	7	4	13	29	0	8	3	14	19	25	0	2	58	8	6	68	1	9	66	4	80	202	
12:00 PM	0	11	2	2	6	15	0	7	1	5	4	13	0	2	19	0	2	21	0	8	38	5	51	100	
12:15 PM	0	17	4	4	16	25	0	7	2	7	9	16	0	2	16	3	5	21	0	0	37	4	41	103	
12:30 PM	0	12	2	8	17	22	0	4	0	3	10	7	0	3	17	2	2	22	0	8	26	5	39	90	
12:45 PM	0	14	5	3	5	22	0	2	2	8	9	12	0	2	22	0	2	24	0	3	37	8	48	106	
Hourly Total	0	54	13	17	44	84	0	20	5	23	32	48	0	9	74	5	11	88	0	19	138	22	179	399	
1:00 PM	0	18	3	2	14	23	0	3	2	7	10	12	0	2	17	3	4	22	0	0	29	4	33	90	
1:15 PM	0	16	5	2	16	23	0	4	4	1	9	9	1	2	24	3	7	30	0	6	22	5	33	95	
1:30 PM	0	10	1	10	14	21	0	4	0	1	9	5	0	0	18	0	4	18	0	5	34	5	44	88	
1:45 PM	0	6	4	5	5	15	0	1	1	0	13	2	0	3	33	1	4	37	0	3	28	2	33	87	
Hourly Total	0	50	13	19	49	82	0	12	7	9	41	28	1	7	92	7	19	107	0	14	113	16	143	360	

Grand Total	0	286	56	101	218	443	0	60	31	100	246	191	1	42	552	45	101	640	1	88	763	100	952	2226
Approach %	0.0	64.6	12.6	22.8	-	-	0.0	31.4	16.2	52.4	-	-	0.2	6.6	86.3	7.0	-	-	0.1	9.2	80.1	10.5	-	-
Total %	0.0	12.8	2.5	4.5	-	19.9	0.0	2.7	1.4	4.5	-	8.6	0.0	1.9	24.8	2.0	-	28.8	0.0	4.0	34.3	4.5	42.8	-
Lights	0	280	56	95	-	431	0	58	27	92	-	177	1	41	537	42	-	621	1	79	750	94	924	2153
% Lights	-	97.9	100.0	94.1	-	97.3	-	96.7	87.1	92.0	-	92.7	100.0	97.6	97.3	93.3	-	97.0	100.0	89.8	98.3	94.0	97.1	96.7
Buses	0	0	0	2	-	2	0	0	0	0	-	0	0	0	1	0	-	1	0	0	0	0	0	3
% Buses	-	0.0	0.0	2.0	-	0.5	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.2	0.0	-	0.2	0.0	0.0	0.0	0.0	0.0	0.1
Single-Unit Trucks	0	5	0	2	-	7	0	1	1	1	-	3	0	1	5	0	-	6	0	3	3	1	7	23
% Single-Unit Trucks	-	1.7	0.0	2.0	-	1.6	-	1.7	3.2	1.0	-	1.6	0.0	2.4	0.9	0.0	-	0.9	0.0	3.4	0.4	1.0	0.7	1.0
Articulated Trucks	0	0	0	1	-	1	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	0	1
% Articulated Trucks	-	0.0	0.0	1.0	-	0.2	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Bicycles on Road	0	1	0	1	-	2	0	1	3	7	-	11	0	0	9	3	-	12	0	6	10	5	21	46
% Bicycles on Road	-	0.3	0.0	1.0	-	0.5	-	1.7	9.7	7.0	-	5.8	0.0	0.0	1.6	6.7	-	1.9	0.0	6.8	1.3	5.0	2.2	2.1
Pedestrians	-	-	-	-	218	-	-	-	-	-	246	-	-	-	-	-	101	-	-	-	-	-	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	-



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Count Name: Marion Street with Pleasant Street  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Peak Hour Data (7:15 AM)

Start Time	Pleasant Street Eastbound						Pleasant Street Westbound						Marion Street Northbound						Marion Street Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:15 AM	0	9	0	0	3	9	0	1	0	0	7	1	0	0	4	2	7	6	0	0	13	1	14	30	
7:30 AM	0	8	3	4	3	15	0	0	2	0	5	2	0	0	9	0	3	9	0	1	18	3	22	48	
7:45 AM	0	9	0	2	9	11	0	0	1	2	13	3	0	0	16	0	8	16	0	2	14	1	17	47	
8:00 AM	0	10	2	3	6	15	0	0	1	1	17	2	0	1	17	3	3	21	0	0	13	0	13	51	
Total	0	36	5	9	21	50	0	1	4	3	42	8	0	1	46	5	21	52	0	3	58	5	66	176	
Approach %	0.0	72.0	10.0	18.0	-	-	0.0	12.5	50.0	37.5	-	-	0.0	1.9	88.5	9.6	-	-	0.0	4.5	87.9	7.6	-	-	
Total %	0.0	20.5	2.8	5.1	-	28.4	0.0	0.6	2.3	1.7	-	4.5	0.0	0.6	26.1	2.8	-	29.5	0.0	1.7	33.0	2.8	37.5	-	
PHF	0.000	0.900	0.417	0.563	-	0.833	0.000	0.250	0.500	0.375	-	0.667	0.000	0.250	0.676	0.417	-	0.619	0.000	0.375	0.806	0.417	0.750	0.863	
Lights	0	35	5	9	-	49	0	1	4	2	-	7	0	1	42	5	-	48	0	1	56	5	62	166	
% Lights	-	97.2	100.0	100.0	-	98.0	-	100.0	100.0	66.7	-	87.5	-	100.0	91.3	100.0	-	92.3	-	33.3	96.6	100.0	93.9	94.3	
Buses	0	0	0	0	-	0	0	0	0	-	0	0	0	0	0	0	-	0	0	0	0	0	0		
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0		
Single-Unit Trucks	0	1	0	0	-	1	0	0	0	0	-	0	0	0	1	0	-	1	0	2	0	0	2		
% Single-Unit Trucks	-	2.8	0.0	0.0	-	2.0	-	0.0	0.0	0.0	-	0.0	-	0.0	2.2	0.0	-	1.9	-	66.7	0.0	0.0	3.0	2.3	
Articulated Trucks	0	0	0	0	-	0	0	0	0	-	0	0	0	0	0	0	-	0	0	0	0	0	0		
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	0.0		
Bicycles on Road	0	0	0	0	-	0	0	0	1	-	1	0	0	3	0	-	3	0	0	2	0	2	6		
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	33.3	-	12.5	-	0.0	6.5	0.0	-	5.8	-	0.0	3.4	0.0	3.0	3.4	
Pedestrians	-	-	-	-	-	21	-	-	-	-	42	-	-	-	-	-	-	21	-	-	-	-	-		
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	-	-	100.0	-	-	-	-	-		



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Count Name: Marion Street with Pleasant Street  
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Start Date: 09/10/2020  
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## Turning Movement Peak Hour Data (4:15 PM)



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Count Name: Marion Street with Pleasant Street  
Site Code:  
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## Turning Movement Peak Hour Data (12:45 PM)



Kenig Lindgren O'Hara Aboona, Inc.  
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(847)518-9990

Count Name: Marion Street with Randolph  
Street  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Data

Start Time	Randolph Street Eastbound						Randolph Street Westbound						Marion Street Northbound						Marion Street Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:00 AM	0	2	17	0	2	19	0	0	16	2	4	18	0	3	7	1	4	11	0	1	7	6	4	14	62
7:15 AM	0	3	25	2	1	30	0	1	22	0	5	23	0	2	2	0	4	4	0	1	7	8	2	16	73
7:30 AM	0	3	20	5	4	28	0	2	26	2	3	30	0	3	5	3	7	11	0	0	12	5	2	17	86
7:45 AM	0	7	29	6	4	42	0	2	21	4	10	27	0	4	11	0	6	15	0	2	7	3	3	12	96
Hourly Total	0	15	91	13	11	119	0	5	85	8	22	98	0	12	25	4	21	41	0	4	33	22	11	59	317
8:00 AM	0	5	26	3	5	34	0	0	21	3	11	24	0	4	12	0	4	16	0	1	6	11	4	18	92
8:15 AM	0	6	20	2	5	28	0	0	22	2	9	24	0	0	6	1	4	7	0	3	10	6	5	19	78
8:30 AM	0	7	20	4	8	31	0	1	16	2	10	19	0	0	4	1	3	5	0	1	9	5	8	15	70
8:45 AM	0	11	27	6	3	44	0	0	16	1	5	17	0	1	7	1	2	9	0	2	11	6	1	19	89
Hourly Total	0	29	93	15	21	137	0	1	75	8	35	84	0	5	29	3	13	37	0	7	36	28	18	71	329
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
4:00 PM	0	11	54	5	3	70	0	0	21	6	7	27	0	7	14	3	8	24	0	5	16	17	7	38	159
4:15 PM	0	14	49	9	2	72	0	5	31	5	6	41	0	5	14	1	3	20	0	8	21	5	7	34	167
4:30 PM	0	18	51	10	4	79	0	0	33	3	5	36	0	8	14	2	3	24	0	8	30	10	4	48	187
4:45 PM	0	14	70	10	3	94	0	1	41	4	8	46	1	7	20	3	1	31	0	5	20	20	0	45	216
Hourly Total	0	57	224	34	12	315	0	6	126	18	26	150	1	27	62	9	15	99	0	26	87	52	18	165	729
5:00 PM	0	16	58	8	2	82	0	3	34	4	4	41	0	4	17	1	4	22	0	11	31	21	3	63	208
5:15 PM	0	15	46	7	3	68	0	0	30	6	2	36	0	3	16	1	1	20	0	8	38	11	5	57	181
5:30 PM	0	11	57	8	4	76	0	1	33	2	3	36	0	9	15	4	2	28	0	4	25	19	1	48	188
5:45 PM	0	7	51	10	4	68	0	2	24	2	3	28	0	5	13	0	2	18	0	7	28	19	4	54	168
Hourly Total	0	49	212	33	13	294	0	6	121	14	12	141	0	21	61	6	9	88	0	30	122	70	13	222	745
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
11:30 AM	0	13	27	4	3	44	0	0	25	5	1	30	0	4	16	0	0	20	0	1	23	12	2	36	130
11:45 AM	0	10	46	3	2	59	0	1	22	4	3	27	0	4	19	2	2	25	0	8	16	9	3	33	144
Hourly Total	0	23	73	7	5	103	0	1	47	9	4	57	0	8	35	2	2	45	0	9	39	21	5	69	274
12:00 PM	0	11	31	4	5	46	0	2	42	5	0	49	0	3	8	0	0	11	0	6	28	17	4	51	157
12:15 PM	0	7	38	4	3	49	0	2	22	2	5	26	0	4	16	1	3	21	0	6	24	14	3	44	140
12:30 PM	0	6	34	3	4	43	0	0	19	2	6	21	0	2	9	1	5	12	0	4	18	17	1	39	115
12:45 PM	0	13	39	3	1	55	0	1	32	1	2	34	0	3	15	4	1	22	0	9	21	9	3	39	150
Hourly Total	0	37	142	14	13	193	0	5	115	10	13	130	0	12	48	6	9	66	0	25	91	57	11	173	562
1:00 PM	0	10	34	6	4	50	0	1	17	1	8	19	0	0	10	1	2	11	0	10	15	9	2	34	114
1:15 PM	0	13	38	5	1	56	0	2	22	0	6	24	0	4	19	3	0	26	0	6	14	8	2	28	134
1:30 PM	0	9	33	4	5	46	0	2	24	2	5	28	0	2	15	3	3	20	0	10	24	10	9	44	138
1:45 PM	0	12	33	3	1	48	0	1	20	5	4	26	0	2	19	1	4	22	0	4	18	12	2	34	130
Hourly Total	0	44	138	18	11	200	0	6	83	8	23	97	0	8	63	8	9	79	0	30	71	39	15	140	516

Grand Total	0	254	973	134	86	1361	0	30	652	75	135	757	1	93	323	38	78	455	0	131	479	289	91	899	3472
Approach %	0.0	18.7	71.5	9.8	-	-	0.0	4.0	86.1	9.9	-	-	0.2	20.4	71.0	8.4	-	-	0.0	14.6	53.3	32.1	-	-	-
Total %	0.0	7.3	28.0	3.9	-	39.2	0.0	0.9	18.8	2.2	-	21.8	0.0	2.7	9.3	1.1	-	13.1	0.0	3.8	13.8	8.3	-	25.9	-
Lights	0	253	961	132	-	1346	0	27	635	70	-	732	1	92	315	38	-	446	0	127	469	284	-	880	3404
% Lights	-	99.6	98.8	98.5	-	98.9	-	90.0	97.4	93.3	-	96.7	100.0	98.9	97.5	100.0	-	98.0	-	96.9	97.9	98.3	-	97.9	98.0
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	2	0	-	2	0	0	1	0	-	1	3
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.6	0.0	-	0.4	-	0.0	0.2	0.0	-	0.1	0.1
Single-Unit Trucks	0	0	3	1	-	4	0	1	3	0	-	4	0	0	3	0	-	3	0	3	1	2	-	6	17
% Single-Unit Trucks	-	0.0	0.3	0.7	-	0.3	-	3.3	0.5	0.0	-	0.5	0.0	0.0	0.9	0.0	-	0.7	-	2.3	0.2	0.7	-	0.7	0.5
Articulated Trucks	0	0	1	0	-	1	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	-	0	0	2
% Articulated Trucks	-	0.0	0.1	0.0	-	0.1	-	0.0	0.2	0.0	-	0.1	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.1
Bicycles on Road	0	1	8	1	-	10	0	2	13	5	-	20	0	1	3	0	-	4	0	1	8	3	-	12	46
% Bicycles on Road	-	0.4	0.8	0.7	-	0.7	-	6.7	2.0	6.7	-	2.6	0.0	1.1	0.9	0.0	-	0.9	-	0.8	1.7	1.0	-	1.3	1.3
Pedestrians	-	-	-	-	-	86	-	-	-	-	-	135	-	-	-	-	-	78	-	-	-	-	-	91	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-



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### Turning Movement Peak Hour Data (7:15 AM)

Start Time	Randolph Street Eastbound						Randolph Street Westbound						Marion Street Northbound						Marion Street Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:15 AM	0	3	25	2	1	30	0	1	22	0	5	23	0	2	2	0	4	4	0	1	7	8	2	16	73
7:30 AM	0	3	20	5	4	28	0	2	26	2	3	30	0	3	5	3	7	11	0	0	12	5	2	17	86
7:45 AM	0	7	29	6	4	42	0	2	21	4	10	27	0	4	11	0	6	15	0	2	7	3	3	12	96
8:00 AM	0	5	26	3	5	34	0	0	21	3	11	24	0	4	12	0	4	16	0	1	6	11	4	18	92
Total	0	18	100	16	14	134	0	5	90	9	29	104	0	13	30	3	21	46	0	4	32	27	11	63	347
Approach %	0.0	13.4	74.6	11.9	-	-	0.0	4.8	86.5	8.7	-	-	0.0	28.3	65.2	6.5	-	-	0.0	6.3	50.8	42.9	-	-	-
Total %	0.0	5.2	28.8	4.6	-	38.6	0.0	1.4	25.9	2.6	-	30.0	0.0	3.7	8.6	0.9	-	13.3	0.0	1.2	9.2	7.8	-	18.2	-
PHF	0.000	0.643	0.862	0.667	-	0.798	0.000	0.625	0.865	0.563	-	0.867	0.000	0.813	0.625	0.250	-	0.719	0.000	0.500	0.667	0.614	-	0.875	0.904
Lights	0	17	97	16	-	130	0	4	84	8	-	96	0	13	30	3	-	46	0	4	30	27	-	61	333
% Lights	-	94.4	97.0	100.0	-	97.0	-	80.0	93.3	88.9	-	92.3	-	100.0	100.0	100.0	-	100.0	-	100.0	93.8	100.0	-	96.8	96.0
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	
Single-Unit Trucks	0	0	1	0	-	1	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	
% Single-Unit Trucks	-	0.0	1.0	0.0	-	0.7	-	0.0	1.1	0.0	-	1.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	
Bicycles on Road	0	1	2	0	-	3	0	1	5	1	-	7	0	0	0	0	-	0	0	0	2	0	-	2	
% Bicycles on Road	-	5.6	2.0	0.0	-	2.2	-	20.0	5.6	11.1	-	6.7	-	0.0	0.0	0.0	-	0.0	-	0.0	6.3	0.0	-	3.2	
Pedestrians	-	-	-	-	14	-	-	-	-	-	29	-	-	-	-	-	21	-	-	-	-	11	-	-	
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	



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(847)518-9990

Count Name: Marion Street with Randolph Street  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Peak Hour Data (4:15 PM)

Start Time	Randolph Street Eastbound						Randolph Street Westbound						Marion Street Northbound						Marion Street Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
4:15 PM	0	14	49	9	2	72	0	5	31	5	6	41	0	5	14	1	3	20	0	8	21	5	7	34	167
4:30 PM	0	18	51	10	4	79	0	0	33	3	5	36	0	8	14	2	3	24	0	8	30	10	4	48	187
4:45 PM	0	14	70	10	3	94	0	1	41	4	8	46	1	7	20	3	1	31	0	5	20	20	0	45	216
5:00 PM	0	16	58	8	2	82	0	3	34	4	4	41	0	4	17	1	4	22	0	11	31	21	3	63	208
Total	0	62	228	37	11	327	0	9	139	16	23	164	1	24	65	7	11	97	0	32	102	56	14	190	778
Approach %	0.0	19.0	69.7	11.3	-	-	0.0	5.5	84.8	9.8	-	-	1.0	24.7	67.0	7.2	-	-	0.0	16.8	53.7	29.5	-	-	-
Total %	0.0	8.0	29.3	4.8	-	42.0	0.0	1.2	17.9	2.1	-	21.1	0.1	3.1	8.4	0.9	-	12.5	0.0	4.1	13.1	7.2	-	24.4	-
PHF	0.000	0.861	0.814	0.925	-	0.870	0.000	0.450	0.848	0.800	-	0.891	0.250	0.750	0.813	0.583	-	0.782	0.000	0.727	0.823	0.667	-	0.754	0.900
Lights	0	62	227	36	-	325	0	8	136	13	-	157	1	23	62	7	-	93	0	31	99	55	-	185	760
% Lights	-	100.0	99.6	97.3	-	99.4	-	88.9	97.8	81.3	-	95.7	100.0	95.8	95.4	100.0	-	95.9	-	96.9	97.1	98.2	-	97.4	97.7
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	1	0	-	1	1
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	1.0	0.0	-	0.5	0.1
Single-Unit Trucks	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	1
% Single-Unit Trucks	-	0.0	0.4	0.0	-	0.3	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.1
Articulated Trucks	0	0	0	0	-	0	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	1
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.7	0.0	-	0.6	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.1
Bicycles on Road	0	0	0	1	-	1	0	1	2	3	-	6	0	1	3	0	-	4	0	1	2	1	-	4	15
% Bicycles on Road	-	0.0	0.0	2.7	-	0.3	-	11.1	1.4	18.8	-	3.7	0.0	4.2	4.6	0.0	-	4.1	-	3.1	2.0	1.8	-	2.1	1.9
Pedestrians	-	-	-	-	-	11	-	-	-	-	-	23	-	-	-	-	-	11	-	-	-	-	-	14	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-



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Count Name: Marion Street with Randolph  
Street  
Site Code:  
Start Date: 09/10/2020  
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## Turning Movement Peak Hour Data (12:45 PM)

Start Time	Randolph Street Eastbound						Randolph Street Westbound						Marion Street Northbound						Marion Street Southbound						Int. Total	
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total		
12:45 PM	0	13	39	3	1	55	0	1	32	1	2	34	0	3	15	4	1	22	0	9	21	9	3	39	150	
1:00 PM	0	10	34	6	4	50	0	1	17	1	8	19	0	0	10	1	2	11	0	10	15	9	2	34	114	
1:15 PM	0	13	38	5	1	56	0	2	22	0	6	24	0	4	19	3	0	26	0	6	14	8	2	28	134	
1:30 PM	0	9	33	4	5	46	0	2	24	2	5	28	0	2	15	3	3	20	0	10	24	10	9	44	138	
Total	0	45	144	18	11	207	0	6	95	4	21	105	0	9	59	11	6	79	0	35	74	36	16	145	536	
Approach %	0.0	21.7	69.6	8.7	-	-	0.0	5.7	90.5	3.8	-	-	0.0	11.4	74.7	13.9	-	-	0.0	24.1	51.0	24.8	-	-	-	
Total %	0.0	8.4	26.9	3.4	-	38.6	0.0	1.1	17.7	0.7	-	19.6	0.0	1.7	11.0	2.1	-	14.7	0.0	6.5	13.8	6.7	-	27.1	-	
PHF	0.000	0.865	0.923	0.750	-	0.924	0.000	0.750	0.742	0.500	-	0.772	0.000	0.563	0.776	0.688	-	0.760	0.000	0.875	0.771	0.900	-	0.824	0.893	
Lights	0	45	143	18	-	206	0	6	94	4	-	104	0	9	58	11	-	78	0	34	73	36	-	143	531	
% Lights	-	100.0	99.3	100.0	-	99.5	-	100.0	98.9	100.0	-	99.0	-	100.0	98.3	100.0	-	98.7	-	97.1	98.6	100.0	-	98.6	99.1	
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	1	0	-	1	0	0	0	0	-	0	1	
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	1.7	0.0	-	1.3	-	0.0	0.0	0.0	-	0.0	0.2	
Single-Unit Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	1	0	0	-	1	1	
% Single-Unit Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	2.9	0.0	0.0	-	0.7	0.2	
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0	
Bicycles on Road	0	0	1	0	-	1	0	0	1	0	-	1	0	0	0	0	-	0	0	0	1	0	-	1	3	
% Bicycles on Road	-	0.0	0.7	0.0	-	0.5	-	0.0	1.1	0.0	-	1.0	-	0.0	0.0	0.0	-	0.0	-	0.0	1.4	0.0	-	0.7	0.6	
Pedestrians	-	-	-	-	-	11	-	-	-	-	-	21	-	-	-	-	-	-	-	-	-	-	-	16	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-



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Count Name: Marion Street with South Boulevard  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Data

Start Time	South Boulevard Eastbound						South Boulevard Westbound						Marion Street Northbound						Marion Street Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:00 AM	0	2	0	2	6	4	0	2	24	1	3	27	0	1	8	2	1	11	0	1	7	3	1	11	53
7:15 AM	0	0	0	0	8	0	0	1	30	4	9	35	0	2	9	0	2	11	0	1	12	6	1	19	65
7:30 AM	0	0	0	2	4	2	0	8	37	3	3	48	0	2	13	4	1	19	0	2	10	3	1	15	84
7:45 AM	0	0	0	2	11	2	0	3	32	10	8	45	0	1	23	2	5	26	0	1	11	5	1	17	90
Hourly Total	0	2	0	6	29	8	0	14	123	18	23	155	0	6	53	8	9	67	0	5	40	17	4	62	292
8:00 AM	0	1	4	2	13	7	0	2	29	12	11	43	1	2	26	2	4	31	0	2	8	6	1	16	97
8:15 AM	0	1	6	0	9	7	0	4	28	5	8	37	0	3	10	3	6	16	0	1	15	7	0	23	83
8:30 AM	0	1	0	1	3	2	0	3	25	12	9	40	0	1	17	2	4	20	0	3	16	3	1	22	84
8:45 AM	0	2	2	2	9	6	0	8	31	15	7	54	0	2	15	1	3	18	0	1	13	7	0	21	99
Hourly Total	0	5	12	5	34	22	0	17	113	44	35	174	1	8	68	8	17	85	0	7	52	23	2	82	363
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
4:00 PM	0	0	6	2	9	8	0	12	58	19	5	89	0	4	31	2	12	37	0	4	41	5	3	50	184
4:15 PM	0	0	3	1	12	4	0	8	65	15	7	88	0	2	29	4	7	35	0	6	24	12	0	42	169
4:30 PM	0	4	3	3	11	10	0	13	46	13	7	72	0	9	34	6	4	49	0	3	38	11	0	52	183
4:45 PM	0	4	4	1	11	9	0	8	64	21	12	93	0	10	33	10	10	53	0	5	36	15	2	56	211
Hourly Total	0	8	16	7	43	31	0	41	233	68	31	342	0	25	127	22	33	174	0	18	139	43	5	200	747
5:00 PM	0	6	4	6	20	16	0	7	59	23	4	89	0	8	46	4	11	58	0	3	47	11	1	61	224
5:15 PM	0	2	5	1	13	8	0	6	51	22	9	79	0	6	48	5	11	59	0	1	48	20	2	69	215
5:30 PM	0	3	4	0	5	7	0	16	53	18	3	87	0	4	49	1	4	54	0	4	38	14	2	56	204
5:45 PM	0	3	6	3	14	12	0	9	66	19	5	94	0	8	28	0	11	36	0	6	41	17	0	64	206
Hourly Total	0	14	19	10	52	43	0	38	229	82	21	349	0	26	171	10	37	207	0	14	174	62	5	250	849
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
11:30 AM	0	2	3	1	7	6	0	7	50	18	4	75	0	5	36	3	7	44	0	2	31	9	0	42	167
11:45 AM	0	3	3	0	9	6	0	9	50	16	10	75	0	8	40	8	8	56	0	2	29	9	1	40	177
Hourly Total	0	5	6	1	16	12	0	16	100	34	14	150	0	13	76	11	15	100	0	4	60	18	1	82	344
12:00 PM	0	4	2	4	15	10	0	11	43	14	5	68	0	0	28	5	10	33	0	2	35	5	3	42	153
12:15 PM	0	1	1	1	19	3	0	9	50	22	11	81	0	2	32	4	10	38	0	2	33	9	2	44	166
12:30 PM	0	3	1	1	15	5	0	4	52	18	11	74	0	5	27	2	11	34	0	3	36	11	0	50	163
12:45 PM	0	3	4	2	16	9	0	10	51	18	10	79	0	3	31	5	5	39	0	5	30	7	0	42	169
Hourly Total	0	11	8	8	65	27	0	34	196	72	37	302	0	10	118	16	36	144	0	12	134	32	5	178	651
1:00 PM	0	4	4	2	9	10	0	5	48	17	12	70	0	7	35	2	5	44	0	4	24	9	0	37	161
1:15 PM	0	2	1	1	16	4	0	8	40	17	4	65	0	9	28	7	8	44	0	2	26	12	0	40	153
1:30 PM	0	3	4	6	18	13	0	7	61	10	10	78	0	3	22	2	9	27	0	2	32	5	1	39	157
1:45 PM	0	1	3	4	9	8	0	6	43	19	13	68	0	1	30	6	8	37	0	1	29	8	0	38	151
Hourly Total	0	10	12	13	52	35	0	26	192	63	39	281	0	20	115	17	30	152	0	9	111	34	1	154	622

Grand Total	0	55	73	50	291	178	0	186	1186	381	200	1753	1	108	728	92	177	929	0	69	710	229	23	1008	3868
Approach %	0.0	30.9	41.0	28.1	-	-	0.0	10.6	67.7	21.7	-	-	0.1	11.6	78.4	9.9	-	-	0.0	6.8	70.4	22.7	-	-	-
Total %	0.0	1.4	1.9	1.3	-	4.6	0.0	4.8	30.7	9.9	-	45.3	0.0	2.8	18.8	2.4	-	24.0	0.0	1.8	18.4	5.9	-	26.1	-
Lights	0	51	71	48	-	170	0	184	1122	375	-	1681	0	103	712	90	-	905	0	69	697	225	-	991	3747
% Lights	-	92.7	97.3	96.0	-	95.5	-	98.9	94.6	98.4	-	95.9	0.0	95.4	97.8	97.8	-	97.4	-	100.0	98.2	98.3	-	98.3	96.9
Buses	0	0	0	0	-	0	0	0	46	0	-	46	1	0	1	0	-	2	0	0	0	0	-	0	48
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	3.9	0.0	-	2.6	100.0	0.0	0.1	0.0	-	0.2	-	0.0	0.0	0.0	-	0.0	1.2
Single-Unit Trucks	0	0	1	2	-	3	0	2	11	1	-	14	0	3	3	2	-	8	0	0	2	1	-	3	28
% Single-Unit Trucks	-	0.0	1.4	4.0	-	1.7	-	1.1	0.9	0.3	-	0.8	0.0	2.8	0.4	2.2	-	0.9	-	0.0	0.3	0.4	-	0.3	0.7
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	0	
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	4	1	0	-	5	0	0	7	5	-	12	0	2	12	0	-	14	0	0	11	3	-	14	45
% Bicycles on Road	-	7.3	1.4	0.0	-	2.8	-	0.0	0.6	1.3	-	0.7	0.0	1.9	1.6	0.0	-	1.5	-	0.0	1.5	1.3	-	1.4	1.2
Pedestrians	-	-	-	-	-	291	-	-	-	-	-	200	-	-	-	-	-	177	-	-	-	-	-	23	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-



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Count Name: Marion Street with South Boulevard  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Peak Hour Data (7:15 AM)

Start Time	South Boulevard Eastbound						South Boulevard Westbound						Marion Street Northbound						Marion Street Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:15 AM	0	0	0	0	8	0	0	1	30	4	9	35	0	2	9	0	2	11	0	1	12	6	1	19	65
7:30 AM	0	0	0	2	4	2	0	8	37	3	3	48	0	2	13	4	1	19	0	2	10	3	1	15	84
7:45 AM	0	0	0	2	11	2	0	3	32	10	8	45	0	1	23	2	5	26	0	1	11	5	1	17	90
8:00 AM	0	1	4	2	13	7	0	2	29	12	11	43	1	2	26	2	4	31	0	2	8	6	1	16	97
Total	0	1	4	6	36	11	0	14	128	29	31	171	1	7	71	8	12	87	0	6	41	20	4	67	336
Approach %	0.0	9.1	36.4	54.5	-	-	0.0	8.2	74.9	17.0	-	-	1.1	8.0	81.6	9.2	-	-	0.0	9.0	61.2	29.9	-	-	-
Total %	0.0	0.3	1.2	1.8	-	3.3	0.0	4.2	38.1	8.6	-	50.9	0.3	2.1	21.1	2.4	-	25.9	0.0	1.8	12.2	6.0	-	19.9	-
PHF	0.000	0.250	0.250	0.750	-	0.393	0.000	0.438	0.865	0.604	-	0.891	0.250	0.875	0.683	0.500	-	0.702	0.000	0.750	0.854	0.833	-	0.882	0.866
Lights	0	1	4	5	-	10	0	14	114	29	-	157	0	6	67	8	-	81	0	6	39	19	-	64	312
% Lights	-	100.0	100.0	83.3	-	90.9	-	100.0	89.1	100.0	-	91.8	0.0	85.7	94.4	100.0	-	93.1	-	100.0	95.1	95.0	-	95.5	92.9
Buses	0	0	0	0	-	0	0	0	9	0	-	9	1	0	0	0	-	1	0	0	0	0	-	0	10
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	7.0	0.0	-	5.3	100.0	0.0	0.0	0.0	-	1.1	-	0.0	0.0	0.0	-	0.0	3.0
Single-Unit Trucks	0	0	0	1	-	1	0	0	3	0	-	3	0	1	1	0	-	2	0	0	0	1	-	1	7
% Single-Unit Trucks	-	0.0	0.0	16.7	-	9.1	-	0.0	2.3	0.0	-	1.8	0.0	14.3	1.4	0.0	-	2.3	-	0.0	0.0	5.0	-	1.5	2.1
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	2	0	-	2	0	0	3	0	-	3	0	0	2	0	-	2	7
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	1.6	0.0	-	1.2	0.0	0.0	4.2	0.0	-	3.4	-	0.0	4.9	0.0	-	3.0	2.1
Pedestrians	-	-	-	-	36	-	-	-	-	-	31	-	-	-	-	-	12	-	-	-	-	4	-	-	
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	



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Count Name: Marion Street with South Boulevard  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Peak Hour Data (4:15 PM)

Start Time	South Boulevard Eastbound						South Boulevard Westbound						Marion Street Northbound						Marion Street Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
4:15 PM	0	0	3	1	12	4	0	8	65	15	7	88	0	2	29	4	7	35	0	6	24	12	0	42	169
4:30 PM	0	4	3	3	11	10	0	13	46	13	7	72	0	9	34	6	4	49	0	3	38	11	0	52	183
4:45 PM	0	4	4	1	11	9	0	8	64	21	12	93	0	10	33	10	10	53	0	5	36	15	2	56	211
5:00 PM	0	6	4	6	20	16	0	7	59	23	4	89	0	8	46	4	11	58	0	3	47	11	1	61	224
Total	0	14	14	11	54	39	0	36	234	72	30	342	0	29	142	24	32	195	0	17	145	49	3	211	787
Approach %	0.0	35.9	35.9	28.2	-	-	0.0	10.5	68.4	21.1	-	-	0.0	14.9	72.8	12.3	-	-	0.0	8.1	68.7	23.2	-	-	-
Total %	0.0	1.8	1.8	1.4	-	5.0	0.0	4.6	29.7	9.1	-	43.5	0.0	3.7	18.0	3.0	-	24.8	0.0	2.2	18.4	6.2	-	26.8	-
PHF	0.000	0.583	0.875	0.458	-	0.609	0.000	0.692	0.900	0.783	-	0.919	0.000	0.725	0.772	0.600	-	0.841	0.000	0.708	0.771	0.817	-	0.865	0.878
Lights	0	14	14	11	-	39	0	36	224	71	-	331	0	29	139	24	-	192	0	17	143	49	-	209	771
% Lights	-	100.0	100.0	100.0	-	100.0	-	100.0	95.7	98.6	-	96.8	-	100.0	97.9	100.0	-	98.5	-	100.0	98.6	100.0	-	99.1	98.0
Buses	0	0	0	0	-	0	0	0	8	0	-	8	0	0	0	0	-	0	0	0	0	0	-	0	8
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	3.4	0.0	-	2.3	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	1.0
Single-Unit Trucks	0	0	0	0	-	0	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	1
% Single-Unit Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.4	0.0	-	0.3	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.1
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	1	1	-	2	0	0	3	0	-	3	0	0	2	0	-	2	7
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	0.4	1.4	-	0.6	-	0.0	2.1	0.0	-	1.5	-	0.0	1.4	0.0	-	0.9	0.9
Pedestrians	-	-	-	-	54	-	-	-	-	-	30	-	-	-	-	-	32	-	-	-	-	-	3	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



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Count Name: Marion Street with South Boulevard  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Peak Hour Data (12:45 PM)

Start Time	South Boulevard Eastbound						South Boulevard Westbound						Marion Street Northbound						Marion Street Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
12:45 PM	0	3	4	2	16	9	0	10	51	18	10	79	0	3	31	5	5	39	0	5	30	7	0	42	169
1:00 PM	0	4	4	2	9	10	0	5	48	17	12	70	0	7	35	2	5	44	0	4	24	9	0	37	161
1:15 PM	0	2	1	1	16	4	0	8	40	17	4	65	0	9	28	7	8	44	0	2	26	12	0	40	153
1:30 PM	0	3	4	6	18	13	0	7	61	10	10	78	0	3	22	2	9	27	0	2	32	5	1	39	157
Total	0	12	13	11	59	36	0	30	200	62	36	292	0	22	116	16	27	154	0	13	112	33	1	158	640
Approach %	0.0	33.3	36.1	30.6	-	-	0.0	10.3	68.5	21.2	-	-	0.0	14.3	75.3	10.4	-	-	0.0	8.2	70.9	20.9	-	-	-
Total %	0.0	1.9	2.0	1.7	-	5.6	0.0	4.7	31.3	9.7	-	45.6	0.0	3.4	18.1	2.5	-	24.1	0.0	2.0	17.5	5.2	-	24.7	-
PHF	0.000	0.750	0.813	0.458	-	0.692	0.000	0.750	0.820	0.861	-	0.924	0.000	0.611	0.829	0.571	-	0.875	0.000	0.650	0.875	0.688	-	0.940	0.947
Lights	0	12	13	11	-	36	0	29	191	61	-	281	0	22	116	15	-	153	0	13	111	32	-	156	626
% Lights	-	100.0	100.0	100.0	-	100.0	-	96.7	95.5	98.4	-	96.2	-	100.0	100.0	93.8	-	99.4	-	100.0	99.1	97.0	-	98.7	97.8
Buses	0	0	0	0	-	0	0	0	6	0	-	6	0	0	0	0	-	0	0	0	0	-	0	6	
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	3.0	0.0	-	2.1	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.9
Single-Unit Trucks	0	0	0	0	-	0	0	1	2	0	-	3	0	0	0	1	-	1	0	0	0	0	-	0	4
% Single-Unit Trucks	-	0.0	0.0	0.0	-	0.0	-	3.3	1.0	0.0	-	1.0	-	0.0	0.0	6.3	-	0.6	-	0.0	0.0	0.0	-	0.0	0.6
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	0	
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	1	1	-	2	0	0	0	0	-	0	0	0	1	1	-	2	4
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	0.5	1.6	-	0.7	-	0.0	0.0	0.0	-	0.0	-	0.0	0.9	3.0	-	1.3	0.6
Pedestrians	-	-	-	-	59	-	-	-	-	-	36	-	-	-	-	-	27	-	-	-	-	-	1	-	
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	



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Count Name: NS Alley and Randolph with Randolph Street  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Data

Start Time	Randolph Street Eastbound						Randolph Street Westbound						NS Alley Northbound						NS Alley Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:00 AM	0	0	22	0	0	22	0	0	20	0	0	20	0	0	0	0	4	0	0	0	2	3	2	44	
7:15 AM	0	0	27	0	1	27	0	0	35	0	0	35	0	0	0	0	3	0	0	1	0	1	5	2	64
7:30 AM	0	0	26	0	0	26	0	0	32	0	0	32	0	1	0	0	6	1	0	1	0	0	3	1	60
7:45 AM	0	1	41	1	0	43	0	0	32	0	0	32	0	1	0	0	4	1	0	1	0	1	6	2	78
Hourly Total	0	1	116	1	1	118	0	0	119	0	0	119	0	2	0	0	17	2	0	3	0	4	17	7	246
8:00 AM	0	1	35	1	0	37	0	0	34	0	0	34	0	0	0	0	5	0	0	2	0	1	4	3	74
8:15 AM	0	1	27	0	0	28	0	0	26	2	1	28	0	1	0	0	1	1	0	0	0	0	9	0	57
8:30 AM	0	1	30	0	1	31	0	0	22	0	1	22	0	0	0	0	5	0	0	0	0	0	6	0	53
8:45 AM	0	2	39	1	1	42	0	0	24	0	1	24	0	3	0	1	0	4	0	1	0	0	4	1	71
Hourly Total	0	5	131	2	2	138	0	0	106	2	3	108	0	4	0	1	11	5	0	3	0	1	23	4	255
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
4:00 PM	0	0	70	3	0	73	0	1	49	1	0	51	0	4	0	1	7	5	0	2	0	1	9	3	132
4:15 PM	0	2	73	1	0	76	0	0	35	1	0	36	0	0	0	1	4	1	0	0	0	1	3	1	114
4:30 PM	0	1	74	5	0	80	1	3	50	1	0	55	0	2	0	0	0	2	0	1	0	4	0	5	142
4:45 PM	0	0	85	1	1	86	0	0	62	1	1	63	0	1	0	1	3	2	0	3	0	4	2	7	158
Hourly Total	0	3	302	10	1	315	1	4	196	4	1	205	0	7	0	3	14	10	0	6	0	10	14	16	546
5:00 PM	0	1	86	0	0	87	0	1	61	2	2	64	0	3	0	1	0	4	0	1	0	1	1	2	157
5:15 PM	0	1	81	1	2	83	0	1	46	1	0	48	0	1	0	1	1	2	0	0	0	1	1	1	134
5:30 PM	0	2	70	1	0	73	0	2	57	2	0	61	0	1	0	0	0	1	0	1	0	0	2	1	136
5:45 PM	1	3	61	2	0	67	0	0	41	1	0	42	0	1	0	3	0	4	0	1	0	1	0	2	115
Hourly Total	1	7	298	4	2	310	0	4	205	6	2	215	0	6	0	5	1	11	0	3	0	3	4	6	542
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
11:30 AM	0	0	44	0	2	44	0	0	37	1	1	38	0	1	0	3	0	4	0	2	0	1	2	3	89
11:45 AM	0	2	58	0	0	60	0	0	38	1	0	39	0	0	0	1	1	1	0	0	0	0	4	0	100
Hourly Total	0	2	102	0	2	104	0	0	75	2	1	77	0	1	0	4	1	5	0	2	0	1	6	3	189
12:00 PM	0	1	41	1	0	43	0	0	58	0	3	58	0	0	0	0	0	0	0	0	0	0	2	2	103
12:15 PM	0	2	52	0	1	54	0	0	41	1	0	42	0	1	0	0	1	1	0	0	0	0	4	3	101
12:30 PM	0	1	48	1	0	50	0	0	36	0	1	36	0	0	0	0	5	0	0	0	0	0	3	0	86
12:45 PM	0	0	54	1	2	55	0	0	48	1	0	49	0	0	0	0	0	0	0	0	0	1	3	2	108
Hourly Total	0	4	195	3	3	202	0	0	183	2	4	185	0	1	0	0	6	1	0	0	1	9	15	10	398
1:00 PM	0	0	49	0	0	49	0	1	28	0	0	29	0	0	0	1	3	1	0	0	0	2	3	2	81
1:15 PM	0	0	52	1	1	53	0	0	34	0	1	34	0	1	0	0	1	1	0	0	0	2	1	2	90
1:30 PM	0	2	55	0	0	57	0	0	33	0	0	33	0	0	0	0	4	0	0	0	0	0	10	0	90
1:45 PM	0	0	43	1	0	44	0	0	35	0	0	35	0	1	0	1	5	2	0	0	0	0	6	0	81
Hourly Total	0	2	199	2	1	203	0	1	130	0	1	131	0	2	0	2	13	4	0	0	0	4	20	4	342

Grand Total	1	24	1343	22	12	1390	1	9	1014	16	12	1040	0	23	0	15	63	38	0	17	1	32	99	50	2518
Approach %	0.1	1.7	96.6	1.6	-	-	0.1	0.9	97.5	1.5	-	-	0.0	60.5	0.0	39.5	-	-	0.0	34.0	2.0	64.0	-	-	-
Total %	0.0	1.0	53.3	0.9	-	55.2	0.0	0.4	40.3	0.6	-	41.3	0.0	0.9	0.0	0.6	-	1.5	0.0	0.7	0.0	1.3	-	2.0	-
Lights	1	23	1325	22	-	1371	1	9	995	16	-	1021	0	23	0	14	-	37	0	17	1	32	-	50	2479
% Lights	100.0	95.8	98.7	100.0	-	98.6	100.0	100.0	98.1	100.0	-	98.2	-	100.0	-	93.3	-	97.4	-	100.0	100.0	100.0	-	100.0	98.5
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	0	0	0	0	-	0	0	
% Buses	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Single-Unit Trucks	0	0	7	0	-	7	0	0	6	0	-	6	0	0	0	1	-	1	0	0	0	0	-	0	14
% Single-Unit Trucks	0.0	0.0	0.5	0.0	-	0.5	0.0	0.0	0.6	0.0	-	0.6	-	0.0	-	6.7	-	2.6	-	0.0	0.0	0.0	-	0.0	0.6
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	0	0	0	0	-	0	0	
% Articulated Trucks	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	1	11	0	-	12	0	0	13	0	-	13	0	0	0	0	-	0	0	0	0	-	0	0	25
% Bicycles on Road	0.0	4.2	0.8	0.0	-	0.9	0.0	0.0	1.3	0.0	-	1.3	-	0.0	-	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	1.0
Pedestrians	-	-	-	-	-	12	-	-	-	-	-	12	-	-	-	-	-	63	-	-	-	-	-	99	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-



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Count Name: NS Alley and Randolph with Randolph Street  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Peak Hour Data (7:15 AM)

Start Time	Randolph Street Eastbound						Randolph Street Westbound						NS Alley Northbound						NS Alley Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:15 AM	0	0	27	0	1	27	0	0	35	0	0	35	0	0	0	0	3	0	0	1	0	1	5	2	64
7:30 AM	0	0	26	0	0	26	0	0	32	0	0	32	0	1	0	0	6	1	0	1	0	0	3	1	60
7:45 AM	0	1	41	1	0	43	0	0	32	0	0	32	0	1	0	0	4	1	0	1	0	1	6	2	78
8:00 AM	0	1	35	1	0	37	0	0	34	0	0	34	0	0	0	0	5	0	0	2	0	1	4	3	74
Total	0	2	129	2	1	133	0	0	133	0	0	133	0	2	0	0	18	2	0	5	0	3	18	8	276
Approach %	0.0	1.5	97.0	1.5	-	-	0.0	0.0	100.0	0.0	-	-	0.0	100.0	0.0	0.0	-	-	0.0	62.5	0.0	37.5	-	-	-
Total %	0.0	0.7	46.7	0.7	-	48.2	0.0	0.0	48.2	0.0	-	48.2	0.0	0.7	0.0	0.0	-	0.7	0.0	1.8	0.0	1.1	-	2.9	-
PHF	0.000	0.500	0.787	0.500	-	0.773	0.000	0.000	0.950	0.000	-	0.950	0.000	0.500	0.000	0.000	-	0.500	0.000	0.625	0.000	0.750	-	0.667	0.885
Lights	0	2	124	2	-	128	0	0	128	0	-	128	0	2	0	0	-	2	0	5	0	3	-	8	266
% Lights	-	100.0	96.1	100.0	-	96.2	-	-	96.2	-	-	96.2	-	100.0	-	-	-	100.0	-	100.0	-	100.0	-	100.0	96.4
Buses	0	0	0	0	-	0	0	0	0	-	0	0	0	0	0	0	-	0	0	0	0	0	-	0	
% Buses	-	0.0	0.0	0.0	-	0.0	-	-	0.0	-	-	0.0	-	0.0	-	-	-	0.0	-	0.0	-	0.0	-	0.0	0.0
Single-Unit Trucks	0	0	2	0	-	2	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	3
% Single-Unit Trucks	-	0.0	1.6	0.0	-	1.5	-	-	0.8	-	-	0.8	-	0.0	-	-	-	0.0	-	0.0	-	0.0	-	0.0	1.1
Articulated Trucks	0	0	0	0	-	0	0	0	0	-	0	0	0	0	0	0	-	0	0	0	0	0	-	0	
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	-	0.0	-	-	0.0	-	0.0	-	-	-	0.0	-	0.0	-	0.0	-	0.0	0.0
Bicycles on Road	0	0	3	0	-	3	0	0	4	0	-	4	0	0	0	0	-	0	0	0	0	0	-	0	7
% Bicycles on Road	-	0.0	2.3	0.0	-	2.3	-	-	3.0	-	-	3.0	-	0.0	-	-	-	0.0	-	0.0	-	0.0	-	0.0	2.5
Pedestrians	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	18	-	-	-	-	18	-	-	
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	



Kenig Lindgren O'Hara Aboona, Inc.  
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Count Name: NS Alley and Randolph with Randolph Street  
Site Code:  
Start Date: 09/10/2020  
Page No: 4

### Turning Movement Peak Hour Data (4:15 PM)

Start Time	Randolph Street Eastbound						Randolph Street Westbound						NS Alley Northbound						NS Alley Southbound						Int. Total	
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total		
4:15 PM	0	2	73	1	0	76	0	0	35	1	0	36	0	0	0	1	4	1	0	0	0	1	3	1	114	
4:30 PM	0	1	74	5	0	80	1	3	50	1	0	55	0	2	0	0	0	2	0	1	0	4	0	5	142	
4:45 PM	0	0	85	1	1	86	0	0	62	1	1	63	0	1	0	1	3	2	0	3	0	4	2	7	158	
5:00 PM	0	1	86	0	0	87	0	1	61	2	2	64	0	3	0	1	0	4	0	1	0	1	1	2	157	
Total	0	4	318	7	1	329	1	4	208	5	3	218	0	6	0	3	7	9	0	5	0	10	6	15	571	
Approach %	0.0	1.2	96.7	2.1	-	-	0.5	1.8	95.4	2.3	-	-	0.0	66.7	0.0	33.3	-	-	0.0	33.3	0.0	66.7	-	-	-	
Total %	0.0	0.7	55.7	1.2	-	57.6	0.2	0.7	36.4	0.9	-	38.2	0.0	1.1	0.0	0.5	-	1.6	0.0	0.9	0.0	1.8	-	2.6	-	
PHF	0.000	0.500	0.924	0.350	-	0.945	0.250	0.333	0.839	0.625	-	0.852	0.000	0.500	0.000	0.750	-	0.563	0.000	0.417	0.000	0.625	-	0.536	0.903	
Lights	0	3	314	7	-	324	1	4	203	5	-	213	0	6	0	3	-	9	0	5	0	10	-	15	561	
% Lights	-	75.0	98.7	100.0	-	98.5	100.0	100.0	97.6	100.0	-	97.7	-	100.0	-	100.0	-	100.0	-	100.0	-	100.0	-	100.0	98.2	
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	0	0	0	0	0	-	0	0	
% Buses	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	0.0	
Single-Unit Trucks	0	0	1	0	-	1	0	0	2	0	-	2	0	0	0	-	0	0	0	0	0	0	-	0	3	
% Single-Unit Trucks	-	0.0	0.3	0.0	-	0.3	0.0	0.0	1.0	0.0	-	0.9	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	0.5	
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	0	0	0	0	0	-	0	0	
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	0.0	
Bicycles on Road	0	1	3	0	-	4	0	0	3	0	-	3	0	0	0	-	0	0	0	0	0	0	-	0	7	
% Bicycles on Road	-	25.0	0.9	0.0	-	1.2	0.0	0.0	1.4	0.0	-	1.4	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	1.2	
Pedestrians	-	-	-	-	-	1	-	-	-	-	-	3	-	-	-	-	-	7	-	-	-	-	-	6	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



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Count Name: NS Alley and Randolph with Randolph Street  
Site Code:  
Start Date: 09/10/2020  
Page No: 5

### Turning Movement Peak Hour Data (12:45 PM)

Start Time	Randolph Street Eastbound						Randolph Street Westbound						NS Alley Northbound						NS Alley Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
12:45 PM	0	0	54	1	2	55	0	0	48	1	0	49	0	0	0	0	0	0	0	0	1	3	2	4	108
1:00 PM	0	0	49	0	0	49	0	1	28	0	0	29	0	0	0	1	3	1	0	0	0	2	3	2	81
1:15 PM	0	0	52	1	1	53	0	0	34	0	1	34	0	1	0	0	1	1	0	0	0	2	1	2	90
1:30 PM	0	2	55	0	0	57	0	0	33	0	0	33	0	0	0	0	4	0	0	0	0	0	10	0	90
Total	0	2	210	2	3	214	0	1	143	1	1	145	0	1	0	1	8	2	0	0	1	7	16	8	369
Approach %	0.0	0.9	98.1	0.9	-	-	0.0	0.7	98.6	0.7	-	-	0.0	50.0	0.0	50.0	-	-	0.0	0.0	12.5	87.5	-	-	-
Total %	0.0	0.5	56.9	0.5	-	58.0	0.0	0.3	38.8	0.3	-	39.3	0.0	0.3	0.0	0.3	-	0.5	0.0	0.0	0.3	1.9	-	2.2	-
PHF	0.000	0.250	0.955	0.500	-	0.939	0.000	0.250	0.745	0.250	-	0.740	0.000	0.250	0.000	0.250	-	0.500	0.000	0.000	0.250	0.583	-	0.500	0.854
Lights	0	2	208	2	-	212	0	1	142	1	-	144	0	1	0	1	-	2	0	0	1	7	-	8	366
% Lights	-	100.0	99.0	100.0	-	99.1	-	100.0	99.3	100.0	-	99.3	-	100.0	-	100.0	-	100.0	-	-	100.0	100.0	-	100.0	99.2
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	0	
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	-	0.0	-	-	-	0.0	0.0	-	0.0	0.0	
Single-Unit Trucks	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	-	0	0	0	0	0	-	0	1	
% Single-Unit Trucks	-	0.0	0.5	0.0	-	0.5	-	0.0	0.0	0.0	-	0.0	-	0.0	-	0.0	-	-	-	0.0	0.0	-	0.0	0.3	
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	0	0	0	0	-	0	0	
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	-	0.0	-	-	-	0.0	0.0	-	0.0	0.0	
Bicycles on Road	0	0	1	0	-	1	0	0	1	0	-	1	0	0	0	-	0	0	0	0	0	-	0	2	
% Bicycles on Road	-	0.0	0.5	0.0	-	0.5	-	0.0	0.7	0.0	-	0.7	-	0.0	-	0.0	-	0.0	-	0.0	0.0	-	0.0	0.5	
Pedestrians	-	-	-	-	3	-	-	-	-	-	1	-	-	-	-	8	-	-	-	-	-	16	-	-	
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	



Kenig Lindgren O'Hara Aboona, Inc.  
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Count Name: NS Alley with Pleasant Street  
Site Code:  
Start Date: 09/10/2020  
Page No: 1

### Turning Movement Data

Start Time	Pleasant Street Eastbound							Pleasant Street Westbound							NS Alley Northbound							NS Alley Southbound							Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left			
7:00 AM	0	0	4	0	3	4	0	0	4	0	0	4	0	1	0	0	3	1	0	0	0	0	1	0	0	9			
7:15 AM	0	0	6	1	1	7	0	0	1	0	0	1	0	1	0	0	4	1	0	0	0	0	1	0	0	9			
7:30 AM	0	0	15	0	0	15	0	0	4	0	0	4	0	0	0	2	4	2	0	0	0	0	0	0	0	21			
7:45 AM	0	0	11	0	1	11	0	0	3	0	0	3	0	0	0	0	4	0	0	0	0	0	0	0	0	14			
Hourly Total	0	0	36	1	5	37	0	0	12	0	0	12	0	2	0	2	15	4	0	0	0	0	0	2	0	53			
8:00 AM	0	1	15	0	0	16	0	0	2	0	0	2	0	2	0	0	5	2	0	0	0	1	0	1	0	21			
8:15 AM	0	1	9	0	0	10	0	0	3	0	0	3	0	0	0	0	1	0	0	0	0	2	3	2	0	15			
8:30 AM	0	1	14	0	3	15	0	0	0	0	1	0	0	3	0	2	6	5	0	0	0	0	0	0	0	20			
8:45 AM	0	2	9	0	0	11	0	0	4	0	2	4	0	1	0	0	3	1	0	0	0	1	1	1	0	17			
Hourly Total	0	5	47	0	3	52	0	0	9	0	3	9	0	6	0	2	15	8	0	0	0	4	4	4	4	73			
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-				
4:00 PM	0	2	12	0	2	14	0	2	9	1	0	12	0	0	0	1	3	1	0	1	0	0	0	1	0	28			
4:15 PM	0	0	10	0	1	10	0	0	6	1	0	7	0	0	0	1	8	1	0	0	0	3	0	3	0	21			
4:30 PM	1	0	23	0	1	24	0	1	12	0	0	13	0	0	1	1	2	2	0	0	0	1	0	1	0	40			
4:45 PM	0	2	20	0	1	22	0	2	8	4	0	14	0	0	0	3	1	3	0	3	0	1	0	4	0	43			
Hourly Total	1	4	65	0	5	70	0	5	35	6	0	46	0	0	1	6	14	7	0	4	0	5	0	9	0	132			
5:00 PM	0	1	21	1	3	23	1	2	12	0	0	15	0	2	0	2	2	4	0	2	0	2	0	4	0	46			
5:15 PM	1	4	26	1	3	32	0	0	6	1	1	7	0	0	0	2	9	2	0	1	0	1	3	2	0	43			
5:30 PM	2	0	15	1	2	18	1	0	6	0	1	7	0	1	0	0	4	1	0	0	1	4	0	5	0	31			
5:45 PM	0	2	15	0	0	17	0	0	3	0	2	3	0	0	0	1	2	1	0	0	0	1	0	1	0	22			
Hourly Total	3	7	77	3	8	90	2	2	27	1	4	32	0	3	0	5	17	8	0	3	1	8	3	12	0	142			
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-				
11:30 AM	0	0	13	0	0	13	0	0	4	0	5	4	0	0	0	1	10	1	0	1	0	2	0	3	0	21			
11:45 AM	0	0	14	0	1	14	0	1	5	0	0	6	0	0	0	0	6	0	0	1	0	1	1	2	0	22			
Hourly Total	0	0	27	0	1	27	0	1	9	0	5	10	0	0	0	1	16	1	0	2	0	3	1	5	0	43			
12:00 PM	0	1	12	1	1	14	0	0	8	1	4	9	0	2	1	1	4	4	0	0	0	0	1	0	0	27			
12:15 PM	1	0	20	0	0	21	0	0	3	2	0	5	0	0	0	3	2	3	0	2	0	0	0	2	0	31			
12:30 PM	0	0	18	0	3	18	0	1	9	0	3	10	0	0	0	1	3	1	0	1	0	1	0	2	0	31			
12:45 PM	0	0	23	0	0	23	0	0	11	0	2	11	0	0	0	1	3	1	0	1	0	1	1	2	0	37			
Hourly Total	1	1	73	1	4	76	0	1	31	3	9	35	0	2	1	6	12	9	0	4	0	2	2	6	0	126			
1:00 PM	0	2	21	0	0	23	1	2	6	0	0	9	0	1	0	1	2	4	0	1	0	0	0	1	0	35			
1:15 PM	0	0	23	1	1	24	0	0	11	1	0	12	0	1	0	0	3	1	0	0	0	1	2	1	0	38			
1:30 PM	1	1	19	0	4	21	0	0	5	1	2	6	0	0	0	0	2	0	0	0	3	0	3	0	30				
1:45 PM	1	0	16	0	0	17	2	0	3	1	6	6	0	0	0	0	1	0	0	1	0	1	0	2	0	25			
Hourly Total	2	3	79	1	5	85	3	2	25	3	8	33	0	2	0	1	7	3	0	2	0	5	2	7	0	128			

Grand Total	7	20	404	6	31	437	5	11	148	13	29	177	0	15	2	23	96	40	0	15	1	27	14	43	697
Approach %	1.6	4.6	92.4	1.4	-	-	2.8	6.2	83.6	7.3	-	-	0.0	37.5	5.0	57.5	-	-	0.0	34.9	2.3	62.8	-	-	-
Total %	1.0	2.9	58.0	0.9	-	62.7	0.7	1.6	21.2	1.9	-	25.4	0.0	2.2	0.3	3.3	-	5.7	0.0	2.2	0.1	3.9	-	6.2	-
Lights	7	18	392	6	-	423	5	11	143	9	-	168	0	15	2	23	-	40	0	14	0	26	-	40	671
% Lights	100.0	90.0	97.0	100.0	-	96.8	100.0	100.0	96.6	69.2	-	94.9	-	100.0	100.0	100.0	-	100.0	-	93.3	0.0	96.3	-	93.0	96.3
Buses	0	0	2	0	-	2	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	2	
% Buses	0.0	0.0	0.5	0.0	-	0.5	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.3
Single-Unit Trucks	0	1	8	0	-	9	0	0	3	0	-	3	0	0	0	0	-	0	0	1	0	1	-	2	14
% Single-Unit Trucks	0.0	5.0	2.0	0.0	-	2.1	0.0	0.0	2.0	0.0	-	1.7	-	0.0	0.0	0.0	-	0.0	-	6.7	0.0	3.7	-	4.7	2.0
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	0	
% Articulated Trucks	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	1	2	0	-	3	0	0	2	4	-	6	0	0	0	0	-	0	0	0	1	0	-	1	10
% Bicycles on Road	0.0	5.0	0.5	0.0	-	0.7	0.0	0.0	1.4	30.8	-	3.4	-	0.0	0.0	0.0	-	0.0	-	0.0	100.0	0.0	-	2.3	1.4
Pedestrians	-	-	-	-	-	31	-	-	-	-	-	29	-	-	-	-	-	96	-	-	-	-	14	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-



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Count Name: NS Alley with Pleasant Street  
Site Code:  
Start Date: 09/10/2020  
Page No: 3

### Turning Movement Peak Hour Data (7:15 AM)

Start Time	Pleasant Street Eastbound						Pleasant Street Westbound						NS Alley Northbound						NS Alley Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:15 AM	0	0	6	1	1	7	0	0	1	0	0	1	0	1	0	0	4	1	0	0	0	0	1	0	9
7:30 AM	0	0	15	0	0	15	0	0	4	0	0	4	0	0	0	2	4	2	0	0	0	0	0	0	21
7:45 AM	0	0	11	0	1	11	0	0	3	0	0	3	0	0	0	0	4	0	0	0	0	0	0	0	14
8:00 AM	0	1	15	0	0	16	0	0	2	0	0	2	0	2	0	0	5	2	0	0	0	1	0	1	21
Total	0	1	47	1	2	49	0	0	10	0	0	10	0	3	0	2	17	5	0	0	0	1	1	1	65
Approach %	0.0	2.0	95.9	2.0	-	-	0.0	0.0	100.0	0.0	-	-	0.0	60.0	0.0	40.0	-	-	0.0	0.0	0.0	100.0	-	-	-
Total %	0.0	1.5	72.3	1.5	-	75.4	0.0	0.0	15.4	0.0	-	15.4	0.0	4.6	0.0	3.1	-	7.7	0.0	0.0	0.0	1.5	-	1.5	-
PHF	0.000	0.250	0.783	0.250	-	0.766	0.000	0.000	0.625	0.000	-	0.625	0.000	0.375	0.000	0.250	-	0.625	0.000	0.000	0.000	0.250	-	0.250	0.774
Lights	0	1	46	1	-	48	0	0	10	0	-	10	0	3	0	2	-	5	0	0	0	1	-	1	64
% Lights	-	100.0	97.9	100.0	-	98.0	-	-	100.0	-	-	100.0	-	100.0	-	100.0	-	-	-	-	100.0	-	100.0	-	98.5
Buses	0	0	0	0	-	0	0	0	0	-	0	0	0	0	0	0	-	0	0	0	0	-	0	0	
% Buses	-	0.0	0.0	0.0	-	0.0	-	-	0.0	-	-	0.0	-	0.0	-	0.0	-	-	-	-	0.0	-	0.0	0.0	
Single-Unit Trucks	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	1	
% Single-Unit Trucks	-	0.0	2.1	0.0	-	2.0	-	-	0.0	-	-	0.0	-	0.0	-	0.0	-	-	-	-	0.0	-	0.0	1.5	
Articulated Trucks	0	0	0	0	-	0	0	0	0	-	0	0	0	0	0	0	-	0	0	0	0	-	0	0	
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	-	0.0	-	-	0.0	-	0.0	-	0.0	-	-	-	-	0.0	-	0.0	0.0	
Bicycles on Road	0	0	0	0	-	0	0	0	0	-	0	0	0	0	0	0	-	0	0	0	0	-	0	0	
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	-	0.0	-	-	0.0	-	0.0	-	0.0	-	-	-	-	0.0	-	0.0	0.0	
Pedestrians	-	-	-	-	2	-	-	-	-	-	0	-	-	-	-	-	17	-	-	-	-	1	-	-	
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	





Kenig Lindgren O'Hara Aboona, Inc.  
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018  
(847)518-9990

Count Name: NS Alley with Pleasant Street  
Site Code:  
Start Date: 09/10/2020  
Page No: 5

### Turning Movement Peak Hour Data (12:45 PM)

Start Time	Pleasant Street Eastbound						Pleasant Street Westbound						NS Alley Northbound						NS Alley Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
12:45 PM	0	0	23	0	0	23	0	0	11	0	2	11	0	0	0	1	3	1	0	1	0	1	1	2	37
1:00 PM	0	2	21	0	0	23	1	2	6	0	0	9	0	1	0	1	1	2	0	1	0	0	0	1	35
1:15 PM	0	0	23	1	1	24	0	0	11	1	0	12	0	1	0	0	3	1	0	0	0	1	2	1	38
1:30 PM	1	1	19	0	4	21	0	0	5	1	2	6	0	0	0	0	2	0	0	0	0	3	0	3	30
Total	1	3	86	1	5	91	1	2	33	2	4	38	0	2	0	2	9	4	0	2	0	5	3	7	140
Approach %	1.1	3.3	94.5	1.1	-	-	2.6	5.3	86.8	5.3	-	-	0.0	50.0	0.0	50.0	-	-	0.0	28.6	0.0	71.4	-	-	-
Total %	0.7	2.1	61.4	0.7	-	65.0	0.7	1.4	23.6	1.4	-	27.1	0.0	1.4	0.0	1.4	-	2.9	0.0	1.4	0.0	3.6	-	5.0	-
PHF	0.250	0.375	0.935	0.250	-	0.948	0.250	0.250	0.750	0.500	-	0.792	0.000	0.500	0.000	0.500	-	0.500	0.000	0.500	0.000	0.417	-	0.583	0.921
Lights	1	3	84	1	-	89	1	2	32	2	-	37	0	2	0	2	-	4	0	2	0	4	-	6	136
% Lights	100.0	100.0	97.7	100.0	-	97.8	100.0	100.0	97.0	100.0	-	97.4	-	100.0	-	100.0	-	100.0	-	100.0	-	80.0	-	85.7	97.1
Buses	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	1	
% Buses	0.0	0.0	1.2	0.0	-	1.1	0.0	0.0	0.0	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.7	
Single-Unit Trucks	0	0	1	0	-	1	0	0	1	0	-	1	0	0	0	0	-	0	0	0	0	1	-	1	3
% Single-Unit Trucks	0.0	0.0	1.2	0.0	-	1.1	0.0	0.0	3.0	0.0	-	2.6	-	0.0	-	0.0	-	0.0	-	0.0	-	20.0	-	14.3	2.1
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	0	
% Articulated Trucks	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	-	0	0	
% Bicycles on Road	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	-	0.0	
Pedestrians	-	-	-	-	5	-	-	-	-	-	4	-	-	-	-	-	9	-	-	-	-	3	-	-	
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	



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Count Name: NS Alley with South Boulevard  
Site Code:  
Start Date: 09/10/2020  
Page No: 1

### Turning Movement Data

Start Time	South Boulevard					South Boulevard					NS Alley					Int. Total	
	Eastbound					Westbound					Northbound						
	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total		
7:00 AM	0	4	0	3	4	0	0	30	0	30	0	0	0	2	0	34	
7:15 AM	0	0	0	1	0	0	0	38	0	38	0	0	0	4	0	38	
7:30 AM	0	4	0	0	4	0	0	39	0	39	0	0	0	4	0	43	
7:45 AM	0	3	0	0	3	0	0	41	0	41	0	0	0	6	0	44	
Hourly Total	0	11	0	4	11	0	0	148	0	148	0	0	0	16	0	159	
8:00 AM	0	5	0	0	5	0	0	37	0	37	0	0	2	3	2	44	
8:15 AM	0	7	0	3	7	0	0	38	2	38	0	1	0	5	1	46	
8:30 AM	0	2	0	2	2	0	0	28	1	28	0	0	0	0	0	30	
8:45 AM	0	5	0	0	5	2	1	38	0	41	0	0	0	1	0	46	
Hourly Total	0	19	0	5	19	2	1	141	3	144	0	1	2	9	3	166	
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
4:00 PM	0	6	0	8	6	0	0	68	0	68	0	2	0	18	2	76	
4:15 PM	0	6	0	5	6	0	0	79	0	79	0	2	1	11	3	88	
4:30 PM	0	7	0	5	7	1	2	66	0	69	0	0	1	13	1	77	
4:45 PM	0	9	0	2	9	0	2	88	2	90	0	5	1	4	6	105	
Hourly Total	0	28	0	20	28	1	4	301	2	306	0	9	3	46	12	346	
5:00 PM	0	13	0	1	13	0	0	80	2	80	0	1	0	11	1	94	
5:15 PM	1	10	1	7	12	0	1	77	0	78	0	1	0	13	1	91	
5:30 PM	0	7	1	4	8	1	1	71	1	73	0	0	1	20	1	82	
5:45 PM	0	8	0	2	8	1	0	85	2	86	0	1	1	15	2	96	
Hourly Total	1	38	2	14	41	2	2	313	5	317	0	3	2	59	5	363	
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
11:30 AM	0	6	0	1	6	0	2	61	0	63	0	0	0	10	0	69	
11:45 AM	0	6	1	0	7	0	1	66	0	67	0	2	4	11	6	80	
Hourly Total	0	12	1	1	13	0	3	127	0	130	0	2	4	21	6	149	
12:00 PM	0	9	0	3	9	0	0	50	0	50	0	0	1	8	1	60	
12:15 PM	0	3	0	1	3	0	3	57	0	60	0	2	1	2	3	66	
12:30 PM	0	4	0	1	4	0	0	71	1	71	0	0	0	9	0	75	
12:45 PM	0	10	0	1	10	0	1	57	1	58	0	0	0	19	0	68	
Hourly Total	0	26	0	6	26	0	4	235	2	239	0	2	2	38	4	269	
1:00 PM	0	10	0	0	10	0	0	63	0	63	0	1	0	18	1	74	
1:15 PM	0	6	0	6	6	0	0	61	0	61	0	0	0	21	0	67	
1:30 PM	0	12	0	2	12	0	1	69	1	70	0	0	1	13	1	83	
1:45 PM	0	5	0	5	5	0	0	58	1	58	0	1	2	18	3	66	
Hourly Total	0	33	0	13	33	0	1	251	2	252	0	2	3	70	5	290	
Grand Total	1	167	3	63	171	5	15	1516	14	1536	0	19	16	259	35	1742	

Approach %	0.6	97.7	1.8	-	-	0.3	1.0	98.7	-	-	0.0	54.3	45.7	-	-	-
Total %	0.1	9.6	0.2	-	9.8	0.3	0.9	87.0	-	88.2	0.0	1.1	0.9	-	2.0	-
Lights	1	157	2	-	160	5	13	1441	-	1459	0	14	15	-	29	1648
% Lights	100.0	94.0	66.7	-	93.6	100.0	86.7	95.1	-	95.0	-	73.7	93.8	-	82.9	94.6
Buses	0	0	0	-	0	0	0	48	-	48	0	0	0	-	0	48
% Buses	0.0	0.0	0.0	-	0.0	0.0	0.0	3.2	-	3.1	-	0.0	0.0	-	0.0	2.8
Single-Unit Trucks	0	2	0	-	2	0	1	16	-	17	0	0	1	-	1	20
% Single-Unit Trucks	0.0	1.2	0.0	-	1.2	0.0	6.7	1.1	-	1.1	-	0.0	6.3	-	2.9	1.1
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	8	1	-	9	0	1	11	-	12	0	5	0	-	5	26
% Bicycles on Road	0.0	4.8	33.3	-	5.3	0.0	6.7	0.7	-	0.8	-	26.3	0.0	-	14.3	1.5
Pedestrians	-	-	-	-	63	-	-	-	-	14	-	-	-	-	259	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	100.0	-



Kenig Lindgren O'Hara Aboona, Inc.  
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018  
(847)518-9990

Count Name: NS Alley with South Boulevard  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Peak Hour Data (7:15 AM)

Start Time	South Boulevard					South Boulevard					NS Alley					
	Eastbound				App. Total	Westbound				App. Total	Northbound				Int. Total	
	U-Turn	Thru	Right	Peds		U-Turn	Left	Thru	Peds		U-Turn	Left	Right	Peds		
7:15 AM	0	0	0	1	0	0	0	38	0	38	0	0	0	4	0	38
7:30 AM	0	4	0	0	4	0	0	39	0	39	0	0	0	4	0	43
7:45 AM	0	3	0	0	3	0	0	41	0	41	0	0	0	6	0	44
8:00 AM	0	5	0	0	5	0	0	37	0	37	0	0	2	3	2	44
Total	0	12	0	1	12	0	0	155	0	155	0	0	2	17	2	169
Approach %	0.0	100.0	0.0	-	-	0.0	0.0	100.0	-	-	0.0	0.0	100.0	-	-	-
Total %	0.0	7.1	0.0	-	7.1	0.0	0.0	91.7	-	91.7	0.0	0.0	1.2	-	1.2	-
PHF	0.000	0.600	0.000	-	0.600	0.000	0.000	0.945	-	0.945	0.000	0.000	0.250	-	0.250	0.960
Lights	0	11	0	-	11	0	0	139	-	139	0	0	2	-	2	152
% Lights	-	91.7	-	-	91.7	-	-	89.7	-	89.7	-	-	100.0	-	100.0	89.9
Buses	0	0	0	-	0	0	0	9	-	9	0	0	0	-	0	9
% Buses	-	0.0	-	-	0.0	-	-	5.8	-	5.8	-	-	0.0	-	0.0	5.3
Single-Unit Trucks	0	1	0	-	1	0	0	5	-	5	0	0	0	-	0	6
% Single-Unit Trucks	-	8.3	-	-	8.3	-	-	3.2	-	3.2	-	-	0.0	-	0.0	3.6
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	-	-	0.0	-	-	0.0	-	0.0	-	-	0.0	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	0	0	2	-	2	0	0	0	-	0	2
% Bicycles on Road	-	0.0	-	-	0.0	-	-	1.3	-	1.3	-	-	0.0	-	0.0	1.2
Pedestrians	-	-	-	1	-	-	-	0	-	-	-	-	17	-	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	-	-	-	-	100.0	-	-	-



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Count Name: NS Alley with South Boulevard  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Peak Hour Data (4:15 PM)

Start Time	South Boulevard					South Boulevard					NS Alley					
	Eastbound				App. Total	Westbound				App. Total	Northbound				Int. Total	
	U-Turn	Thru	Right	Peds		U-Turn	Left	Thru	Peds		U-Turn	Left	Right	Peds		
4:15 PM	0	6	0	5	6	0	0	79	0	79	0	2	1	11	3	88
4:30 PM	0	7	0	5	7	1	2	66	0	69	0	0	1	13	1	77
4:45 PM	0	9	0	2	9	0	2	88	2	90	0	5	1	4	6	105
5:00 PM	0	13	0	1	13	0	0	80	2	80	0	1	0	11	1	94
Total	0	35	0	13	35	1	4	313	4	318	0	8	3	39	11	364
Approach %	0.0	100.0	0.0	-	-	0.3	1.3	98.4	-	-	0.0	72.7	27.3	-	-	-
Total %	0.0	9.6	0.0	-	9.6	0.3	1.1	86.0	-	87.4	0.0	2.2	0.8	-	3.0	-
PHF	0.000	0.673	0.000	-	0.673	0.250	0.500	0.889	-	0.883	0.000	0.400	0.750	-	0.458	0.867
Lights	0	33	0	-	33	1	4	303	-	308	0	4	3	-	7	348
% Lights	-	94.3	-	-	94.3	100.0	100.0	96.8	-	96.9	-	50.0	100.0	-	63.6	95.6
Buses	0	0	0	-	0	0	0	8	-	8	0	0	0	-	0	8
% Buses	-	0.0	-	-	0.0	0.0	0.0	2.6	-	2.5	-	0.0	0.0	-	0.0	2.2
Single-Unit Trucks	0	0	0	-	0	0	0	1	-	1	0	0	0	-	0	1
% Single-Unit Trucks	-	0.0	-	-	0.0	0.0	0.0	0.3	-	0.3	-	0.0	0.0	-	0.0	0.3
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	-	-	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	2	0	-	2	0	0	1	-	1	0	4	0	-	4	7
% Bicycles on Road	-	5.7	-	-	5.7	0.0	0.0	0.3	-	0.3	-	50.0	0.0	-	36.4	1.9
Pedestrians	-	-	-	13	-	-	-	-	4	-	-	-	-	39	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	100.0	-	-



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Count Name: NS Alley with South Boulevard  
Site Code:  
Start Date: 09/10/2020  
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### Turning Movement Peak Hour Data (12:45 PM)

Start Time	South Boulevard					South Boulevard					NS Alley					Int. Total	
	Eastbound					Westbound					Northbound						
	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total		
12:45 PM	0	10	0	1	10	0	1	57	1	58	0	0	0	19	0	68	
1:00 PM	0	10	0	0	10	0	0	63	0	63	0	1	0	18	1	74	
1:15 PM	0	6	0	6	6	0	0	61	0	61	0	0	0	21	0	67	
1:30 PM	0	12	0	2	12	0	1	69	1	70	0	0	1	13	1	83	
Total	0	38	0	9	38	0	2	250	2	252	0	1	1	71	2	292	
Approach %	0.0	100.0	0.0	-	-	0.0	0.8	99.2	-	-	0.0	50.0	50.0	-	-	-	
Total %	0.0	13.0	0.0	-	13.0	0.0	0.7	85.6	-	86.3	0.0	0.3	0.3	-	0.7	-	
PHF	0.000	0.792	0.000	-	0.792	0.000	0.500	0.906	-	0.900	0.000	0.250	0.250	-	0.500	0.880	
Lights	0	38	0	-	38	0	2	241	-	243	0	1	1	-	2	283	
% Lights	-	100.0	-	-	100.0	-	100.0	96.4	-	96.4	-	100.0	100.0	-	100.0	96.9	
Buses	0	0	0	-	0	0	0	5	-	5	0	0	0	-	0	5	
% Buses	-	0.0	-	-	0.0	-	0.0	2.0	-	2.0	-	0.0	0.0	-	0.0	1.7	
Single-Unit Trucks	0	0	0	-	0	0	0	2	-	2	0	0	0	-	0	2	
% Single-Unit Trucks	-	0.0	-	-	0.0	-	0.0	0.8	-	0.8	-	0.0	0.0	-	0.0	0.7	
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0	
% Articulated Trucks	-	0.0	-	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0	
Bicycles on Road	0	0	0	-	0	0	0	2	-	2	0	0	0	-	0	2	
% Bicycles on Road	-	0.0	-	-	0.0	-	0.0	0.8	-	0.8	-	0.0	0.0	-	0.0	0.7	
Pedestrians	-	-	-	9	-	-	-	-	2	-	-	-	-	71	-	-	
% Pedestrians	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	100.0	-	-	

Oak Park, IL Weather: Cool and Afternoon Rain 09/11/20  
 Randolph St and Maple Ave Cars and Vans Only 14:35:36  
 Thursday September 10, 2020

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Counts: All Vehicles - by Mvmt

Intersection # 8 randolph/maple/carsvans													
Begin Time	N-Approach			E-Approach			S-Approach			W-Approach			Int Total
	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT	
700	5	1	0	0	22	0	2	1	1	4	20	2	58
715	6	0	0	0	31	2	1	1	0	1	24	1	67
730	3	2	0	0	27	5	2	0	0	6	21	1	67
745	2	0	1	1	30	3	6	4	2	3	34	3	89
800	4	2	1	1	31	1	0	2	2	2	33	1	80
815	4	1	1	0	28	1	2	2	1	1	26	1	68
830	3	3	0	2	19	0	1	0	1	3	29	1	62
845	2	1	0	3	23	1	1	1	5	2	39	1	79
<hr/>													
1600	4	1	4	4	50	1	2	2	1	5	67	1	142
1615	6	1	0	0	32	2	3	1	2	3	69	4	123
1630	4	1	2	2	49	3	7	4	4	7	69	3	155
1645	3	0	4	1	60	3	5	0	5	6	76	5	168
1700	6	1	1	4	60	2	3	4	3	4	83	6	177
1715	6	2	3	0	45	0	3	2	1	5	70	8	145
1730	4	1	0	2	52	3	4	1	2	3	73	7	152
1745	5	0	0	1	44	1	3	5	2	3	67	4	135
<hr/>													
Total	67	17	17	21	603	28	45	30	32	58	800	49	1767

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Counts: All Vehicles - Totals

Intersection # 8 randolph/maple/carsvans													
Begin Time	Approach Totals				Exit Totals								
	N	E	S	W	N	E	S	W	N	E	S	W	
700	6	22	4	26		3	22	5	28				58
715	6	33	2	26		2	25	3	37				67
730	5	32	2	28		1	23	13	30				67
745	3	34	12	40		8	41	6	34				89
800	7	33	4	36		4	34	5	37				80
815	6	29	5	28		3	29	3	33				68
830	6	21	2	33		3	30	6	23				62
845	3	27	7	42		5	40	4	30				79
<hr/>													
1600	9	55	5	73		7	73	7	55				142
1615	7	34	6	76		5	72	6	40				123
1630	7	54	15	79		9	78	11	57				155
1645	7	64	10	87		6	85	9	68				168
1700	8	66	10	93		14	87	7	69				177
1715	11	45	6	83		10	76	7	52				145
1730	5	57	7	83		10	77	7	58				152
1745	5	46	10	74		10	70	4	51				135
<hr/>													
Total	101	652	107	907		100	862	103	702				1767

Oak Park, IL Weather:  
Randolph St and Maple Ave  
Thursday September 10, 2020

**Weather:** Cool and Afternoon Rain  
**People:** Cars and Vans Only

09/11/20  
14:35:36

URNS/TEAPAC[Ver 3.61.12] - 15-Minute Flow Rates: by Movement

Intersection # 8 randolph/maple/carsvans

Begin Time	N-Approach			E-Approach			S-Approach			W-Approach			Int Total
	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT	
700	20	4	0	0	88	0	8	4	4	16	80	8	232
715	24	0	0	0	124	8	4	4	0	4	96	4	268
730	12	8	0	0	108	20	8	0	0	24	84	4	268
745	8	0	4	4	120	12	24	16	8	12	136	12	356
800	16	8	4	4	124	4	0	8	8	8	132	4	320
815	16	4	4	0	112	4	8	8	4	4	104	4	272
830	12	12	0	8	76	0	4	0	4	12	116	4	248
845	8	4	0	12	92	4	4	4	20	8	156	4	316
1600	16	4	16	16	200	4	8	8	4	20	268	4	568
1615	24	4	0	0	128	8	12	4	8	12	276	16	492
1630	16	4	8	8	196	12	28	16	16	28	276	12	620
1645	12	0	16	4	240	12	20	0	20	24	304	20	672
1700	24	4	4	16	240	8	12	16	12	16	332	24	708
1715	24	8	12	0	180	0	12	8	4	20	280	32	580
1730	16	4	0	8	208	12	16	4	8	12	292	28	608
1745	20	0	0	4	176	4	12	20	8	12	268	16	540

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Flow Rates: Appr/Exit Totals

Intersection # 8 randolph/maple/carsvans

Begin Time	Approach Totals				Exit Totals				Int Total
	N	E	S	W	N	E	S	W	
700	24	88	16	104	12	88	20	112	232
715	24	132	8	104	8	100	12	148	268
730	20	128	8	112	4	92	52	120	268
745	12	136	48	160	32	164	24	136	356
800	28	132	16	144	16	136	20	148	320
815	24	116	20	112	12	116	12	132	272
830	24	84	8	132	12	120	24	92	248
845	12	108	28	168	20	160	16	120	316
1600	36	220	20	292	28	292	28	220	568
1615	28	136	24	304	20	288	24	160	492
1630	28	216	60	316	36	312	44	228	620
1645	28	256	40	348	24	340	36	272	672
1700	32	264	40	372	56	348	28	276	708
1715	44	180	24	332	40	304	28	208	580
1730	20	228	28	332	40	308	28	232	608
1745	20	184	40	296	40	280	16	204	540

Oak Park, IL Weather:  
Randolph St and Maple Ave  
Thursday September 10, 2020

Cool and Afternoon Rain  
Cars and Vans Only

09/11/20  
14:35:36

TURNS/TEAPAC[Ver 3.61.12] - 60-Minute Volumes: by Movement

Intersection # 8 randolph/maple/carsvans

Begin Time	N-Approach			E-Approach			S-Approach			W-Approach			Int Total
	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT	
700	16	3	1	1	110	10	11	6	3	14	99	7	281
715	15	4	2	2	119	11	9	7	4	12	112	6	303
730	13	5	3	2	116	10	10	8	5	12	114	6	304
745	13	6	3	4	108	5	9	8	6	9	122	6	299
800	13	7	2	6	101	3	4	5	9	8	127	4	289
815	9	5	1	5	70	2	4	3	7	6	94	3	209*
830	5	4	0	5	42	1	2	1	6	5	68	2	141*
845	2	1	0	3	23	1	1	1	5	2	39	1	79*
1600	17	3	10	7	191	9	17	7	12	21	281	13	588
1615	19	3	7	7	201	10	18	9	14	20	297	18	623
1630	19	4	10	7	214	8	18	10	13	22	298	22	645
1645	19	4	8	7	217	8	15	7	11	18	302	26	642
1700	21	4	4	7	201	6	13	12	8	15	293	25	609
1715	15	3	3	3	141	4	10	8	5	11	210	19	432*
1730	9	1	0	3	96	4	7	6	4	6	140	11	287*
1745	5	0	0	1	44	1	3	5	2	3	67	4	135*

TURNS/TEAPAC[Ver 3.61.12] - 60-Minute Volumes: Appr/Exit Totals

Intersection # 8 randolph/maple/carsvans

Begin Time	Approach Totals				Exit Totals				Int Total
	N	E	S	W	N	E	S	W	
700	20	121	20	120	14	111	27	129	281
715	21	132	20	130	15	123	27	138	303
730	21	128	23	132	16	127	27	134	304
745	22	117	23	137	18	134	20	127	299
800	22	110	18	139	15	133	18	123	289
815	15	77	14	103	11	99	13	86	209*
830	9	48	9	75	8	70	10	53	141*
845	3	27	7	42	5	40	4	30	79*
1600	30	207	36	315	27	308	33	220	588
1615	29	218	41	335	34	322	33	234	623
1630	33	229	41	342	39	326	34	246	645
1645	31	232	33	346	40	325	30	247	642
1700	29	214	33	333	44	310	25	230	609
1715	21	148	23	240	30	223	18	161	432*
1730	10	103	17	157	20	147	11	109	287*
1745	5	46	10	74	10	70	4	51	135*

Oak Park, IL  
Randolph St and Maple Ave  
Thursday September 10, 2020

Weather: Cool and Afternoon Rain  
Heavy Vehicles Only

09/11/20  
09:59:03

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Counts: All Vehicles - by Mvmt

Intersection # 7 randolph/maple/heavyveh

Begin Time	N-Approach			E-Approach			S-Approach			W-Approach			Int Total
	RT	TH	LT										
700	0	0	0	0	0	0	0	0	0	0	0	1	1
715	0	0	0	0	0	0	0	0	0	0	0	0	0
730	0	0	0	1	0	0	0	1	0	0	1	0	3
745	0	0	0	0	0	0	0	0	0	0	1	0	1
800	0	0	0	0	0	0	0	0	1	0	1	0	2
815	0	0	0	0	0	0	0	0	0	0	0	0	0
830	0	0	1	0	0	0	0	0	0	0	0	0	1
845	0	0	0	0	0	0	0	1	0	0	0	1	2
<hr/>													
1600	0	0	0	0	1	0	0	0	0	0	0	0	1
1615	0	0	0	0	1	0	0	0	0	0	0	2	3
1630	0	0	0	1	1	0	0	0	0	0	1	0	3
1645	1	0	0	0	0	0	0	0	0	0	1	0	2
1700	0	0	0	0	1	0	0	0	0	0	0	0	1
1715	0	0	0	0	0	0	0	0	0	0	1	0	1
1730	0	0	0	0	1	0	0	0	0	0	1	0	2
1745	0	0	0	0	0	0	0	0	0	0	1	0	1
<hr/>													
Total	1	0	1	2	5	0	0	2	1	0	8	4	24

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Counts: All Vehicles - Totals

Intersection # 7 randolph/maple/heavyveh

Begin Time	Approach Totals				Exit Totals				Int Total
	N	E	S	W	N	E	S	W	
700	0	0	0	1	1	0	0	0	1
715	0	0	0	0	0	0	0	0	0
730	0	1	1	1	2	1	0	0	3
745	0	0	0	1	0	1	0	0	1
800	0	0	1	1	0	1	0	1	2
815	0	0	0	0	0	0	0	0	0
830	1	0	0	0	0	1	0	0	1
845	0	0	1	1	2	0	0	0	2
<hr/>									
1600	0	1	0	0	0	0	0	1	1
1615	0	1	0	2	2	0	0	1	3
1630	0	2	0	1	1	1	0	1	3
1645	1	0	0	1	0	1	0	1	2
1700	0	1	0	0	0	0	0	1	1
1715	0	0	0	1	0	1	0	0	1
1730	0	1	0	1	0	1	0	1	2
1745	0	0	0	1	0	1	0	0	1
<hr/>									
Total	2	7	3	12	8	9	0	7	24

Oak Park, IL Weather:  
Randolph St and Maple Ave  
Thursday September 10, 2020

**Weather:** Cool and Afternoon Rain

Cool and Afternoon Rain

09/11/20

09:59:03

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Flow Rates: by Movement

Intersection # 7 randolph/maple/heavyveh

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Flow Rates: Appr/Exit Totals

Intersection # 7 randolph/maple/heavyveh

Oak Park, IL Weather:  
Randolph St and Maple Ave  
Thursday September 10, 2020

Weather: Cool and Afternoon Rain  
ole Ave Heavy Vehicles Only

09/11/20  
09:59:03

TURNS/TEAPAC[Ver 3.61.12] - 60-Minute Volumes: by Movement

Intersection # 7 randolph/maple/heavyveh

TURNS/TEAPAC[Ver 3.61.12] - 60-Minute Volumes: Appr/Exit Totals

Intersection # 7 randolph/maple/heavyveh

Begin Time	Approach Totals				Exit Totals				Int Total
	N	E	S	W	N	E	S	W	
700	0	1	1	3	3	2	0	0	5
715	0	1	2	3	2	3	0	1	6
730	0	1	2	3	2	3	0	1	6
745	1	0	1	2	0	3	0	1	4
800	1	0	2	2	2	2	0	1	5
815	1	0	1	1	2	1	0	0	3*
830	1	0	1	1	2	1	0	0	3*
845	0	0	1	1	2	0	0	0	2*
1600	1	4	0	4	3	2	0	4	9
1615	1	4	0	4	3	2	0	4	9
1630	1	3	0	3	1	3	0	3	7
1645	1	2	0	3	0	3	0	3	6
1700	0	2	0	3	0	3	0	2	5
1715	0	1	0	3	0	3	0	1	4*
1730	0	1	0	2	0	2	0	1	3*
1745	0	0	0	1	0	1	0	0	1*

City: Oak Park

Count Location: Randolph St. and Maple Ave

Study Date: – September 10<sup>th</sup>, 2020 (On-Street Biker Counts)

Time	East Approach	West Approach	North Approach	South Approach	Total Bikers
7:00-7:15 a.m.	0	1	0	1	2
7:15-7:30 a.m.	2	0	0	0	2
7:30-7:45 a.m.	0	1	0	0	1
7:45-8:00 a.m.	0	1	0	0	1
8:00-8:15 a.m.	1	1	1	1	4
8:15-8:30 a.m.	0	1	0	0	1
8:30-8:45 a.m.	1	1	0	0	2
8:45-9:00 a.m.	1	1	1	0	3
<b>Morning Totals</b>	<b>5</b>	<b>7</b>	<b>2</b>	<b>2</b>	<b>16</b>
4:00-4:15 p.m.	0	1	0	1	2
4:15-4:30 p.m.	1	0	0	1	2
4:30-4:45 p.m.	1	2	0	0	3
4:45-5:00 p.m.	0	0	0	1	1
5:00-5:15 p.m.	0	3	0	0	3
5:15-5:30 p.m.	1	0	0	0	1
5:30-5:45 p.m.	0	0	0	0	0
5:45-6:00 p.m.	0	0	0	0	0
<b>Afternoon Totals</b>	<b>3</b>	<b>6</b>	<b>0</b>	<b>3</b>	<b>12</b>

City: Oak Park

Count Location: Randolph St. and Maple Ave

Study Date: – September 10<sup>th</sup>, 2020 (Pedestrian Crosswalk Counts)

Time	East Crosswalk	West Crosswalk	North Crosswalk	South Crosswalk	Total Pedestrians
7:00-7:15 a.m.	0	1	1	1	3
7:15-7:30 a.m.	2	1	5	4	12
7:30-7:45 a.m.	1	1	1	1	4
7:45-8:00 a.m.	2	3	6	9	20
8:00-8:15 a.m.	1	2	3	9	15
8:15-8:30 a.m.	3	1	4	3	11
8:30-8:45 a.m.	1	1	5	5	12
8:45-9:00 a.m.	2	2	1	4	9
<b>Morning Totals</b>	<b>12</b>	<b>12</b>	<b>26</b>	<b>36</b>	<b>86</b>
4:00-4:15 p.m.	6	2	6	5	19
4:15-4:30 p.m.	0	6	5	3	14
4:30-4:45 p.m.	0	1	4	12	17
4:45-5:00 p.m.	2	3	3	15	23
5:00-5:15 p.m.	3	2	3	4	12
5:15-5:30 p.m.	5	1	3	1	10
5:30-5:45 p.m.	2	1	3	0	6
5:45-6:00 p.m.	1	0	1	0	2
<b>Afternoon Totals</b>	<b>19</b>	<b>16</b>	<b>28</b>	<b>40</b>	<b>103</b>

Westbound Queue Observation Notes:

- During the morning count there were no observed westbound backups on Randolph St. extending back to/beyond Maple Ave.
- During the morning count there were multiple observed westbound backups on Randolph St. extending back to/beyond Maple Ave. These backups were brief (about 20-60 seconds in duration) and each backup cleared quickly with every green light cycle phase. The times of these backups were at 4:45 p.m. (backup just up to west crosswalk), 4:53 p.m. (backup beyond Maple of 4 vehicles to the east of the intersection), 5:01 p.m. (backup beyond Maple of 1 vehicle to the east of the intersection), 5:06 p.m. (backup just up to west crosswalk), 5:08 p.m. (backup just past west crosswalk but not blocking intersection), 5:25 p.m. (backup extends partially into the intersection but not beyond), 5:37 p.m. (backup beyond Maple of 2 vehicles to the east of the intersection), 5:39 p.m., (backup just up to west crosswalk), 5:46 p.m. (backup just up to west crosswalk), and 5:59 p.m. (backup extends partially into the intersection).
- Please note that none of the observed backups were completely blocking the intersection as north and south thru movements at the intersection could easily be made.
- There were a few observations when these backups on Randolph occurred that vehicles on Maple Ave had to temporarily wait to turn westbound onto Randolph St. until the green light at Harlem Ave. cleared the westbound backups on Randolph. These backups on Maple were few and only 1 maybe 2 vehicles at most waiting.

Oak Park, IL  
Randolph St and Maple Ave  
Saturday September 12, 2020

Weather:  
Cars and Vans Only

09/14/20  
13:37:59

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Counts: All Vehicles - by Mvmt

Intersection # 9 randolph/maple/cars/vans/sat

Begin Time	N-Approach			E-Approach			S-Approach			W-Approach			Int Total
	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT	
1130	6	2	1	0	34	2	1	0	1	2	39	2	90
1145	4	1	1	2	33	2	3	2	1	3	57	4	113
1200	4	2	2	1	62	1	0	1	3	1	44	7	128
1215	4	1	0	2	39	1	2	7	1	1	49	2	109
1230	1	3	2	1	33	2	3	5	2	3	43	1	99
1245	4	2	1	0	48	0	4	3	2	3	50	6	123
1300	9	1	0	1	26	2	3	0	0	5	44	2	93
1315	4	3	0	1	36	1	2	1	5	1	52	7	113
Total	36	15	7	8	311	11	18	19	15	19	378	31	868

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Counts: All Vehicles - Totals

Intersection # 9 randolph/maple/cars/vans/sat

Begin Time	Approach Totals				Exit Totals				Int Total
	N	E	S	W	N	E	S	W	
1130	9	36	2	43	2	41	6	41	90
1145	6	37	6	64	8	61	6	38	113
1200	8	64	4	52	9	46	4	69	128
1215	5	42	10	52	11	51	3	44	109
1230	6	36	10	47	7	48	8	36	99
1245	7	48	9	59	9	55	5	54	123
1300	10	29	3	51	3	47	8	35	93
1315	7	38	8	60	9	54	5	45	113
Total	58	330	52	428	58	403	45	362	868

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Flow Rates: by Movement

Intersection # 9 randolph/maple/cars/vans/sat

Begin Time	N-Approach			E-Approach			S-Approach			W-Approach			Int Total
	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT	
1130	24	8	4	0	136	8	4	0	4	8	156	8	360
1145	16	4	4	8	132	8	12	8	4	12	228	16	452
1200	16	8	8	4	248	4	0	4	12	4	176	28	512
1215	16	4	0	8	156	4	8	28	4	4	196	8	436
1230	4	12	8	4	132	8	12	20	8	12	172	4	396
1245	16	8	4	0	192	0	16	12	8	12	200	24	492
1300	36	4	0	4	104	8	12	0	0	20	176	8	372
1315	16	12	0	4	144	4	8	4	20	4	208	28	452

Oak Park, IL  
Randolph St and Maple Ave  
Saturday September 12, 2020

Weather: Cool and Light Rain  
Cars and Vans Only

09/14/20  
13:37:59

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Flow Rates: Appr/Exit Totals

Intersection # 9 randolph/maple/cars/vans/sat

Begin Time	Approach Totals				Exit Totals				Int Total
	N	E	S	W	N	E	S	W	
1130	36	144	8	172	8	164	24	164	360
1145	24	148	24	256	32	244	24	152	452
1200	32	256	16	208	36	184	16	276	512
1215	20	168	40	208	44	204	12	176	436
1230	24	144	40	188	28	192	32	144	396
1245	28	192	36	236	36	220	20	216	492
1300	40	116	12	204	12	188	32	140	372
1315	28	152	32	240	36	216	20	180	452

TURNS/TEAPAC[Ver 3.61.12] - 60-Minute Volumes: by Movement

Intersection # 9 randolph/maple/cars/vans/sat

Begin Time	N-Approach			E-Approach			S-Approach			W-Approach			Int Total
	RT	TH	LT	RT	TH	LT	RT	TH	LT	RT	TH	LT	
1130	18	6	4	5	168	6	6	10	6	7	189	15	440
1145	13	7	5	6	167	6	8	15	7	8	193	14	449
1200	13	8	5	4	182	4	9	16	8	8	186	16	459
1215	18	7	3	4	146	5	12	15	5	12	186	11	424
1230	18	9	3	3	143	5	12	9	9	12	189	16	428
1245	17	6	1	2	110	3	9	4	7	9	146	15	329*
1300	13	4	0	2	62	3	5	1	5	6	96	9	206*
1315	4	3	0	1	36	1	2	1	5	1	52	7	113*

TURNS/TEAPAC[Ver 3.61.12] - 60-Minute Volumes: Appr/Exit Totals

Intersection # 9 randolph/maple/cars/vans/sat

Begin Time	Approach Totals				Exit Totals				Int Total
	N	E	S	W	N	E	S	W	
1130	28	179	22	211	30	199	19	192	440
1145	25	179	30	215	35	206	21	187	449
1200	26	190	33	210	36	200	20	203	459
1215	28	155	32	209	30	201	24	169	424
1230	30	151	30	217	28	204	26	170	428
1245	24	115	20	170	21	156	18	134	329*
1300	17	67	11	111	12	101	13	80	206*
1315	7	38	8	60	9	54	5	45	113*

Oak Park, IL  
Randolph St and Maple Ave  
Saturday September 12, 2020

Weather: Cool and Light Rain  
Heavy Vehicles Only

09/14/20  
13:44:35

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Counts: All Vehicles - by Mvmt

Intersection # 10 Randolph/maple/heavy/sat

Begin Time	N-Approach			E-Approach			S-Approach			W-Approach			Int Total
	RT	TH	LT										
1130	0	0	0	0	1	0	0	0	1	0	0	0	2
1145	0	0	0	0	0	0	0	0	0	0	0	1	1
1200	0	0	0	0	0	0	0	0	0	1	0	0	1
1215	0	0	0	0	1	0	0	1	0	0	0	0	2
1230	0	0	0	0	0	0	0	0	0	0	0	0	0
1245	0	0	0	0	1	0	0	0	0	0	0	0	1
1300	0	1	0	0	0	0	0	0	0	0	1	0	2
1315	0	1	0	0	0	0	0	0	0	0	1	0	2
Total	0	2	0	0	3	0	0	1	1	1	2	1	11

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Counts: All Vehicles - Totals

Intersection # 10 Randolph/maple/heavy/sat

Begin Time	Approach Totals				Exit Totals				Int Total
	N	E	S	W	N	E	S	W	
1130	0	1	1	0	0	0	0	2	2
1145	0	0	0	1	1	0	0	0	1
1200	0	0	0	1	0	0	1	0	1
1215	0	1	1	0	1	0	0	1	2
1230	0	0	0	0	0	0	0	0	0
1245	0	1	0	0	0	0	0	1	1
1300	1	0	0	1	0	1	1	0	2
1315	1	0	0	1	0	1	1	0	2
Total	2	3	2	4	2	2	3	4	11

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Flow Rates: by Movement

Intersection # 10 Randolph/maple/heavy/sat

Begin Time	N-Approach			E-Approach			S-Approach			W-Approach			Int Total
	RT	TH	LT										
1130	0	0	0	0	4	0	0	0	4	0	0	0	8
1145	0	0	0	0	0	0	0	0	0	0	0	4	4
1200	0	0	0	0	0	0	0	0	0	4	0	0	4
1215	0	0	0	0	4	0	0	4	0	0	0	0	8
1230	0	0	0	0	0	0	0	0	0	0	0	0	0
1245	0	0	0	0	4	0	0	0	0	0	0	0	4
1300	0	4	0	0	0	0	0	0	0	0	4	0	8
1315	0	4	0	0	0	0	0	0	0	0	4	0	8

Oak Park, IL  
Randolph St and Maple Ave  
Saturday September 12, 2020

Weather: Cool and Light Rain  
Heavy Vehicles Only

09/14/20  
13:44:35

TURNS/TEAPAC[Ver 3.61.12] - 15-Minute Flow Rates: Appr/Exit Totals

Intersection # 10 Randolph/maple/heavy/sat

Begin Time	Approach Totals				Exit Totals				Int Total
	N	E	S	W	N	E	S	W	
1130	0	4	4	0	0	0	0	8	8
1145	0	0	0	4	4	0	0	0	4
1200	0	0	0	4	0	0	4	0	4
1215	0	4	4	0	4	0	0	4	8
1230	0	0	0	0	0	0	0	0	0
1245	0	4	0	0	0	0	0	4	4
1300	4	0	0	4	0	4	4	0	8
1315	4	0	0	4	0	4	4	0	8

TURNS/TEAPAC[Ver 3.61.12] - 60-Minute Volumes: by Movement

Intersection # 10 Randolph/maple/heavy/sat

Begin Time	N-Approach			E-Approach			S-Approach			W-Approach			Int Total
	RT	TH	LT										
1130	0	0	0	0	2	0	0	1	1	1	0	1	6
1145	0	0	0	0	1	0	0	1	0	1	0	1	4
1200	0	0	0	0	2	0	0	1	0	1	0	0	4
1215	0	1	0	0	2	0	0	1	0	0	1	0	5
1230	0	2	0	0	1	0	0	0	0	0	2	0	5
1245	0	2	0	0	1	0	0	0	0	0	2	0	5*
1300	0	2	0	0	0	0	0	0	0	0	2	0	4*
1315	0	1	0	0	0	0	0	0	0	0	1	0	2*

TURNS/TEAPAC[Ver 3.61.12] - 60-Minute Volumes: Appr/Exit Totals

Intersection # 10 Randolph/maple/heavy/sat

Begin Time	Approach Totals				Exit Totals				Int Total
	N	E	S	W	N	E	S	W	
1130	0	2	2	2	2	0	1	3	6
1145	0	1	1	2	2	0	1	1	4
1200	0	2	1	1	1	0	1	2	4
1215	1	2	1	1	1	1	1	2	5
1230	2	1	0	2	0	2	2	1	5
1245	2	1	0	2	0	2	2	1	5*
1300	2	0	0	2	0	2	2	0	4*
1315	1	0	0	1	0	1	1	0	2*

City: Oak Park

Count Location: Randolph St. and Maple Ave

Study Date: – September 12<sup>th</sup>, 2020 (On-Street Biker Counts)

Time	East Approach	West Approach	North Approach	South Approach	Total Bikers
11:30-11:45 a.m.	2	0	0	0	2
11:45-12:00 p.m.	0	0	0	0	0
12:00-12:15 p.m.	0	0	0	0	0
12:15-12:30 p.m.	0	0	0	0	0
12:30-12:45 p.m.	0	1	0	1	2
12:45-1:00 p.m.	0	0	0	0	0
1:00-1:15 p.m.	1	0	0	0	1
1:15-1:30 p.m.	0	0	0	0	0
<b>Afternoon Totals</b>	<b>3</b>	<b>1</b>	<b>0</b>	<b>1</b>	<b>5</b>

City: Oak Park

Count Location: Randolph St. and Maple Ave

Study Date: – September 12<sup>th</sup>, 2020 (Pedestrian Crosswalk Counts)

Time	East Crosswalk	West Crosswalk	North Crosswalk	South Crosswalk	Total Pedestrians
11:30-11:45 a.m.	0	4	1	2	7
11:45-12:00 p.m.	2	1	1	1	5
12:00-12:15 p.m.	3	4	7	2	16
12:15-12:30 p.m.	1	1	4	1	7
12:30-12:45 p.m.	1	0	2	2	5
12:45-1:00 p.m.	1	0	1	0	2
1:00-1:15 p.m.	5	3	4	2	14
1:15-1:30 p.m.	2	3	0	0	5
<b>Afternoon Totals</b>	<b>15</b>	<b>16</b>	<b>20</b>	<b>10</b>	<b>61</b>

Westbound Queue Observation Notes:

- During the count there were a few observed westbound backups on Randolph St. extending back to/beyond Maple Ave. These backups were brief (about 20-60 seconds in duration) and each backup cleared quickly with every green light cycle phase. The times of these backups were at 12:57 p.m. (backup beyond Maple of 4 vehicles to the east of the intersection), 12:59 p.m. (backup just past west crosswalk but not blocking intersection), and 1:25 p.m. (backup just up to west crosswalk).
- Please note that none of the observed backups were completely blocking the intersection as north and south thru movements at the intersection could easily be made.

# Site Plan

# CARLETON HOTEL

## PLEASANT STREET

SOUTH MARION STREET



## LEVEL 1 PLAN -

1/32" = 1'-0"

FLOOR PLANS

BOOTH HANSEN

0 8 16 32 64

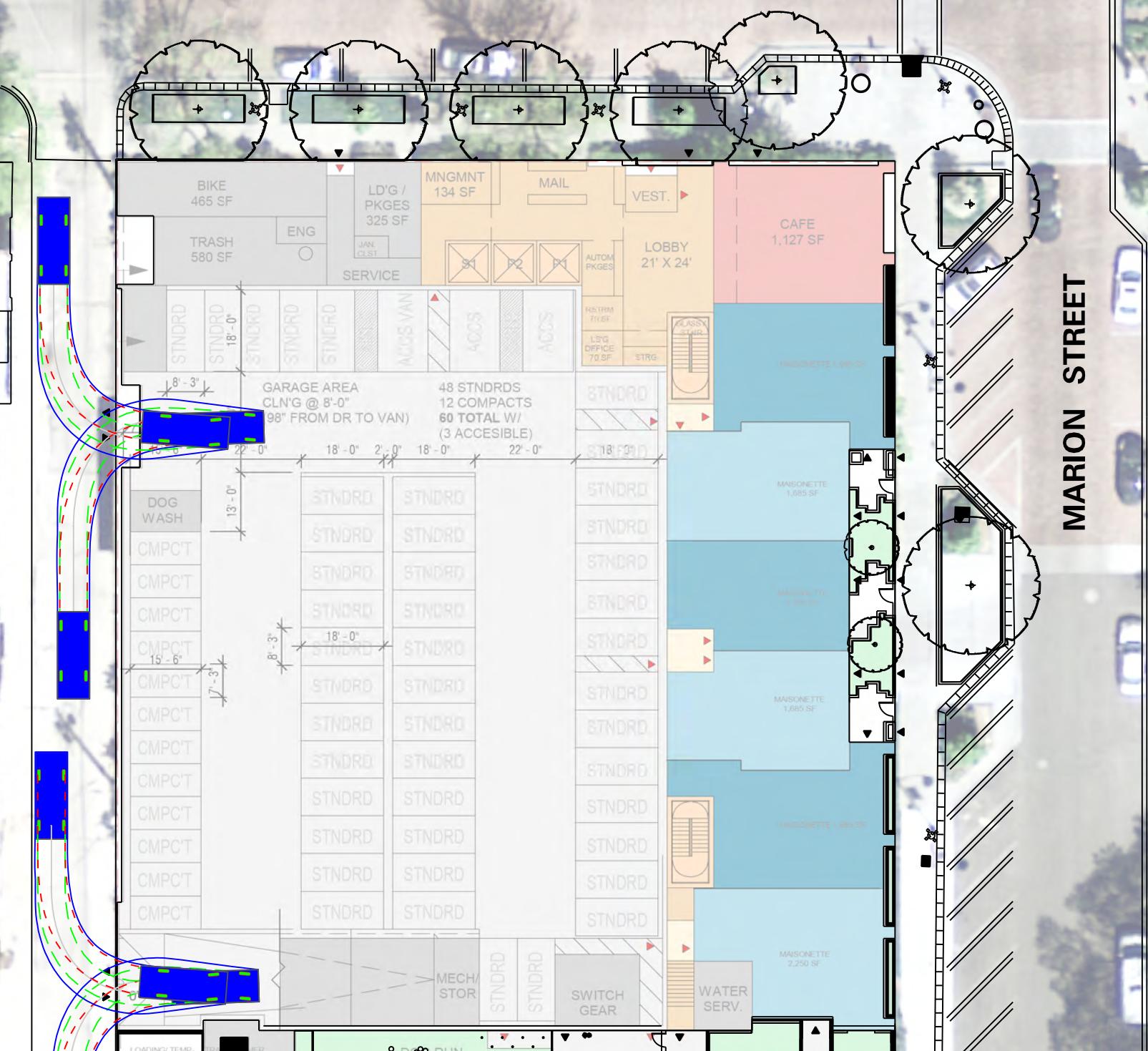
203 S. MARION ST.  
2021 01 19  
2000.00

# AutoTurn Exhibits

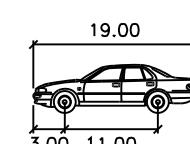


SCALE: 1" = 30'

# PLEASANT STREET



DESIGN VEHICLE



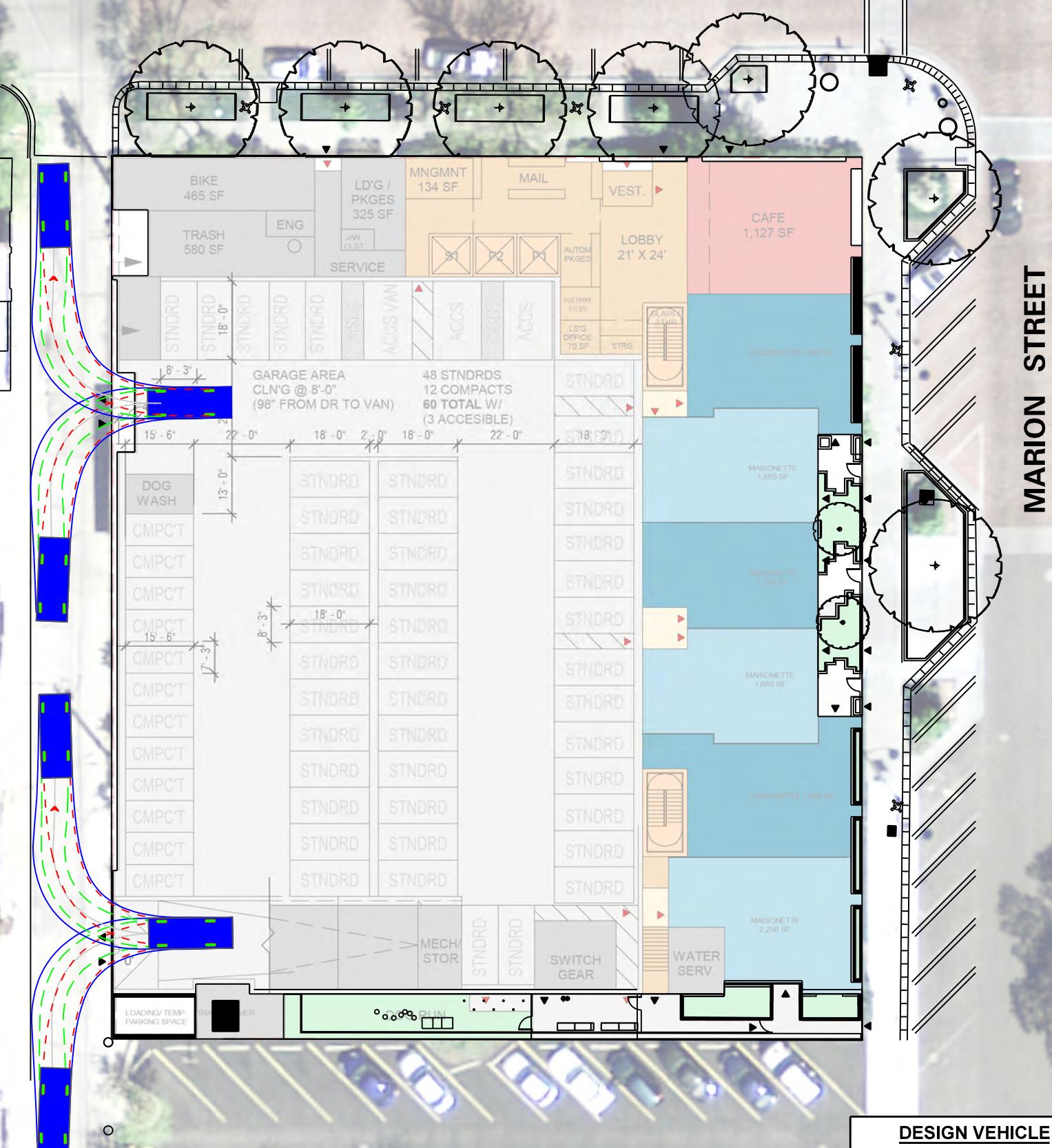
<b>Passenger Vehicle</b>	Feet
Width	: 7.00
Track	: 6.00
Lock to Lock Time	: 6.00

Steering Angle  
Body of Vehicle -  
Front Tires Path -  
Rear Tires Path

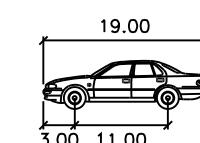


SCALE: 1" = 30'

## PLEASANT STREET



### DESIGN VEHICLE



Passenger Vehicle      Feet  
Width : 7.00  
Track : 6.00  
Lock to Lock Time : 6.00  
Steering Angle : 31.5

Body of Vehicle -  
Front Tires Path -  
Rear Tires Path -

# Census Data

[LOG OUT](#)[USER GUIDE](#)[STATION DOWNLOADS](#)

## Chicago

[Data](#)[Geographies](#)[Close Report](#)Transit Zone:  .25 mile  .5 mile Smart Zoom
 Selected Station
 Station
 Existing Transit
 Potential Transit
 Transit Region

## TOD Report

View as: [Table](#) | [List](#) Download as: [Word](#) | [Excel](#) | [CSV](#)**Chicago Transit Region:**

Average Travel Time to Work: <sup>(3)</sup>	31.34
Median Household Income 2009: <sup>(4)</sup>	61,502
Percent who take public transportation 2009: <sup>(5)</sup>	12.40
Percent who bicycle 2009: <sup>(6)</sup>	0.55
Percent who walk 2009: <sup>(7)</sup>	3.16
Percent who take public transportation, bicycle or walk 2009: <sup>(8)</sup>	16.10
Average number of vehicles available per household 2009: <sup>(9)</sup>	1.62
Average number of vehicles available per household 2009: Owner Occupied: <sup>(10)</sup>	1.91
Average number of vehicles available per household 2009: Renter Occupied: <sup>(11)</sup>	1.02
Percent of households with 0 or 1 vehicle available 2009: <sup>(12)</sup>	47.62
Median Year Structure Built 2009: <sup>(13)</sup>	1,965
<b>Station .25 Mile Transit Zone: CTA Green Line; Harlem-Green</b>	
Year Opened: <sup>(1)</sup>	Pre-2000
Latitude: <sup>(2)</sup>	41.886848
Longitude: <sup>(2)</sup>	-87.803176
Average Travel Time to Work: <sup>(3)</sup>	30.62
Median Household Income 2009: <sup>(4)</sup>	50,046
Percent who take public transportation 2009: <sup>(5)</sup>	31.17
Percent who bicycle 2009: <sup>(6)</sup>	0.71
Percent who walk 2009: <sup>(7)</sup>	9.30
Percent who take public transportation, bicycle or walk 2009: <sup>(8)</sup>	41.18
Average number of vehicles available per household 2009: <sup>(9)</sup>	0.94
Average number of vehicles available per household 2009: Owner Occupied: <sup>(10)</sup>	1.36
Average number of vehicles available per household 2009: Renter Occupied: <sup>(11)</sup>	0.70
Percent of households with 0 or 1 vehicle available 2009: <sup>(12)</sup>	79.12
Median Year Structure Built 2009: <sup>(13)</sup>	1,958

<sup>1</sup> The year in which this station opened. This value is intended to inform the analysis of available statistics, and therefore all stations open prior to 2000 report as "Pre-2000", the year of the earliest available statistic.

<sup>2</sup> Station location, current as of October 1, 2020. Station locations are updated (as necessary) on a quarterly basis which may result in changes in aggregated data.

<sup>3</sup> American Community Survey 2005-2009 5-Year Estimates b08013\_001 / b08132\_001 aggregated from Census 2009 Tracts

<sup>4</sup> American Community Survey 2005-2009 5-Year Estimates b19013\_001 aggregated from Census 2009 Block Groups

<sup>5</sup> American Community Survey 2005-2009 5-Year Estimates (b08301\_010) / (b08301\_001) aggregated from Census 2009 Block Groups

<sup>6</sup> American Community Survey 2005-2009 5-Year Estimates (b08301\_018) / (b08301\_001) aggregated from Census 2009 Block Groups

<sup>7</sup> American Community Survey 2005-2009 5-Year Estimates (b08301\_019) / (b08301\_001) aggregated from Census 2009 Block Groups

<sup>8</sup> American Community Survey 2005-2009 5-Year Estimates (b08301\_010 + b08301\_018 + b08301\_019) / (b08301\_001) aggregated from Census 2009 Block Groups

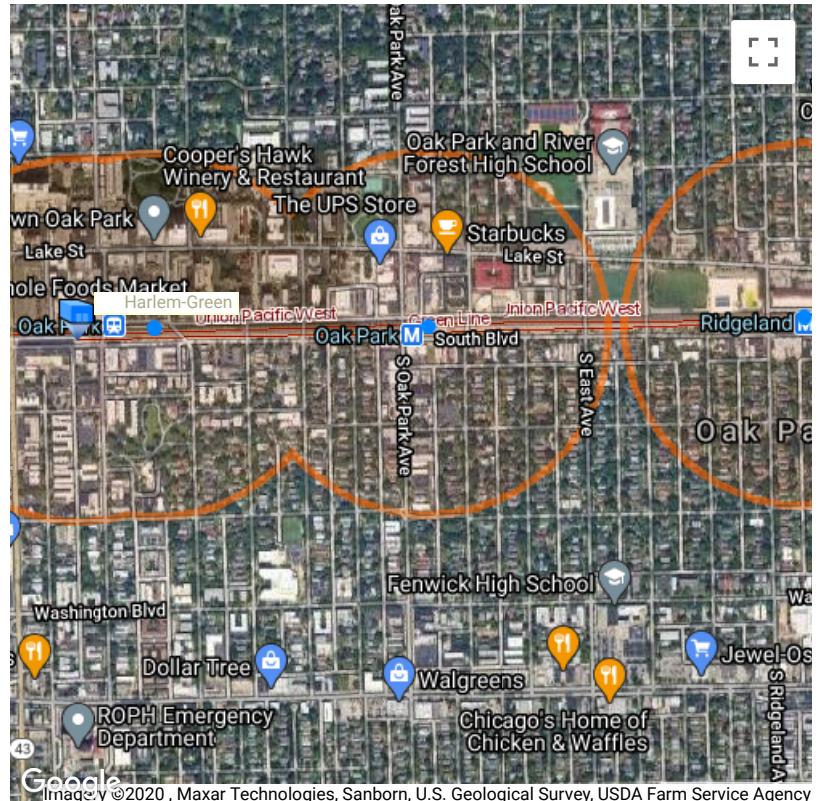
<sup>9</sup> American Community Survey 2005-2009 5-Year Estimates b25046\_001 / b25044\_001 aggregated from Census 2009 Block Groups

<sup>10</sup> American Community Survey 2005-2009 5-Year Estimates b25046\_002 / b25044\_002 aggregated from Census 2009 Block Groups

<sup>11</sup> American Community Survey 2005-2009 5-Year Estimates b25046\_003 / b25044\_009 aggregated from Census 2009 Block Groups

<sup>12</sup> American Community Survey 2005-2009 5-Year Estimates (b25044\_003+b25044\_004+b25044\_010+b25044\_011) / b25044\_001 aggregated from Census 2009 Block Groups

<sup>13</sup> American Community Survey 2005-2009 5-Year Estimates b25035\_001 aggregated from Census 2009 Block Groups



# ITE Trip Generation Sheets

## Multifamily Housing (Low-Rise) (220)

Vehicle Trip Ends vs: Dwelling Units  
On a: Weekday

Setting/Location: General Urban/Suburban

Number of Studies: 29

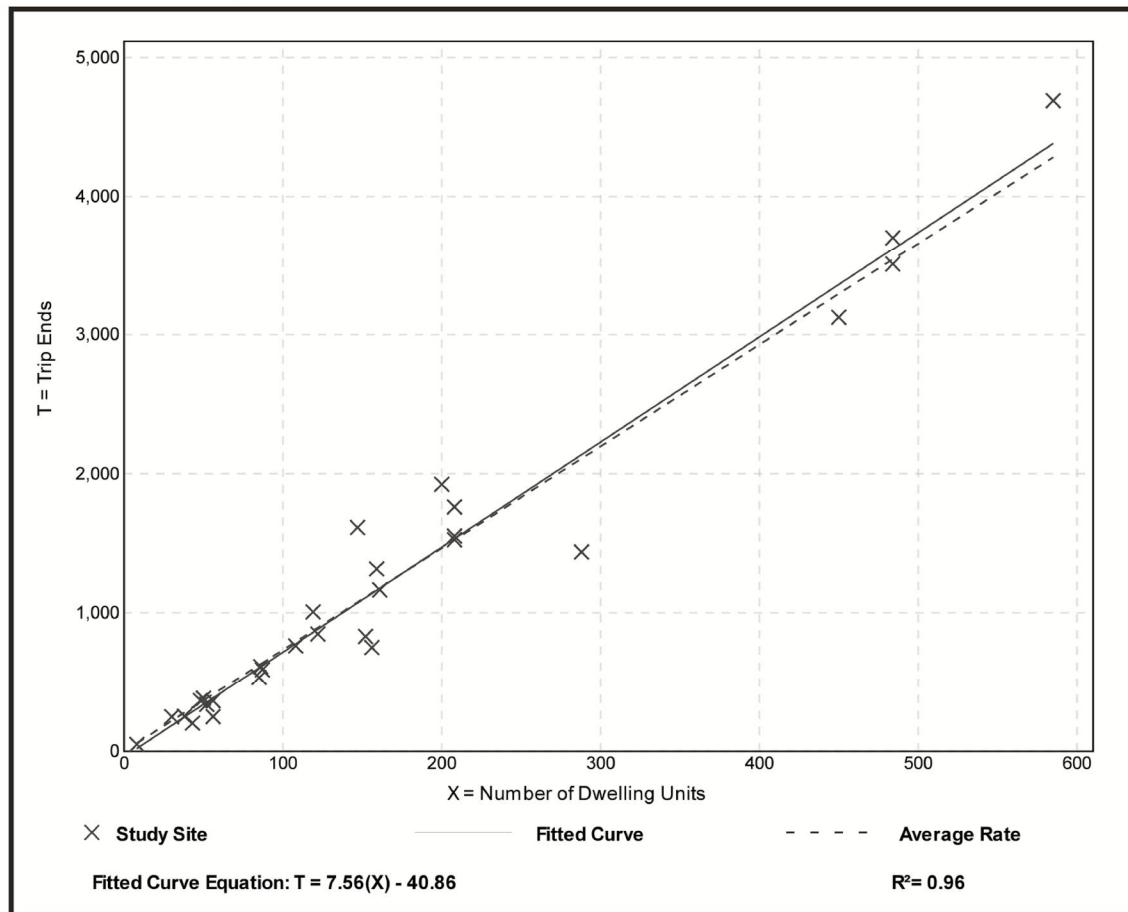
Avg. Num. of Dwelling Units: 168

Directional Distribution: 50% entering, 50% exiting

### Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
7.32	4.45 - 10.97	1.31

### Data Plot and Equation



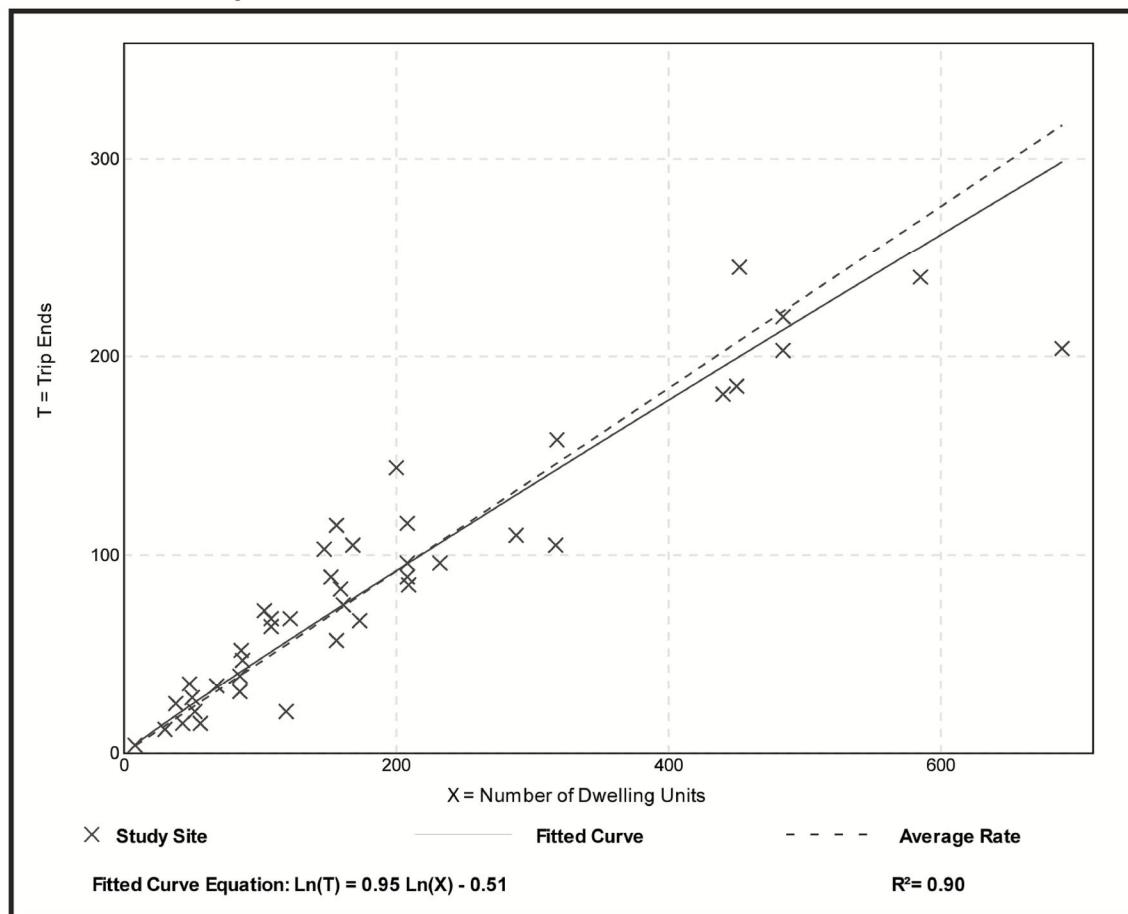
## Multifamily Housing (Low-Rise) (220)

**Vehicle Trip Ends vs:** Dwelling Units  
**On a:** Weekday,  
Peak Hour of Adjacent Street Traffic,  
One Hour Between 7 and 9 a.m.  
**Setting/Location:** General Urban/Suburban  
Number of Studies: 42  
Avg. Num. of Dwelling Units: 199  
Directional Distribution: 23% entering, 77% exiting

### Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.46	0.18 - 0.74	0.12

### Data Plot and Equation



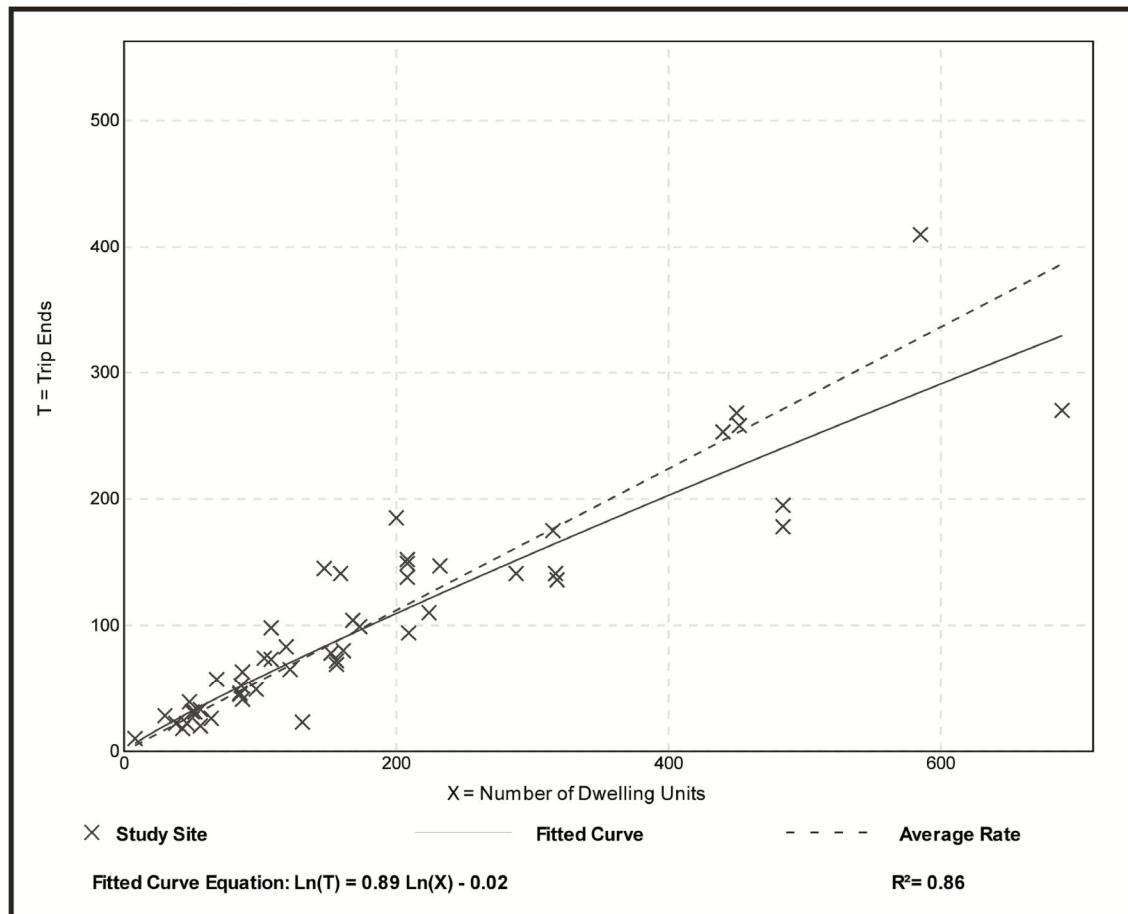
## Multifamily Housing (Low-Rise) (220)

**Vehicle Trip Ends vs:** Dwelling Units  
**On a:** Weekday,  
Peak Hour of Adjacent Street Traffic,  
One Hour Between 4 and 6 p.m.  
**Setting/Location:** General Urban/Suburban  
Number of Studies: 50  
Avg. Num. of Dwelling Units: 187  
Directional Distribution: 63% entering, 37% exiting

### Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.56	0.18 - 1.25	0.16

### Data Plot and Equation



## Multifamily Housing (Low-Rise) (220)

Vehicle Trip Ends vs: Dwelling Units  
On a: Saturday, Peak Hour of Generator

Setting/Location: General Urban/Suburban

Number of Studies: 5

Avg. Num. of Dwelling Units: 89

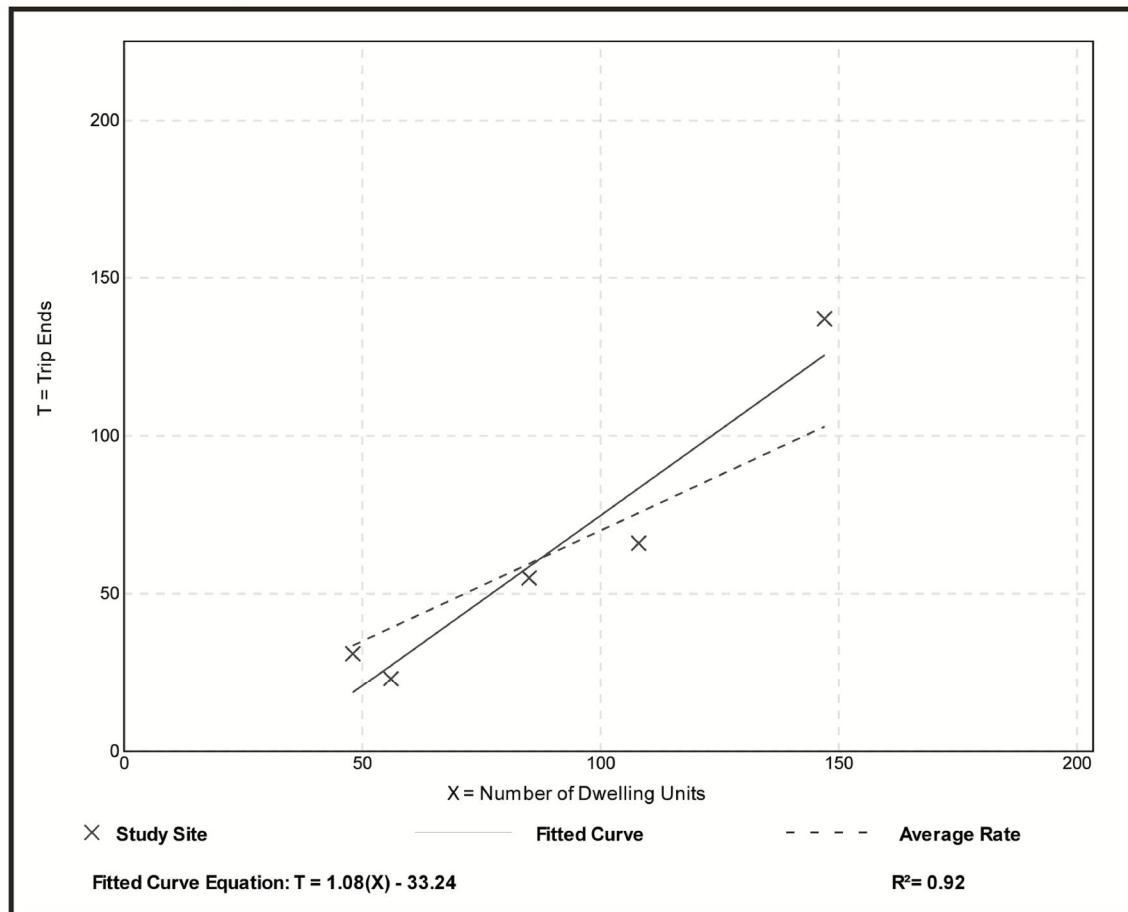
Directional Distribution: Not Available

### Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.70	0.41 - 0.93	0.20

### Data Plot and Equation

*Caution – Small Sample Size*



## Multifamily Housing (Mid-Rise) (221)

Vehicle Trip Ends vs: Dwelling Units  
On a: Weekday

Setting/Location: General Urban/Suburban

Number of Studies: 27

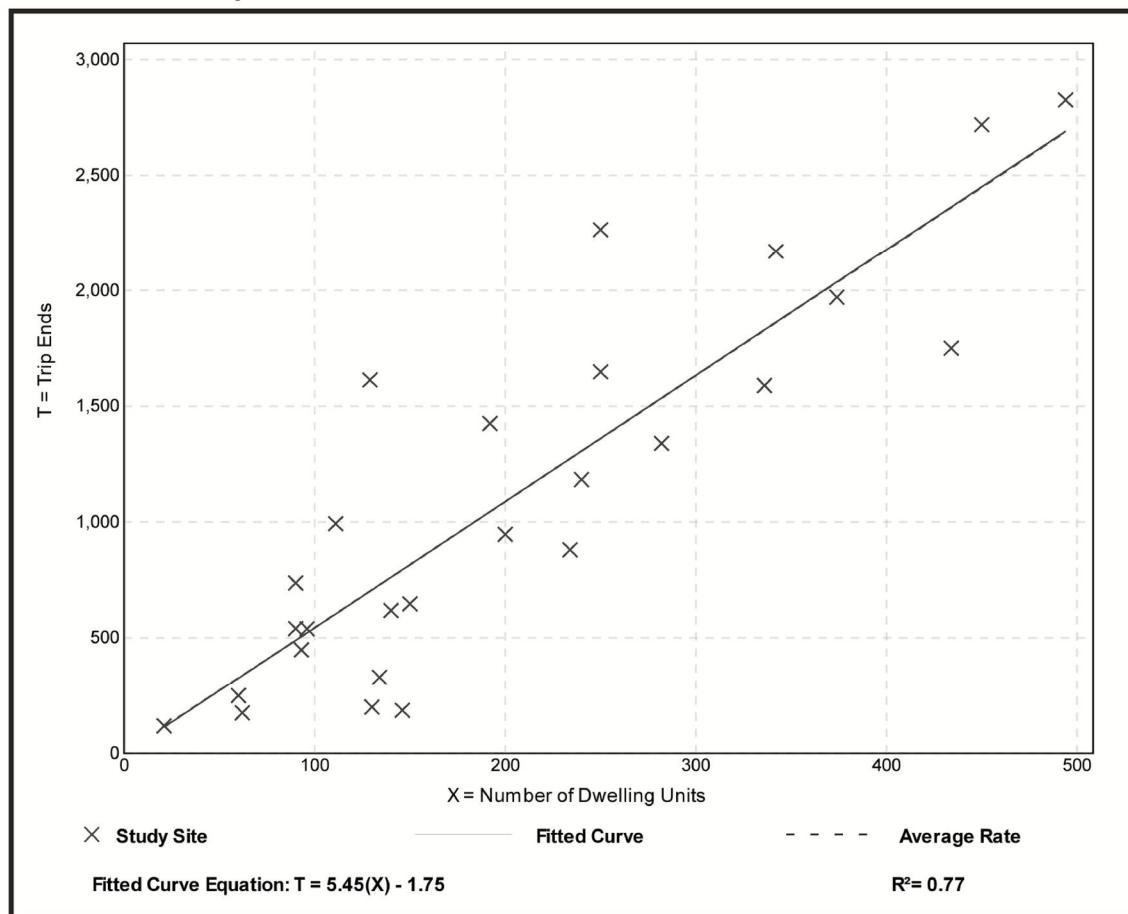
Avg. Num. of Dwelling Units: 205

Directional Distribution: 50% entering, 50% exiting

### Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
5.44	1.27 - 12.50	2.03

### Data Plot and Equation



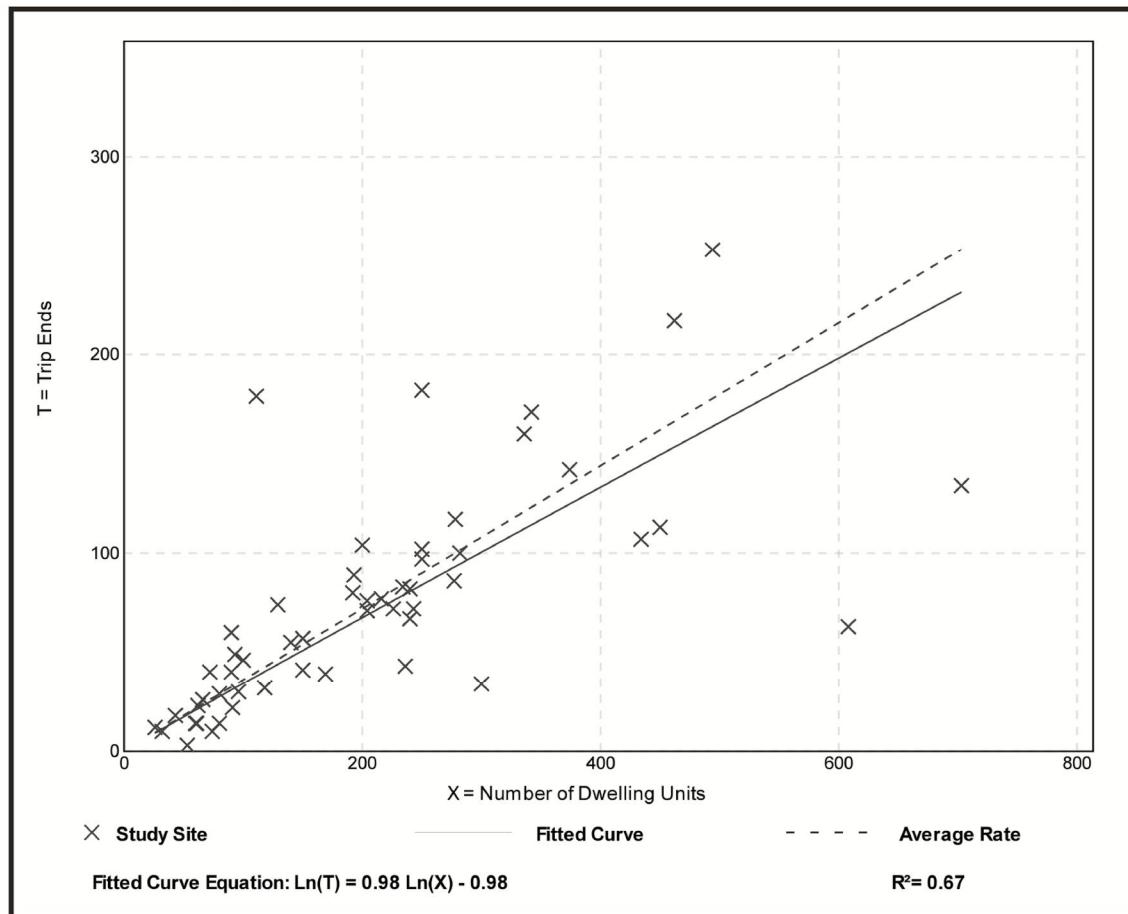
## Multifamily Housing (Mid-Rise) (221)

**Vehicle Trip Ends vs:** Dwelling Units  
**On a:** Weekday,  
Peak Hour of Adjacent Street Traffic,  
One Hour Between 7 and 9 a.m.  
**Setting/Location:** General Urban/Suburban  
Number of Studies: 53  
Avg. Num. of Dwelling Units: 207  
Directional Distribution: 26% entering, 74% exiting

### Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.36	0.06 - 1.61	0.19

### Data Plot and Equation



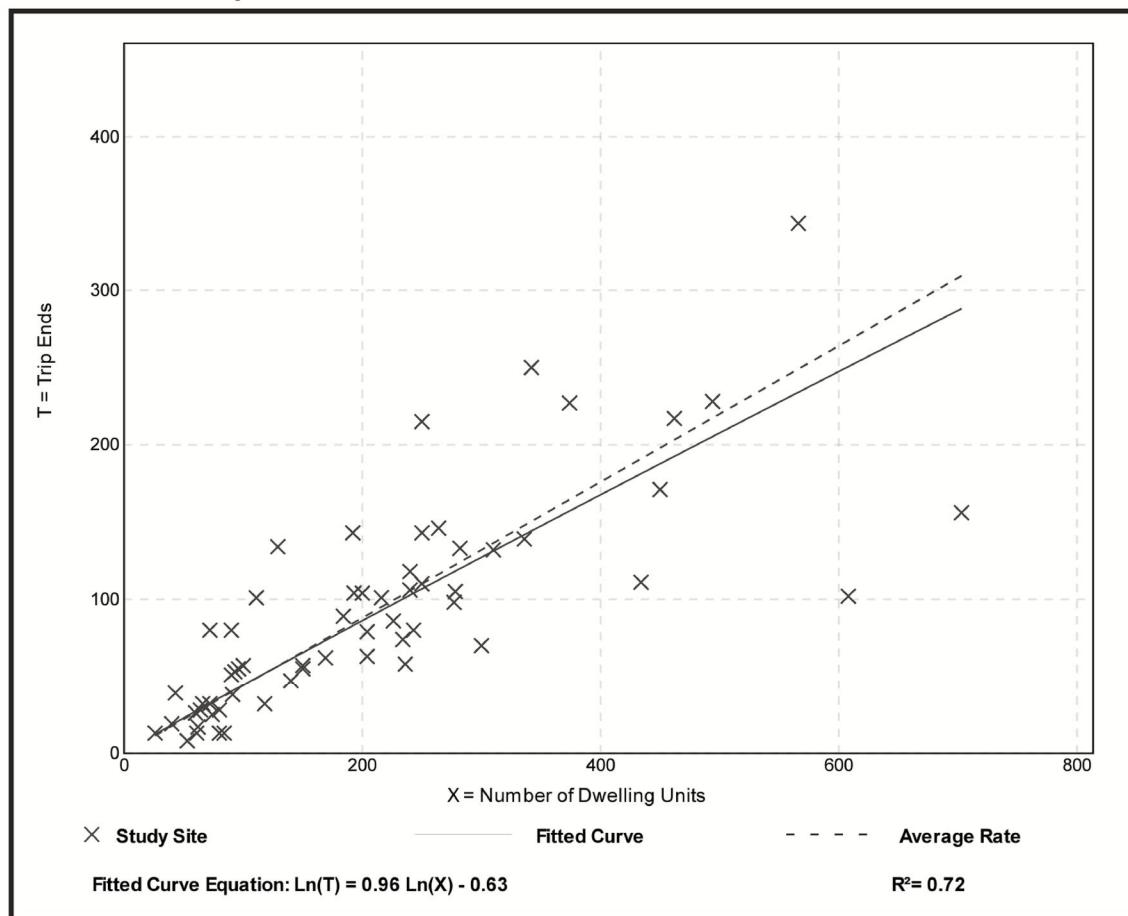
## Multifamily Housing (Mid-Rise) (221)

**Vehicle Trip Ends vs:** Dwelling Units  
**On a:** Weekday,  
Peak Hour of Adjacent Street Traffic,  
One Hour Between 4 and 6 p.m.  
**Setting/Location:** General Urban/Suburban  
Number of Studies: 60  
Avg. Num. of Dwelling Units: 208  
Directional Distribution: 61% entering, 39% exiting

### Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.44	0.15 - 1.11	0.19

### Data Plot and Equation



## Multifamily Housing (Mid-Rise) (221)

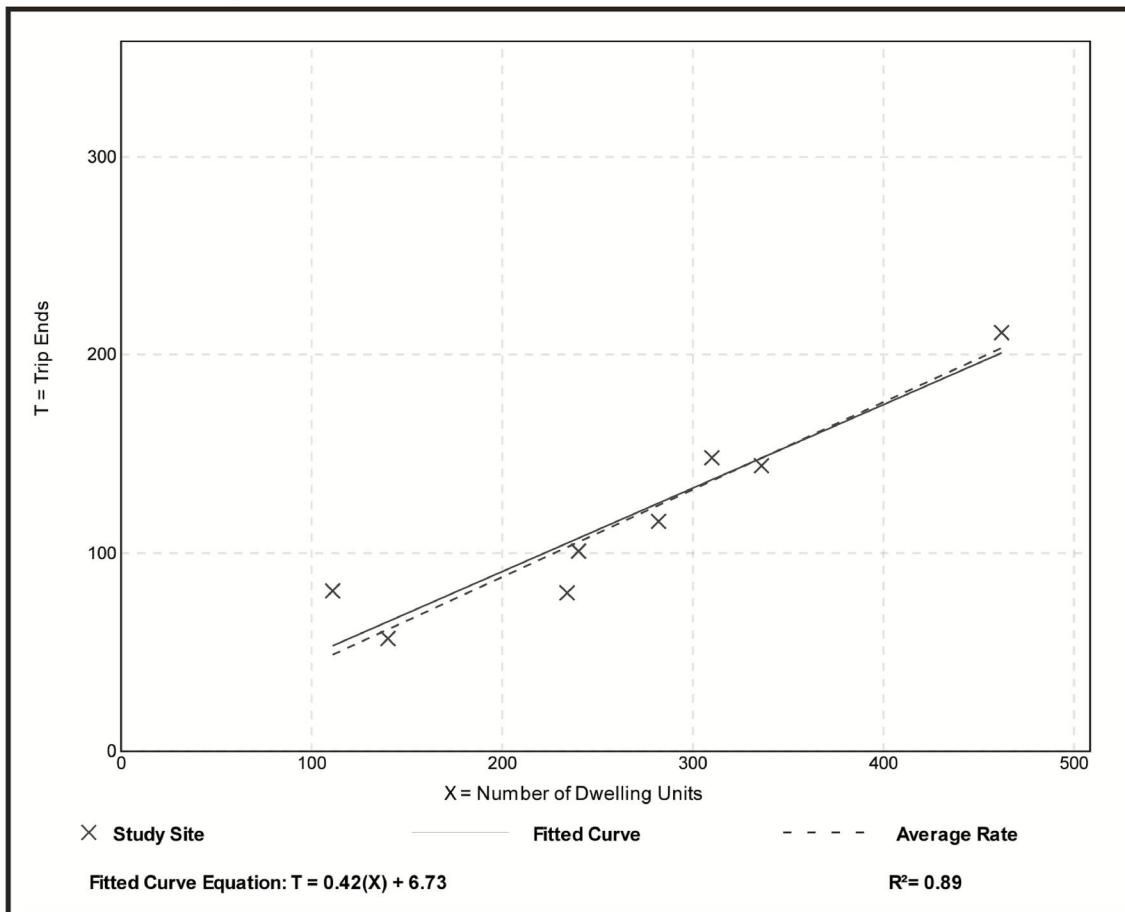
Vehicle Trip Ends vs: Dwelling Units  
On a: Saturday, Peak Hour of Generator

Setting/Location: General Urban/Suburban  
Number of Studies: 8  
Avg. Num. of Dwelling Units: 264  
Directional Distribution: 49% entering, 51% exiting

### Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.44	0.34 - 0.73	0.08

### Data Plot and Equation



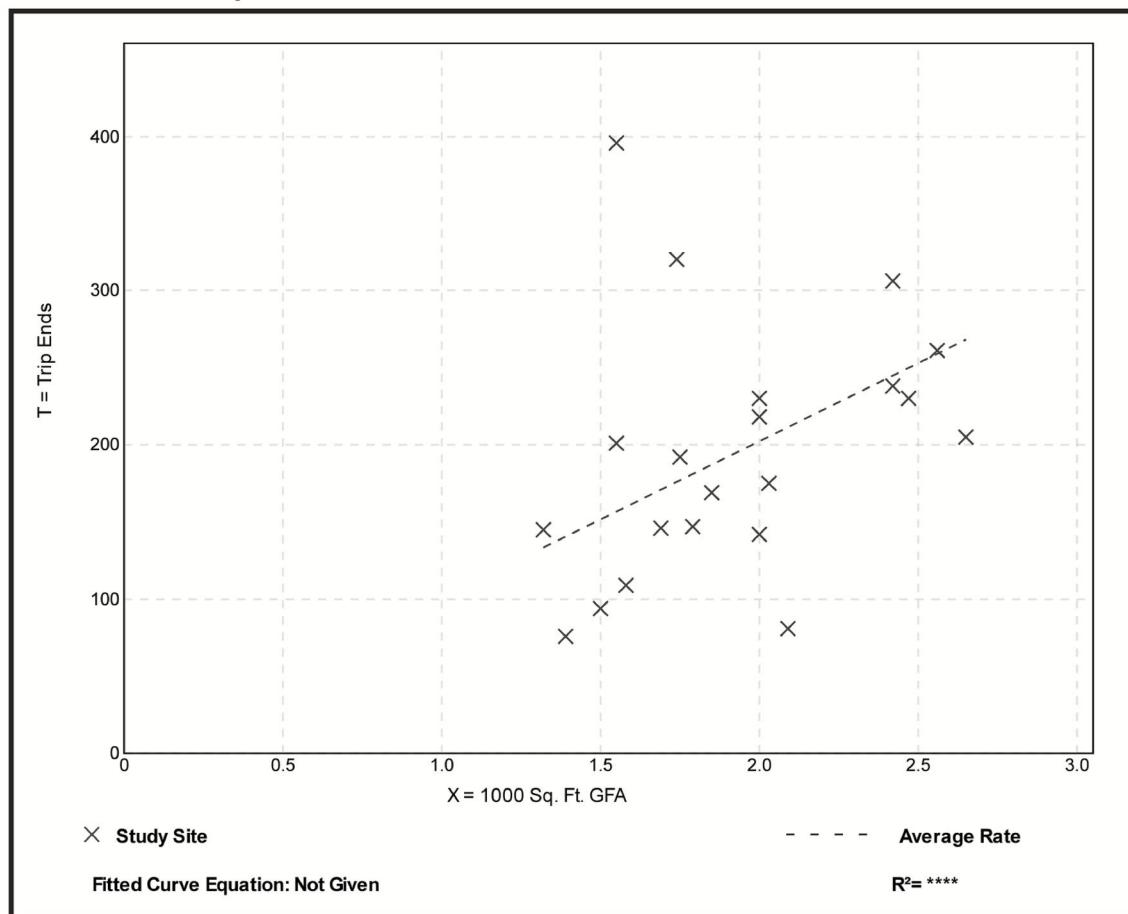
## Coffee/Donut Shop without Drive-Through Window (936)

**Vehicle Trip Ends vs:** 1000 Sq. Ft. GFA  
**On a:** Weekday,  
Peak Hour of Adjacent Street Traffic,  
One Hour Between 7 and 9 a.m.  
**Setting/Location:** General Urban/Suburban  
Number of Studies: 21  
1000 Sq. Ft. GFA: 2  
Directional Distribution: 51% entering, 49% exiting

### Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
101.14	38.76 - 255.48	43.44

### Data Plot and Equation



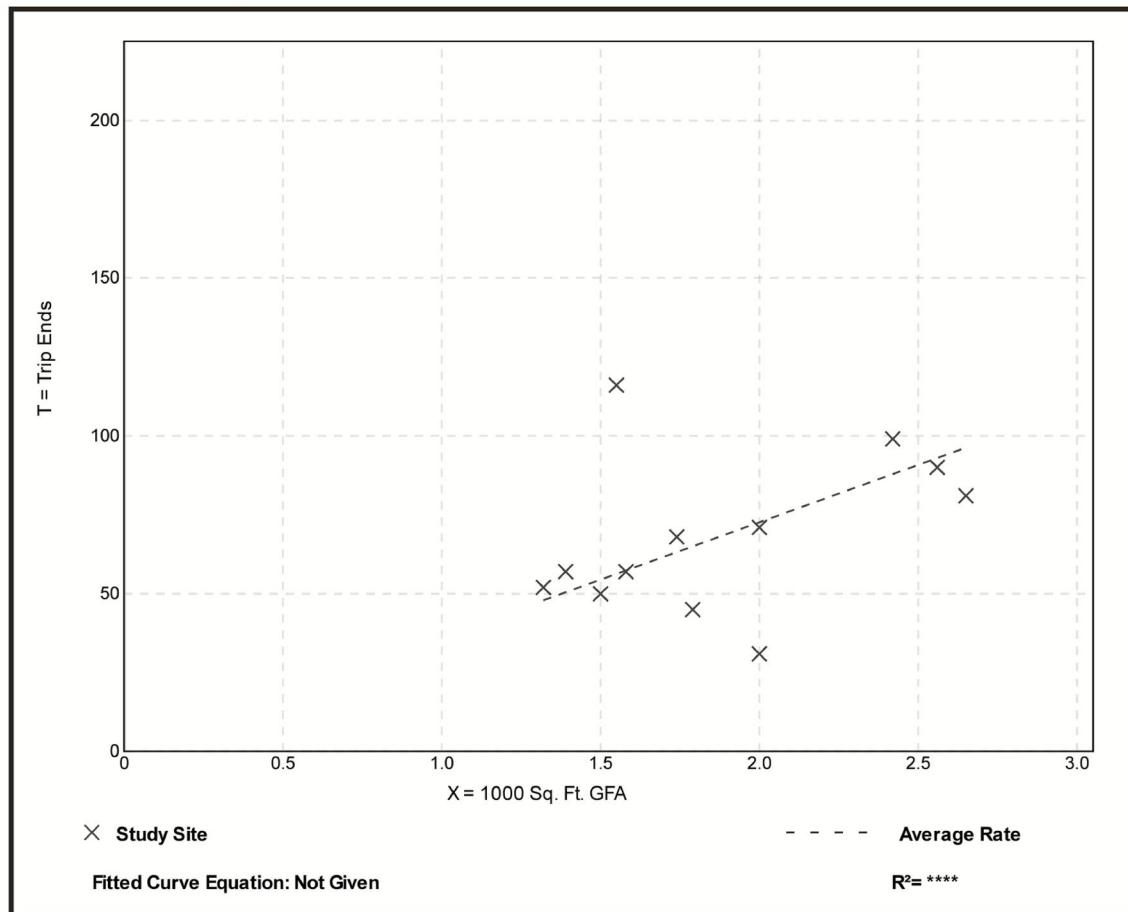
## Coffee/Donut Shop without Drive-Through Window (936)

**Vehicle Trip Ends vs:** 1000 Sq. Ft. GFA  
**On a:** Weekday,  
Peak Hour of Adjacent Street Traffic,  
One Hour Between 4 and 6 p.m.  
**Setting/Location:** General Urban/Suburban  
Number of Studies: 12  
1000 Sq. Ft. GFA: 2  
Directional Distribution: 50% entering, 50% exiting

### Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
36.31	15.50 - 74.84	13.22

### Data Plot and Equation



## Coffee/Donut Shop without Drive-Through Window (936)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA  
On a: Saturday, Peak Hour of Generator

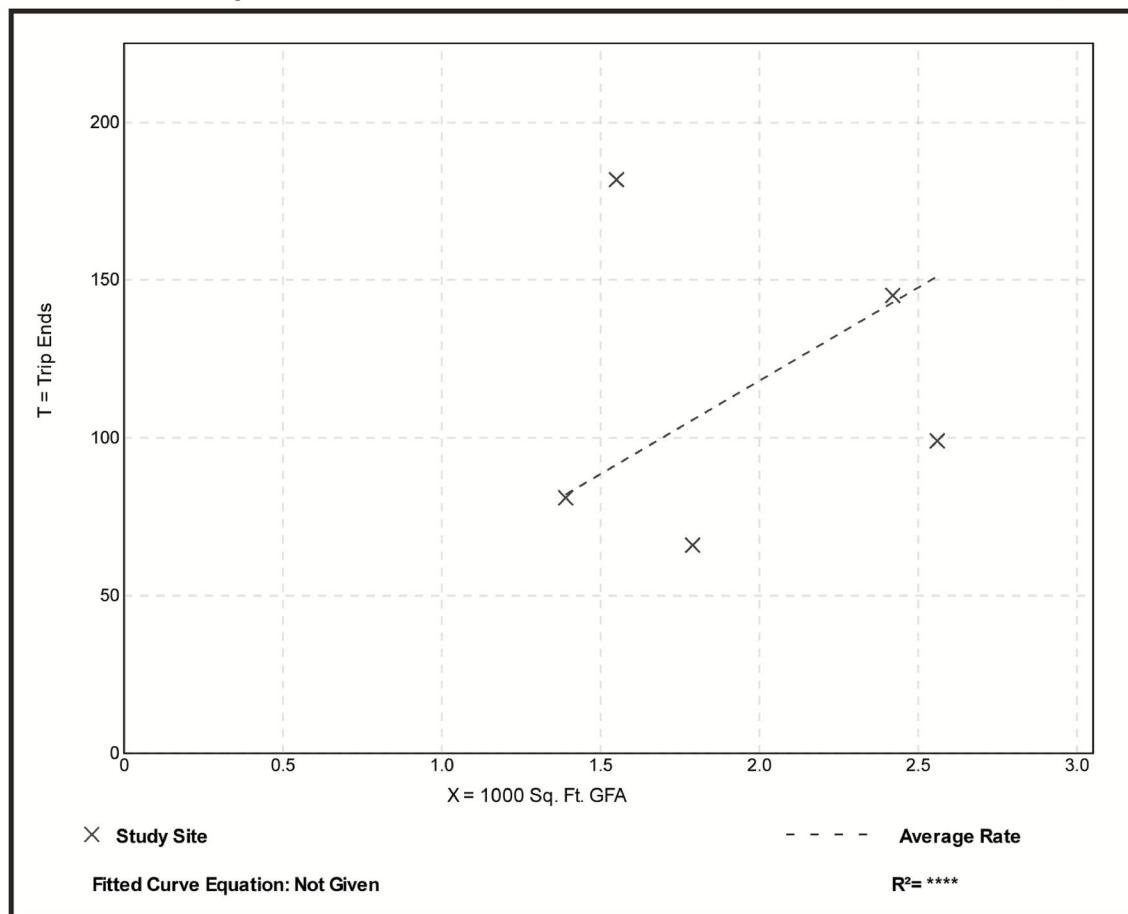
Setting/Location: General Urban/Suburban  
Number of Studies: 5  
1000 Sq. Ft. GFA: 2  
Directional Distribution: 49% entering, 51% exiting

### Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
59.01	36.87 - 117.42	30.50

### Data Plot and Equation

*Caution – Small Sample Size*



# CMAP Projections Letter



# Chicago Metropolitan Agency for Planning

233 South Wacker Drive  
Suite 800  
Chicago, Illinois 60606

312 454 0400  
[www.cmap.illinois.gov](http://www.cmap.illinois.gov)

October 1, 2020

Elise Purguette  
Traffic Engineer  
Kenig, Lindgren, O'Hara and Aboona, Inc.  
9575 West Higgins Road  
Suite 400  
Rosemont IL, 60018

**Subject: *Harlem Avenue - Randolph Street - Marion Street***  
IDOT

Dear Ms. Purguette:

In response to a request made on your behalf and dated September 30, 2020, we have developed year 2050 average daily traffic (ADT) projections for the subject location.

ROAD SEGMENT	Current Volumes	Year 2050 ADT
Harlem Ave from South Ave to Randolph St	35,300	38,800
Harlem Ave south of Randolph St	35,400	38,900
Harlem Ave north of South Blvd	35,200	38,700
Circle Ave (@ Harlem Ave)	4,550	5,000
Randolph St (@ Harlem Ave)	8,550	9,400
Marion Street	2,200	2,420

Traffic projections are developed using existing ADT data provided in the request letter and the results from the March 2020 CMAP Travel Demand Analysis. The regional travel model uses CMAP 2050 socioeconomic projections and assumes the implementation of the ON TO 2050 Comprehensive Regional Plan for the Northeastern Illinois area. The provision of this data in support of your request does not constitute a CMAP endorsement of the proposed development or any subsequent developments.

If you have any questions, please call me at (312) 386-8806.

Sincerely,

A handwritten signature in black ink, appearing to read "Jose Rodriguez".

Jose Rodriguez, PTP, AICP  
Senior Planner, Research & Analysis

cc: Quigley (IDOT)  
2020\_TrafficForecast\OakPark\ck-100-20\ck-100-20.docx

# Level of Service Criteria

## LEVEL OF SERVICE CRITERIA

Signalized Intersections		
Level of Service	Interpretation	Average Control Delay (seconds per vehicle)
A	Favorable progression. Most vehicles arrive during the green indication and travel through the intersection without stopping.	$\leq 10$
B	Good progression, with more vehicles stopping than for Level of Service A.	>10 - 20
C	Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear. Number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.	>20 - 35
D	The volume-to-capacity ratio is high and either progression is ineffective or the cycle length is too long. Many vehicles stop and individual cycle failures are noticeable.	>35 - 55
E	Progression is unfavorable. The volume-to-capacity ratio is high and the cycle length is long. Individual cycle failures are frequent.	>55 - 80
F	The volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.	>80.0
Unsignalized Intersections		
Level of Service	Average Total Delay (SEC/VEH)	
A	$0 - 10$	
B	> 10 - 15	
C	> 15 - 25	
D	> 25 - 35	
E	> 35 - 50	
F	> 50	

Source: *Highway Capacity Manual*, 2010.

# Capacity Analysis Summary Sheets

Lanes, Volumes, Timings  
1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑↑			↑↑
Traffic Volume (vph)	138	76	1223	16	0	1383
Future Volume (vph)	138	76	1223	16	0	1383
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	2000
Storage Length (ft)	110	0		0	0	
Storage Lanes	1	1		0	0	
Taper Length (ft)	145			25		
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Ped Bike Factor	0.93	0.97	1.00			
Fr <sub>t</sub>		0.850	0.998			
Flt Protected	0.950					
Satd. Flow (prot)	1752	1225	3363	0	0	3623
Flt Permitted	0.950					
Satd. Flow (perm)	1629	1189	3363	0	0	3623
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	30		30			30
Link Distance (ft)	465		176			249
Travel Time (s)	10.6		4.0			5.7
Confl. Peds. (#/hr)	47	11		24	24	
Confl. Bikes (#/hr)		2				
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	14%	7%	7%	0%	3%
Bus Blockages (#/hr)	0	0	0	0	0	9
Parking (#/hr)		7				
Shared Lane Traffic (%)						
Lane Group Flow (vph)	150	83	1346	0	0	1503
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2			6
Permitted Phases		8				
Detector Phase	8	8	2			6
Switch Phase						
Minimum Initial (s)	8.0	8.0	15.0			15.0
Minimum Split (s)	31.0	31.0	28.0			29.0
Total Split (s)	38.0	38.0	87.0			87.0
Total Split (%)	30.4%	30.4%	69.6%			69.6%
Yellow Time (s)	4.0	4.0	4.0			4.0
All-Red Time (s)	2.0	2.0	2.0			2.0
Lost Time Adjust (s)	0.0	0.0	0.0			0.0
Total Lost Time (s)	6.0	6.0	6.0			6.0
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Min			C-Min
Act Effct Green (s)	16.9	16.9	96.1			96.1
Actuated g/C Ratio	0.14	0.14	0.77			0.77
v/c Ratio	0.63	0.52	0.52			0.54
Control Delay	62.6	60.8	12.8			7.0
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	62.6	60.8	12.8			7.0

# Lanes, Volumes, Timings

## 1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
LOS	E	E	B			A
Approach Delay	61.9		12.8			7.0
Approach LOS	E		B			A
Queue Length 50th (ft)	116	63	445			215
Queue Length 95th (ft)	179	112	578			323
Internal Link Dist (ft)	385		96			169
Turn Bay Length (ft)	110					
Base Capacity (vph)	448	304	2585			2784
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.33	0.27	0.52			0.54

### Intersection Summary

Area Type: Other

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 103 (82%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.63

Intersection Signal Delay: 13.7

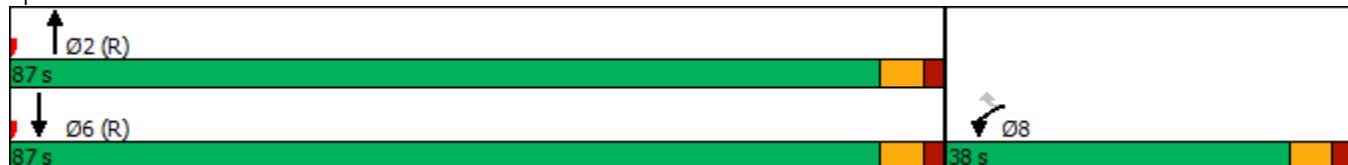
Intersection LOS: B

Intersection Capacity Utilization 58.0%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: Harlem Avenue & South Boulevard



## Lanes, Volumes, Timings

## 2: Harlem Avenue &amp; Randolph Street

10/06/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↓		↑	↓		↑	↑↓		↑	↑↓	
Traffic Volume (vph)	128	146	117	74	108	24	82	1136	24	31	1301	81
Future Volume (vph)	128	146	117	74	108	24	82	1136	24	31	1301	81
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	2000	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)	0%			0%			0%			0%		
Storage Length (ft)	40			0	50		0	165		0	85	0
Storage Lanes	1			0	1		0	1		0	1	0
Taper Length (ft)	90				75			120			90	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	0.99	0.99			0.99	0.99			1.00			1.00
Fr <sub>t</sub>	0.933			0.973			0.997			0.991		
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1787	1724	0	1805	1807	0	1736	3539	0	1736	3577	0
Flt Permitted	0.526				0.324			0.079			0.163	
Satd. Flow (perm)	976	1724	0	611	1807	0	144	3539	0	298	3577	0
Right Turn on Red				No			No			No		
Satd. Flow (RTOR)												
Link Speed (mph)	30			30			30			30		
Link Distance (ft)	454			257			242			813		
Travel Time (s)	10.3			5.8			5.5			18.5		
Confl. Peds. (#/hr)	12		10	10			12	6		6	6	6
Confl. Bikes (#/hr)												
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	2%	1%	0%	2%	0%	4%	7%	5%	4%	3%	6%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	9	0
Parking (#/hr)												
Mid-Block Traffic (%)	0%			0%			0%			0%		
Shared Lane Traffic (%)												
Lane Group Flow (vph)	133	274	0	77	138	0	85	1208	0	32	1439	0
Turn Type	pm+pt	NA										
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	3.0	8.0		3.0	8.0		3.0	15.0		3.0	15.0	
Minimum Split (s)	9.5	30.0		9.5	32.0		9.5	29.0		10.5	33.0	
Total Split (s)	14.0	30.0		14.0	30.0		17.0	67.0		14.0	64.0	
Total Split (%)	11.2%	24.0%		11.2%	24.0%		13.6%	53.6%		11.2%	51.2%	
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0		3.5	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	
Lead/Lag	Lead	Lag										
Lead-Lag Optimize?	Yes	Yes										
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effct Green (s)	34.8	23.9		32.1	20.7		80.5	72.5		76.0	67.1	
Actuated g/C Ratio	0.28	0.19		0.26	0.17		0.64	0.58		0.61	0.54	

# Lanes, Volumes, Timings

## 2: Harlem Avenue & Randolph Street

10/06/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.39	0.83		0.32	0.46		0.43	0.59		0.13	0.75	
Control Delay	36.1	70.5		34.5	51.3		16.5	20.0		12.5	33.6	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	36.1	70.5		34.5	51.3		16.5	20.0		12.5	33.6	
LOS	D	E		C	D		B	C		B	C	
Approach Delay		59.2			45.3			19.8			33.1	
Approach LOS		E			D			B			C	
Queue Length 50th (ft)	78	212		44	99		26	360		13	571	
Queue Length 95th (ft)	131	#357		82	164		49	446		m23	670	
Internal Link Dist (ft)		374			177			162			733	
Turn Bay Length (ft)	40		50			165				85		
Base Capacity (vph)	339	334		265	346		265	2051		311	1918	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.39	0.82		0.29	0.40		0.32	0.59		0.10	0.75	

### Intersection Summary

Area Type: Other

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 51 (41%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.83

Intersection Signal Delay: 31.9

Intersection LOS: C

Intersection Capacity Utilization 78.6%

ICU Level of Service D

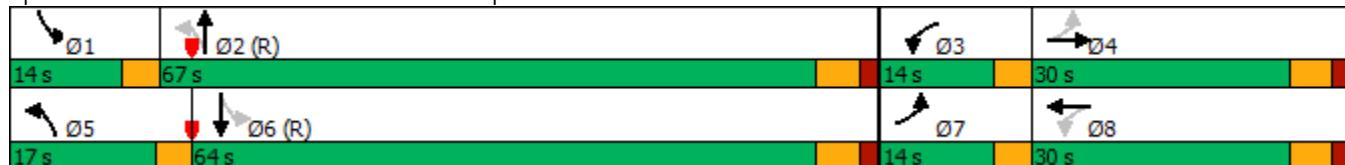
Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Harlem Avenue & Randolph Street



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Intersection

Intersection Delay, s/veh 7.6

Intersection LOS A

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	9	63	14	8	19	0	16	7	18	2	2	2
Future Vol, veh/h	9	63	14	8	19	0	16	7	18	2	2	2
Peak Hour Factor	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67
Heavy Vehicles, %	0	0	0	0	0	0	0	25	12	0	0	14
Mvmt Flow	13	94	21	12	28	0	24	10	27	3	3	3
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	7.7			7.4			7.4			7.2		
HCM LOS	A			A			A			A		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	39%	10%	30%	33%
Vol Thru, %	17%	73%	70%	33%
Vol Right, %	44%	16%	0%	33%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	41	86	27	6
LT Vol	16	9	8	2
Through Vol	7	63	19	2
RT Vol	18	14	0	2
Lane Flow Rate	61	128	40	9
Geometry Grp	1	1	1	1
Degree of Util (X)	0.068	0.142	0.047	0.01
Departure Headway (Hd)	4.013	3.975	4.179	4.106
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	881	899	851	858
Service Time	2.089	2.016	2.235	2.195
HCM Lane V/C Ratio	0.069	0.142	0.047	0.01
HCM Control Delay	7.4	7.7	7.4	7.2
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	0.2	0.5	0.1	0

**Intersection**

Intersection Delay, s/veh

9

Intersection LOS

A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	26	156	24	6	164	12	17	39	4	5	49	35
Future Vol, veh/h	26	156	24	6	164	12	17	39	4	5	49	35
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	29	173	27	7	182	13	19	43	4	6	54	39
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	9.3			9.1			8.6			8.5		
HCM LOS	A			A			A			A		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	28%	13%	3%	6%
Vol Thru, %	65%	76%	90%	55%
Vol Right, %	7%	12%	7%	39%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	60	206	182	89
LT Vol	17	26	6	5
Through Vol	39	156	164	49
RT Vol	4	24	12	35
Lane Flow Rate	67	229	202	99
Geometry Grp	1	1	1	1
Degree of Util (X)	0.093	0.285	0.254	0.13
Departure Headway (Hd)	5.022	4.488	4.529	4.739
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	711	799	793	754
Service Time	3.071	2.523	2.564	2.785
HCM Lane V/C Ratio	0.094	0.287	0.255	0.131
HCM Control Delay	8.6	9.3	9.1	8.5
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	0.3	1.2	1	0.4

**Intersection**

Int Delay, s/veh 0.2

Movement	EBL	EBC	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	18	0	1239	1399	161
Future Vol, veh/h	0	18	0	1239	1399	161
Conflicting Peds, #/hr	11	47	76	0	0	76
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	14	0	7	3	2
Mvmt Flow	0	20	0	1347	1521	175

Major/Minor	Minor2	Major1	Major2	
Conflicting Flow All	-	971	-	0
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	-	7.18	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	-	3.44	-	-
Pot Cap-1 Maneuver	0	232	0	-
Stage 1	0	-	0	-
Stage 2	0	-	0	-
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	-	206	-	-
Mov Cap-2 Maneuver	-	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	24.3	0	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	206	-	-
HCM Lane V/C Ratio	-	0.095	-	-
HCM Control Delay (s)	-	24.3	-	-
HCM Lane LOS	-	C	-	-
HCM 95th %tile Q(veh)	-	0.3	-	-

**Intersection**

Int Delay, s/veh 0.1

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	17	0	1	214	0	2
Future Vol, veh/h	17	0	1	214	0	2
Conflicting Peds, #/hr	0	20	20	0	1	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	96	96	96	96	96	96
Heavy Vehicles, %	8	0	0	9	0	0
Mvmt Flow	18	0	1	223	0	2

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	38	0	264
Stage 1	-	-	-	-	38
Stage 2	-	-	-	-	226
Critical Hdwy	-	-	4.1	-	6.4
Critical Hdwy Stg 1	-	-	-	-	5.4
Critical Hdwy Stg 2	-	-	-	-	5.4
Follow-up Hdwy	-	-	2.2	-	3.5
Pot Cap-1 Maneuver	-	-	1585	-	729
Stage 1	-	-	-	-	990
Stage 2	-	-	-	-	816
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1555	-	714
Mov Cap-2 Maneuver	-	-	-	-	1020
Stage 1	-	-	-	-	714
Stage 2	-	-	-	-	970

Approach	EB	WB	NB
HCM Control Delay, s	0	0	8.5
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	1020	-	-	1555	-
HCM Lane V/C Ratio	0.002	-	-	0.001	-
HCM Control Delay (s)	8.5	-	-	7.3	0
HCM Lane LOS	A	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

## Intersection

Int Delay, s/veh 10.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	0	0	48	6	5	26	33	1213	42	44	1359	14
Future Vol, veh/h	0	0	48	6	5	26	33	1213	42	44	1359	14
Conflicting Peds, #/hr	1	0	0	0	0	1	18	0	5	5	0	18
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	0	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	0	0	3	0	0	6	0	7	0	0	4	0
Mvmt Flow	0	0	51	6	5	27	35	1277	44	46	1431	15

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	-	-	741	2182	2930	667	1464	0	0	1326	0	0
Stage 1	-	-	-	1374	1374	-	-	-	-	-	-	-
Stage 2	-	-	-	808	1556	-	-	-	-	-	-	-
Critical Hdwy	-	-	6.96	7.5	6.5	7.02	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	-	-	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	-	-	3.33	3.5	4	3.36	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	0	0	356	26	15	392	467	-	-	527	-	-
Stage 1	0	0	-	156	215	-	-	-	-	-	-	-
Stage 2	0	0	-	345	176	-	-	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	350	11	6	390	459	-	-	524	-	-
Mov Cap-2 Maneuver	-	-	-	11	6	-	-	-	-	-	-	-
Stage 1	-	-	-	110	151	-	-	-	-	-	-	-
Stage 2	-	-	-	163	95	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	17	\$ 580.9			2.1			2.9		
HCM LOS	C	F								
<hr/>										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR		
Capacity (veh/h)	459	-	-	350	26	524	-	-		
HCM Lane V/C Ratio	0.076	-	-	0.144	1.498	0.088	-	-		
HCM Control Delay (s)	13.5	1.9	-	17	\$ 580.9	12.5	2.6	-		
HCM Lane LOS	B	A	-	C	F	B	A	-		
HCM 95th %tile Q(veh)	0.2	-	-	0.5	4.7	0.3	-	-		

## Notes

~: Volume exceeds capacity    \$: Delay exceeds 300s    +: Computation Not Defined    \*: All major volume in platoon

## Intersection

Int Delay, s/veh 1.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	2	79	2	8	23	0	3	0	2	0	0	1
Future Vol, veh/h	2	79	2	8	23	0	3	0	2	0	0	1
Conflicting Peds, #/hr	1	0	20	20	0	1	2	0	0	0	0	2
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	77	77	77	77	77	77	77	77	77	77	77	77
Heavy Vehicles, %	0	2	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	3	103	3	10	30	0	4	0	3	0	0	1

Major/Minor	Major1	Major2			Minor1			Minor2				
Conflicting Flow All	31	0	0	126	0	0	184	182	125	163	183	33
Stage 1	-	-	-	-	-	-	131	131	-	51	51	-
Stage 2	-	-	-	-	-	-	53	51	-	112	132	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1595	-	-	1473	-	-	781	716	931	806	715	1046
Stage 1	-	-	-	-	-	-	877	792	-	967	856	-
Stage 2	-	-	-	-	-	-	965	856	-	898	791	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1593	-	-	1445	-	-	758	695	913	797	694	1043
Mov Cap-2 Maneuver	-	-	-	-	-	-	758	695	-	797	694	-
Stage 1	-	-	-	-	-	-	859	775	-	964	849	-
Stage 2	-	-	-	-	-	-	955	849	-	894	774	-

Approach	EB	WB			NB			SB					
HCM Control Delay, s	0.2	1.9					9.5					8.5	
HCM LOS							A					A	
<hr/>													
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2	SBLn3	SBLn4	SBLn5	SBLn6
Capacity (veh/h)	813	1593	-	-	1445	-	-	1043	-	-	-	-	-
HCM Lane V/C Ratio	0.008	0.002	-	-	0.007	-	-	0.001	-	-	-	-	-
HCM Control Delay (s)	9.5	7.3	0	-	7.5	0	-	8.5	-	-	-	-	-
HCM Lane LOS	A	A	A	-	A	A	-	A	-	-	-	-	-
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0	-	-	-	-	-

Intersection

Int Delay, s/veh 4.6

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	54	13	14	2	23	3	1	69	7	4	73	7
Future Vol, veh/h	54	13	14	2	23	3	1	69	7	4	73	7
Conflicting Peds, #/hr	0	0	25	25	0	0	25	0	50	50	0	25
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	3	0	0	0	0	0	0	2	0	67	0	0
Mvmt Flow	63	15	16	2	27	3	1	80	8	5	85	8

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	225	264	139	276	264	134	118	0	0	138	0	0
Stage 1	124	124	-	136	136	-	-	-	-	-	-	-
Stage 2	101	140	-	140	128	-	-	-	-	-	-	-
Critical Hdwy	7.13	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.77	-	-
Critical Hdwy Stg 1	6.13	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.13	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.527	4	3.3	3.5	4	3.3	2.2	-	-	2.803	-	-
Pot Cap-1 Maneuver	728	645	915	680	645	920	1483	-	-	1128	-	-
Stage 1	878	797	-	872	788	-	-	-	-	-	-	-
Stage 2	903	785	-	868	794	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	681	596	872	606	596	876	1448	-	-	1074	-	-
Mov Cap-2 Maneuver	681	596	-	606	596	-	-	-	-	-	-	-
Stage 1	856	774	-	829	749	-	-	-	-	-	-	-
Stage 2	866	747	-	811	771	-	-	-	-	-	-	-

Approach	EB	WB			NB		SB	
HCM Control Delay, s	11	11.1			0.1		0.4	
HCM LOS	B	B						
<hr/>								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1448	-	-	691	618	1074	-	-
HCM Lane V/C Ratio	0.001	-	-	0.136	0.053	0.004	-	-
HCM Control Delay (s)	7.5	0	-	11	11.1	8.4	0	-
HCM Lane LOS	A	A	-	B	B	A	A	-
HCM 95th %tile Q(veh)	0	-	-	0.5	0.2	0	-	-

## Intersection

Int Delay, s/veh 1.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	17	179	5	9	194	18	3	6	23	5	10	9
Future Vol, veh/h	17	179	5	9	194	18	3	6	23	5	10	9
Conflicting Peds, #/hr	18	0	28	28	0	18	8	0	7	7	0	8
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	0	0	33	0	3	0	0	0	0	20	12	0
Mvmt Flow	20	208	6	10	226	21	3	7	27	6	12	10

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	265	0	0	242	0	0	555	564	246	550	557	263
Stage 1	-	-	-	-	-	-	279	279	-	275	275	-
Stage 2	-	-	-	-	-	-	276	285	-	275	282	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.3	6.62	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.3	5.62	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.3	5.62	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.68	4.108	3.3
Pot Cap-1 Maneuver	1311	-	-	1336	-	-	445	438	798	419	425	781
Stage 1	-	-	-	-	-	-	732	683	-	694	665	-
Stage 2	-	-	-	-	-	-	735	679	-	694	660	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1289	-	-	1300	-	-	406	408	772	382	396	762
Mov Cap-2 Maneuver	-	-	-	-	-	-	406	408	-	382	396	-
Stage 1	-	-	-	-	-	-	700	652	-	670	648	-
Stage 2	-	-	-	-	-	-	700	661	-	647	630	-

Approach	EB	WB		NB		SB	
HCM Control Delay, s	0.7	0.3		11.2		13	
HCM LOS				B		B	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	617	1289	-	-	1300	-	-	479
HCM Lane V/C Ratio	0.06	0.015	-	-	0.008	-	-	0.058
HCM Control Delay (s)	11.2	7.8	0	-	7.8	0	-	13
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.2	0	-	-	0	-	-	0.2

## Intersection

Int Delay, s/veh 0.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	3	201	3	0	216	0	2	2	0	5	2	3
Future Vol, veh/h	3	201	3	0	216	0	2	2	0	5	2	3
Conflicting Peds, #/hr	22	0	22	22	0	22	1	0	0	0	0	1
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	0	2	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	3	226	3	0	243	0	2	2	0	6	2	3

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	265	0	0	251	0	0	503	521	250	500	522	266
Stage 1	-	-	-	-	-	-	256	256	-	265	265	-
Stage 2	-	-	-	-	-	-	247	265	-	235	257	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1311	-	-	1326	-	-	482	463	794	484	462	778
Stage 1	-	-	-	-	-	-	753	699	-	745	693	-
Stage 2	-	-	-	-	-	-	761	693	-	773	699	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1284	-	-	1298	-	-	467	443	777	471	442	761
Mov Cap-2 Maneuver	-	-	-	-	-	-	467	443	-	471	442	-
Stage 1	-	-	-	-	-	-	735	682	-	727	678	-
Stage 2	-	-	-	-	-	-	754	678	-	768	682	-

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.1	0		13		12		
HCM LOS				B		B		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	455	1284	-	-	1298	-	-	524
HCM Lane V/C Ratio	0.01	0.003	-	-	-	-	-	0.021
HCM Control Delay (s)	13	7.8	0	-	0	-	-	12
HCM Lane LOS	B	A	A	-	A	-	-	B
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0.1

## Lanes, Volumes, Timings

### 1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑↑			↑↑
Traffic Volume (vph)	201	146	1307	40	0	1457
Future Volume (vph)	201	146	1307	40	0	1457
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	2000
Storage Length (ft)	110	0		0	0	
Storage Lanes	1	1		0	0	
Taper Length (ft)	145			25		
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Ped Bike Factor	0.94	0.98	1.00			
Fr <sub>t</sub>		0.850	0.996			
Flt Protected	0.950					
Satd. Flow (prot)	1787	1318	3488	0	0	3623
Flt Permitted	0.950					
Satd. Flow (perm)	1672	1292	3488	0	0	3623
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	30		30			30
Link Distance (ft)	465		176			249
Travel Time (s)	10.6		4.0			5.7
Confl. Peds. (#/hr)	43	5		12	12	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	1%	6%	3%	0%	0%	3%
Bus Blockages (#/hr)	0	0	0	0	0	9
Parking (#/hr)		7				
Shared Lane Traffic (%)						
Lane Group Flow (vph)	207	151	1388	0	0	1502
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2			6
Permitted Phases		8				
Detector Phase	8	8	2			6
Switch Phase						
Minimum Initial (s)	8.0	8.0	15.0			15.0
Minimum Split (s)	31.0	31.0	28.0			29.0
Total Split (s)	38.0	38.0	87.0			87.0
Total Split (%)	30.4%	30.4%	69.6%			69.6%
Yellow Time (s)	4.0	4.0	4.0			4.0
All-Red Time (s)	2.0	2.0	2.0			2.0
Lost Time Adjust (s)	0.0	0.0	0.0			0.0
Total Lost Time (s)	6.0	6.0	6.0			6.0
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Min			C-Min
Act Effct Green (s)	21.2	21.2	91.8			91.8
Actuated g/C Ratio	0.17	0.17	0.73			0.73
v/c Ratio	0.68	0.69	0.54			0.56
Control Delay	59.7	64.3	8.9			9.3
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	59.7	64.3	8.9			9.3
LOS	E	E	A			A

Lanes, Volumes, Timings  
1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Approach Delay	61.6		8.9			9.3
Approach LOS		E		A		A
Queue Length 50th (ft)	160	117	427		251	
Queue Length 95th (ft)	225	177	576		397	
Internal Link Dist (ft)	385		96		169	
Turn Bay Length (ft)	110					
Base Capacity (vph)	457	330	2561		2660	
Starvation Cap Reductn	0	0	0		0	
Spillback Cap Reductn	0	0	0		0	
Storage Cap Reductn	0	0	0		0	
Reduced v/c Ratio	0.45	0.46	0.54		0.56	

Intersection Summary

Area Type: Other

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 104 (83%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.69

Intersection Signal Delay: 14.9

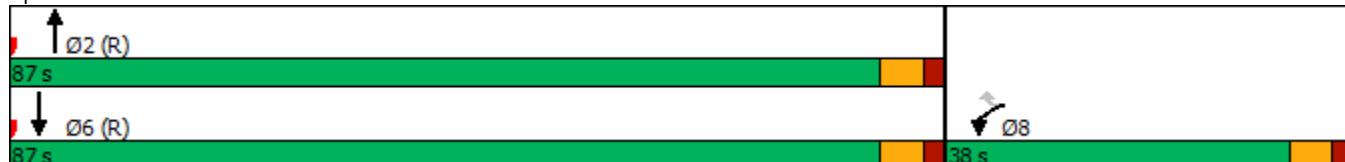
Intersection LOS: B

Intersection Capacity Utilization 60.9%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: Harlem Avenue & South Boulevard



## Lanes, Volumes, Timings

## 2: Harlem Avenue &amp; Randolph Street

10/06/2020

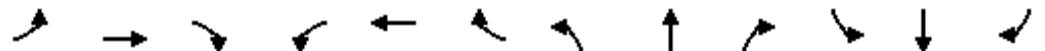


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑		↑	↑	
Traffic Volume (vph)	170	261	120	80	180	38	124	1158	61	84	1254	83
Future Volume (vph)	170	261	120	80	180	38	124	1158	61	84	1254	83
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	2000	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	40		0	50		0	165		0	85		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	90			75			120			90		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	0.98	0.99		1.00	0.99			1.00			0.99	
Fr <sub>t</sub>		0.953			0.974			0.992			0.991	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1799	0	1805	1828	0	1703	3659	0	1805	3578	0
Flt Permitted	0.367			0.161			0.074			0.114		
Satd. Flow (perm)	686	1799	0	305	1828	0	133	3659	0	217	3578	0
Right Turn on Red			No			No			No		No	
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		454			257			242			813	
Travel Time (s)		10.3			5.8			5.5			18.5	
Confl. Peds. (#/hr)	17		5	5		17	29		6	6		29
Confl. Bikes (#/hr)			1			2						
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	3%	6%	3%	0%	0%	3%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	9	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	177	397	0	83	228	0	129	1270	0	88	1392	0
Turn Type	pm+pt	NA										
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	3.0	8.0		3.0	8.0		3.0	15.0		3.0	15.0	
Minimum Split (s)	9.5	30.0		9.5	30.0		9.5	29.0		10.5	33.0	
Total Split (s)	14.0	30.0		14.0	30.0		14.0	67.0		14.0	67.0	
Total Split (%)	11.2%	24.0%		11.2%	24.0%		11.2%	53.6%		11.2%	53.6%	
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0		3.5	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	
Lead/Lag	Lead	Lag										
Lead-Lag Optimize?	Yes	Yes										
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effct Green (s)	38.4	27.5		35.7	24.2		75.2	63.6		72.8	62.3	
Actuated g/C Ratio	0.31	0.22		0.29	0.19		0.60	0.51		0.58	0.50	

## Lanes, Volumes, Timings

### 2: Harlem Avenue & Randolph Street

10/06/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.58	1.01		0.43	0.65		0.66	0.68		0.39	0.78	
Control Delay	41.0	96.1		37.1	56.1		34.8	25.7		18.1	38.5	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	41.0	96.1		37.1	56.1		34.8	25.7		18.1	38.5	
LOS	D	F		D	E		C	C		B	D	
Approach Delay		79.1			51.0			26.5			37.3	
Approach LOS		E			D			C			D	
Queue Length 50th (ft)	107	~365		47	172		40	393		39	555	
Queue Length 95th (ft)	169	#577		87	261		109	490		67	639	
Internal Link Dist (ft)		374			177			162			733	
Turn Bay Length (ft)	40		50			165				85		
Base Capacity (vph)	304	395		216	353		213	1861		263	1784	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.58	1.01		0.38	0.65		0.61	0.68		0.33	0.78	

#### Intersection Summary

Area Type: Other

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 56 (45%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.01

Intersection Signal Delay: 40.8

Intersection LOS: D

Intersection Capacity Utilization 84.8%

ICU Level of Service E

Analysis Period (min) 15

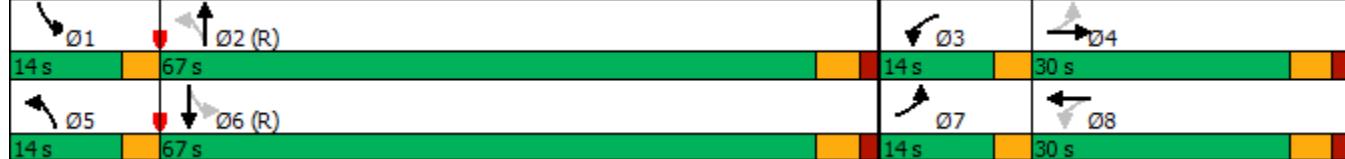
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 2: Harlem Avenue & Randolph Street



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Intersection

Intersection Delay, s/veh 7.6

Intersection LOS A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	5	77	6	31	29	9	12	5	26	3	4	8
Future Vol, veh/h	5	77	6	31	29	9	12	5	26	3	4	8
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles, %	0	0	0	0	0	0	0	0	12	0	0	12
Mvmt Flow	6	100	8	40	38	12	16	6	34	4	5	10
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	7.7			7.7			7.3			7.2		
HCM LOS	A			A			A			A		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	28%	6%	45%	20%
Vol Thru, %	12%	88%	42%	27%
Vol Right, %	60%	7%	13%	53%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	43	88	69	15
LT Vol	12	5	31	3
Through Vol	5	77	29	4
RT Vol	26	6	9	8
Lane Flow Rate	56	114	90	19
Geometry Grp	1	1	1	1
Degree of Util (X)	0.061	0.129	0.103	0.022
Departure Headway (Hd)	3.959	4.07	4.13	4.122
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	888	875	861	874
Service Time	2.058	2.122	2.186	2.122
HCM Lane V/C Ratio	0.063	0.13	0.105	0.022
HCM Control Delay	7.3	7.7	7.7	7.2
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	0.2	0.4	0.3	0.1

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Intersection

Intersection Delay, s/veh 17.9

Intersection LOS C

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	78	299	45	10	228	16	26	68	8	34	147	61
Future Vol, veh/h	78	299	45	10	228	16	26	68	8	34	147	61
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	1	0
Mvmt Flow	87	332	50	11	253	18	29	76	9	38	163	68
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	23.4			14.3			11.7			14.6		
HCM LOS	C			B			B			B		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	25%	18%	4%	14%
Vol Thru, %	67%	71%	90%	61%
Vol Right, %	8%	11%	6%	25%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	102	422	254	242
LT Vol	26	78	10	34
Through Vol	68	299	228	147
RT Vol	8	45	16	61
Lane Flow Rate	113	469	282	269
Geometry Grp	1	1	1	1
Degree of Util (X)	0.213	0.743	0.47	0.465
Departure Headway (Hd)	6.765	5.708	5.992	6.221
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	529	638	601	578
Service Time	4.829	3.708	4.039	4.272
HCM Lane V/C Ratio	0.214	0.735	0.469	0.465
HCM Control Delay	11.7	23.4	14.3	14.6
HCM Lane LOS	B	C	B	B
HCM 95th-tile Q	0.8	6.6	2.5	2.5

**Intersection**

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	29	0	1347	1404	254
Future Vol, veh/h	0	29	0	1347	1404	254
Conflicting Peds, #/hr	5	43	43	0	0	43
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	0	0	3	3	1
Mvmt Flow	0	30	0	1389	1447	262

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	-	941	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	6.9	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	3.3	-
Pot Cap-1 Maneuver	0	268	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	246	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

**Approach**

EB NB SB

HCM Control Delay, s 21.6 0 0

HCM LOS C

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	246	-	-
HCM Lane V/C Ratio	-	0.122	-	-
HCM Control Delay (s)	-	21.6	-	-
HCM Lane LOS	-	C	-	-
HCM 95th %tile Q(veh)	-	0.4	-	-

## Intersection

Int Delay, s/veh 0.6

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑		↔	↔		
Traffic Vol, veh/h	39	1	17	343	4	4
Future Vol, veh/h	39	1	17	343	4	4
Conflicting Peds, #/hr	0	47	47	0	16	5
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	0	0	0	3	0	0
Mvmt Flow	45	1	20	394	5	5

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	93	0	543 98
Stage 1	-	-	-	-	93 -
Stage 2	-	-	-	-	450 -
Critical Hdwy	-	-	4.1	-	6.4 6.2
Critical Hdwy Stg 1	-	-	-	-	5.4 -
Critical Hdwy Stg 2	-	-	-	-	5.4 -
Follow-up Hdwy	-	-	2.2	-	3.5 3.3
Pot Cap-1 Maneuver	-	-	1514	-	504 963
Stage 1	-	-	-	-	936 -
Stage 2	-	-	-	-	647 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1446	-	466 916
Mov Cap-2 Maneuver	-	-	-	-	466 -
Stage 1	-	-	-	-	878 -
Stage 2	-	-	-	-	637 -

Approach	EB	WB	NB
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HCM Control Delay, s 0 0.4 10.9

HCM LOS B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	618	-	-	1446	-
HCM Lane V/C Ratio	0.015	-	-	0.014	-
HCM Control Delay (s)	10.9	-	-	7.5	0
HCM Lane LOS	B	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

## Intersection

Int Delay, s/veh

5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	0	0	50	4	1	44	18	1303	45	43	1367	23
Future Vol, veh/h	0	0	50	4	1	44	18	1303	45	43	1367	23
Conflicting Peds, #/hr	2	0	1	1	0	2	20	0	18	18	0	20
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	0	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96	96	96	96	96	96
Heavy Vehicles, %	0	0	0	0	0	3	6	3	0	0	3	0
Mvmt Flow	0	0	52	4	1	46	19	1357	47	45	1424	24

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	-	-	745	2240	2995	722	1468	0	0	1422	0	0
Stage 1	-	-	-	1437	1437	-	-	-	-	-	-	
Stage 2	-	-	-	803	1558	-	-	-	-	-	-	
Critical Hdwy	-	-	6.9	7.5	6.5	6.96	4.22	-	-	4.1	-	-
Critical Hdwy Stg 1	-	-	-	6.5	5.5	-	-	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	6.5	5.5	-	-	-	-	-	-	
Follow-up Hdwy	-	-	3.3	3.5	4	3.33	2.26	-	-	2.2	-	-
Pot Cap-1 Maneuver	0	0	361	24	14	367	436	-	-	485	-	-
Stage 1	0	0	-	143	201	-	-	-	-	-	-	
Stage 2	0	0	-	348	175	-	-	-	-	-	-	
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	
Mov Cap-1 Maneuver	-	-	354	11	5	360	428	-	-	477	-	-
Mov Cap-2 Maneuver	-	-	-	11	5	-	-	-	-	-	-	
Stage 1	-	-	-	111	157	-	-	-	-	-	-	
Stage 2	-	-	-	152	88	-	-	-	-	-	-	

Approach	EB	WB			NB			SB		
HCM Control Delay, s	16.9	135.3			1.5			3.4		
HCM LOS	C	F								
<hr/>										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR		
Capacity (veh/h)	428	-	-	354	71	477	-	-		
HCM Lane V/C Ratio	0.044	-	-	0.147	0.719	0.094	-	-		
HCM Control Delay (s)	13.8	1.4	-	16.9	135.3	13.3	3.1	-		
HCM Lane LOS	B	A	-	C	F	B	A	-		
HCM 95th %tile Q(veh)	0.1	-	-	0.5	3.3	0.3	-	-		

## Intersection

Int Delay, s/veh 1.9

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	4	101	1	8	60	1	2	3	7	5	6	7
Future Vol, veh/h	4	101	1	8	60	1	2	3	7	5	6	7
Conflicting Peds, #/hr	0	0	16	16	0	0	7	0	0	0	0	7
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	81	81	81	81	81	81	81	81	81	81	81	81
Heavy Vehicles, %	0	3	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	5	125	1	10	74	1	2	4	9	6	7	9

Major/Minor	Major1	Major2			Minor1			Minor2				
Conflicting Flow All	75	0	0	142	0	0	262	247	142	237	247	82
Stage 1	-	-	-	-	-	-	152	152	-	95	95	-
Stage 2	-	-	-	-	-	-	110	95	-	142	152	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1537	-	-	1453	-	-	695	659	911	722	659	983
Stage 1	-	-	-	-	-	-	855	775	-	917	820	-
Stage 2	-	-	-	-	-	-	900	820	-	866	775	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1537	-	-	1431	-	-	663	643	897	707	643	976
Mov Cap-2 Maneuver	-	-	-	-	-	-	663	643	-	707	643	-
Stage 1	-	-	-	-	-	-	840	761	-	914	814	-
Stage 2	-	-	-	-	-	-	872	814	-	851	761	-

Approach	EB	WB			NB			SB			
HCM Control Delay, s	0.3	0.9			9.7			9.9			
HCM LOS					A			A			
<hr/>											
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBT	SBR	SBLn1
Capacity (veh/h)	775	1537	-	-	1431	-	-	764	-	-	-
HCM Lane V/C Ratio	0.019	0.003	-	-	0.007	-	-	0.029	-	-	-
HCM Control Delay (s)	9.7	7.3	0	-	7.5	0	-	9.9	-	-	-
HCM Lane LOS	A	A	A	-	A	A	-	A	-	-	-
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1	-	-	-

## Intersection

Int Delay, s/veh 5.9

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	69	11	33	10	32	25	15	138	9	17	199	22
Future Vol, veh/h	69	11	33	10	32	25	15	138	9	17	199	22
Conflicting Peds, #/hr	0	0	16	16	0	0	44	0	55	55	0	44
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	80	80	80	80	80	80	80	80	80	80	80
Heavy Vehicles, %	0	0	8	0	0	0	0	0	0	0	0	0
Mvmt Flow	86	14	41	13	40	31	19	173	11	21	249	28

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	601	626	323	621	635	234	321	0	0	239	0	0
Stage 1	349	349	-	272	272	-	-	-	-	-	-	-
Stage 2	252	277	-	349	363	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.28	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.372	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	415	403	704	403	399	810	1250	-	-	1340	-	-
Stage 1	671	637	-	738	688	-	-	-	-	-	-	-
Stage 2	757	685	-	671	628	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	338	352	664	332	349	768	1198	-	-	1270	-	-
Mov Cap-2 Maneuver	338	352	-	332	349	-	-	-	-	-	-	-
Stage 1	631	598	-	687	641	-	-	-	-	-	-	-
Stage 2	669	638	-	593	590	-	-	-	-	-	-	-

Approach	EB	WB			NB		SB	
HCM Control Delay, s	19	15.3			0.7		0.6	
HCM LOS	C	C						
<hr/>								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1198	-	-	396	434	1270	-	-
HCM Lane V/C Ratio	0.016	-	-	0.357	0.193	0.017	-	-
HCM Control Delay (s)	8.1	0	-	19	15.3	7.9	0	-
HCM Lane LOS	A	A	-	C	C	A	A	-
HCM 95th %tile Q(veh)	0	-	-	1.6	0.7	0.1	-	-

## Intersection

Int Delay, s/veh 2.5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	13	383	10	25	269	25	11	5	30	14	9	18
Future Vol, veh/h	13	383	10	25	269	25	11	5	30	14	9	18
Conflicting Peds, #/hr	18	0	41	41	0	18	14	0	6	6	0	14
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	79	79	79
Heavy Vehicles, %	5	0	0	12	1	0	0	0	0	0	1	10
Mvmt Flow	16	485	13	32	341	32	14	6	38	18	11	23

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	391	0	0	539	0	0	1017	1020	539	991	1010	389
Stage 1	-	-	-	-	-	-	565	565	-	439	439	-
Stage 2	-	-	-	-	-	-	452	455	-	552	571	-
Critical Hdwy	4.15	-	-	4.22	-	-	7.1	6.5	6.2	7.1	6.51	6.3
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.51	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.51	-
Follow-up Hdwy	2.245	-	-	2.308	-	-	3.5	4	3.3	3.5	4.009	3.39
Pot Cap-1 Maneuver	1151	-	-	981	-	-	218	239	546	227	241	642
Stage 1	-	-	-	-	-	-	513	511	-	601	580	-
Stage 2	-	-	-	-	-	-	591	572	-	522	506	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1131	-	-	943	-	-	182	212	522	192	214	623
Mov Cap-2 Maneuver	-	-	-	-	-	-	182	212	-	192	214	-
Stage 1	-	-	-	-	-	-	483	481	-	579	546	-
Stage 2	-	-	-	-	-	-	526	538	-	465	477	-

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.3	0.7		18.5		20.4		
HCM LOS				C		C		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	325	1131	-	-	943	-	-	285
HCM Lane V/C Ratio	0.179	0.015	-	-	0.034	-	-	0.182
HCM Control Delay (s)	18.5	8.2	0	-	8.9	0	-	20.4
HCM Lane LOS	C	A	A	-	A	A	-	C
HCM 95th %tile Q(veh)	0.6	0	-	-	0.1	-	-	0.7

## Intersection

Int Delay, s/veh 0.6

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	4	414	9	6	303	6	6	2	3	5	0	10
Future Vol, veh/h	4	414	9	6	303	6	6	2	3	5	0	10
Conflicting Peds, #/hr	7	0	8	8	0	7	1	0	4	4	0	1
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	4	460	10	7	337	7	7	2	3	6	0	11

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	351	0	0	478	0	0	842	846	477	842	848	349
Stage 1	-	-	-	-	-	-	481	481	-	362	362	-
Stage 2	-	-	-	-	-	-	361	365	-	480	486	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1219	-	-	1095	-	-	286	301	592	286	301	699
Stage 1	-	-	-	-	-	-	570	557	-	661	629	-
Stage 2	-	-	-	-	-	-	662	627	-	571	554	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1211	-	-	1087	-	-	277	293	585	277	293	694
Mov Cap-2 Maneuver	-	-	-	-	-	-	277	293	-	277	293	-
Stage 1	-	-	-	-	-	-	563	550	-	654	620	-
Stage 2	-	-	-	-	-	-	646	618	-	561	547	-

Approach	EB	WB		NB		SB	
HCM Control Delay, s	0.1	0.2		16.4		13.1	
HCM LOS				C		B	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	327	1211	-	-	1087	-	-	462
HCM Lane V/C Ratio	0.037	0.004	-	-	0.006	-	-	0.036
HCM Control Delay (s)	16.4	8	0	-	8.3	0	-	13.1
HCM Lane LOS	C	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1

# Lanes, Volumes, Timings

## 1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑↑			↑↑
Traffic Volume (vph)	136	145	1160	45	0	1247
Future Volume (vph)	136	145	1160	45	0	1247
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	2000
Storage Length (ft)	110	0		0	0	
Storage Lanes	1	1		0	0	
Taper Length (ft)	145			25		
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Ped Bike Factor	0.90	0.98	1.00			
Fr <sub>t</sub>		0.850	0.994			
Flt Protected	0.950					
Satd. Flow (prot)	1752	1356	3508	0	0	3695
Flt Permitted	0.950					
Satd. Flow (perm)	1583	1329	3508	0	0	3695
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	30		30			30
Link Distance (ft)	465		176			249
Travel Time (s)	10.6		4.0			5.7
Confl. Peds. (#/hr)	67	6		29	29	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	3%	3%	2%	0%	0%	1%
Bus Blockages (#/hr)	0	0	0	0	0	9
Parking (#/hr)		7				
Shared Lane Traffic (%)						
Lane Group Flow (vph)	140	149	1242	0	0	1286
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2			6
Permitted Phases		8				
Detector Phase	8	8	2			6
Switch Phase						
Minimum Initial (s)	8.0	8.0	15.0			15.0
Minimum Split (s)	31.0	31.0	28.0			29.0
Total Split (s)	36.0	36.0	84.0			84.0
Total Split (%)	30.0%	30.0%	70.0%			70.0%
Yellow Time (s)	4.0	4.0	4.0			4.0
All-Red Time (s)	2.0	2.0	2.0			2.0
Lost Time Adjust (s)	0.0	0.0	0.0			0.0
Total Lost Time (s)	6.0	6.0	6.0			6.0
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Min			C-Min
Act Effct Green (s)	19.6	19.6	88.4			88.4
Actuated g/C Ratio	0.16	0.16	0.74			0.74
v/c Ratio	0.49	0.69	0.48			0.47
Control Delay	50.3	62.8	8.6			7.7
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	50.3	62.8	8.6			7.7
LOS	D	E	A			A

Lanes, Volumes, Timings  
1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Approach Delay	56.7		8.6			7.7
Approach LOS		E		A		A
Queue Length 50th (ft)	100	110	338		184	
Queue Length 95th (ft)	153	171	457		288	
Internal Link Dist (ft)	385		96		169	
Turn Bay Length (ft)	110					
Base Capacity (vph)	438	332	2584		2722	
Starvation Cap Reductn	0	0	0		0	
Spillback Cap Reductn	0	0	0		0	
Storage Cap Reductn	0	0	0		0	
Reduced v/c Ratio	0.32	0.45	0.48		0.47	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 101 (84%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.69

Intersection Signal Delay: 13.1

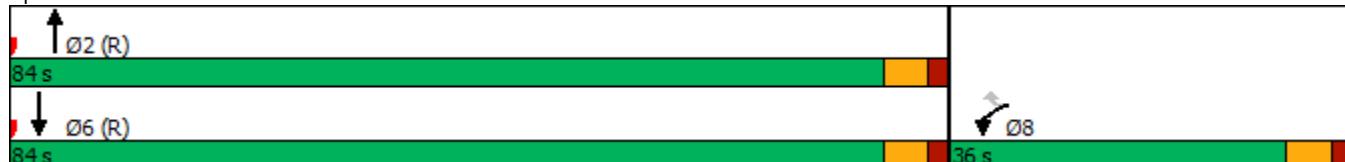
Intersection LOS: B

Intersection Capacity Utilization 55.2%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: Harlem Avenue & South Boulevard



## Lanes, Volumes, Timings

## 2: Harlem Avenue &amp; Randolph Street

10/06/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑↑		↑	↑↑	
Traffic Volume (vph)	152	155	73	64	135	33	84	1077	34	67	1066	87
Future Volume (vph)	152	155	73	64	135	33	84	1077	34	67	1066	87
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	2000	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)	0%			0%			0%			0%		
Storage Length (ft)	40			0	50		0	165		0	85	0
Storage Lanes	1			0	1		0	1		0	1	0
Taper Length (ft)	90				75			120			90	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	0.99	0.99			0.99	0.99			1.00			1.00
Fr <sub>t</sub>	0.952			0.970			0.995			0.989		
Flt Protected	0.950				0.950			0.950			0.950	
Satd. Flow (prot)	1787	1772	0	1805	1832	0	1805	3705	0	1805	3646	0
Flt Permitted	0.441				0.415			0.136			0.161	
Satd. Flow (perm)	819	1772	0	783	1832	0	258	3705	0	306	3646	0
Right Turn on Red	No			No			No			No		
Satd. Flow (RTOR)												
Link Speed (mph)	30			30			30			30		
Link Distance (ft)	454			257			242			813		
Travel Time (s)	10.3			5.8			5.5			18.5		
Confl. Peds. (#/hr)	12	8		8	12		8	5		5	5	
Confl. Bikes (#/hr)	1											
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	0%	4%	0%	0%	0%	0%	2%	0%	0%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	9	0
Parking (#/hr)												
Mid-Block Traffic (%)	0%			0%			0%			0%		
Shared Lane Traffic (%)												
Lane Group Flow (vph)	160	240	0	67	177	0	88	1170	0	71	1214	0
Turn Type	pm+pt	NA										
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	3.0	8.0		3.0	8.0		3.0	15.0		3.0	15.0	
Minimum Split (s)	9.5	30.0		9.5	30.0		9.5	29.0		10.5	33.0	
Total Split (s)	13.0	30.0		13.0	30.0		13.0	64.0		13.0	64.0	
Total Split (%)	10.8%	25.0%		10.8%	25.0%		10.8%	53.3%		10.8%	53.3%	
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0		3.5	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	
Lead/Lag	Lead	Lag										
Lead-Lag Optimize?	Yes	Yes										
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effct Green (s)	33.0	22.9		30.5	19.7		75.3	66.3		73.9	64.1	
Actuated g/C Ratio	0.28	0.19		0.25	0.16		0.63	0.55		0.62	0.53	

Lanes, Volumes, Timings  
2: Harlem Avenue & Randolph Street

10/06/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.53	0.71		0.25	0.59		0.34	0.57		0.25	0.62	
Control Delay	39.2	58.2		31.9	53.8		12.4	20.4		15.0	30.7	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	39.2	58.2		31.9	53.8		12.4	20.4		15.0	30.7	
LOS	D	E		C	D		B	C		B	C	
Approach Delay		50.6			47.8			19.9			29.8	
Approach LOS		D			D			B			C	
Queue Length 50th (ft)	94	177		37	126		24	319		29	432	
Queue Length 95th (ft)	148	265		71	195		48	421		59	541	
Internal Link Dist (ft)		374			177			162			733	
Turn Bay Length (ft)	40		50			165				85		
Base Capacity (vph)	301	356		287	366		286	2047		312	1946	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.53	0.67		0.23	0.48		0.31	0.57		0.23	0.62	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 56 (47%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.71

Intersection Signal Delay: 29.9

Intersection LOS: C

Intersection Capacity Utilization 73.3%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 2: Harlem Avenue & Randolph Street



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Intersection

Intersection Delay, s/veh 7.6

Intersection LOS A

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Vol, veh/h	9	86	7	22	18	16	11	5	26	6	6	8
Future Vol, veh/h	9	86	7	22	18	16	11	5	26	6	6	8
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Heavy Vehicles, %	0	2	0	0	7	17	0	0	0	17	17	0
Mvmt Flow	11	109	9	28	23	20	14	6	33	8	8	10
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	7.8			7.5			7.3			7.6		
HCM LOS	A			A			A			A		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	26%	9%	39%	30%
Vol Thru, %	12%	84%	32%	30%
Vol Right, %	62%	7%	29%	40%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	42	102	56	20
LT Vol	11	9	22	6
Through Vol	5	86	18	6
RT Vol	26	7	16	8
Lane Flow Rate	53	129	71	25
Geometry Grp	1	1	1	1
Degree of Util (X)	0.06	0.146	0.08	0.032
Departure Headway (Hd)	4.045	4.067	4.043	4.499
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	891	874	876	800
Service Time	2.045	2.125	2.114	2.501
HCM Lane V/C Ratio	0.059	0.148	0.081	0.031
HCM Control Delay	7.3	7.8	7.5	7.6
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	0.2	0.5	0.3	0.1

## Intersection

Intersection Delay, s/veh 11.3

Intersection LOS B

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	59	194	23	8	195	5	11	71	13	42	98	43
Future Vol, veh/h	59	194	23	8	195	5	11	71	13	42	98	43
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	3	0	0
Mvmt Flow	66	218	26	9	219	6	12	80	15	47	110	48
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	12.2			11			9.8			11		
HCM LOS	B			B			A			B		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	12%	21%	4%	23%
Vol Thru, %	75%	70%	94%	54%
Vol Right, %	14%	8%	2%	23%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	95	276	208	183
LT Vol	11	59	8	42
Through Vol	71	194	195	98
RT Vol	13	23	5	43
Lane Flow Rate	107	310	234	206
Geometry Grp	1	1	1	1
Degree of Util (X)	0.167	0.442	0.34	0.312
Departure Headway (Hd)	5.636	5.136	5.245	5.466
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	635	701	685	658
Service Time	3.68	3.167	3.279	3.504
HCM Lane V/C Ratio	0.169	0.442	0.342	0.313
HCM Control Delay	9.8	12.2	11	11
HCM Lane LOS	A	B	B	B
HCM 95th-tile Q	0.6	2.3	1.5	1.3

**Intersection**

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	30	0	1205	1226	157
Future Vol, veh/h	0	30	0	1205	1226	157
Conflicting Peds, #/hr	6	67	48	0	0	48
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	0	0	2	1	0
Mvmt Flow	0	31	0	1242	1264	162

Major/Minor	Minor2	Major1	Major2		
Conflicting Flow All	-	828	-	0	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-
Pot Cap-1 Maneuver	0	318	0	-	-
Stage 1	0	-	0	-	-
Stage 2	0	-	0	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	284	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-
Stage 1	-	-	-	-	-
Stage 2	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	19.2	0	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	284	-	-
HCM Lane V/C Ratio	-	0.109	-	-
HCM Control Delay (s)	-	19.2	-	-
HCM Lane LOS	-	C	-	-
HCM 95th %tile Q(veh)	-	0.4	-	-

## Intersection

Int Delay, s/veh 0.4

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑		↔	↔		
Traffic Vol, veh/h	42	4	6	277	4	4
Future Vol, veh/h	42	4	6	277	4	4
Conflicting Peds, #/hr	0	85	85	0	11	2
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	88	88	88	88	88	88
Heavy Vehicles, %	0	0	0	3	0	0
Mvmt Flow	48	5	7	315	5	5

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	138	0	476
Stage 1	-	-	-	-	136
Stage 2	-	-	-	-	340
Critical Hdwy	-	-	4.1	-	6.4
Critical Hdwy Stg 1	-	-	-	-	5.4
Critical Hdwy Stg 2	-	-	-	-	5.4
Follow-up Hdwy	-	-	2.2	-	3.5
Pot Cap-1 Maneuver	-	-	1458	-	551
Stage 1	-	-	-	-	895
Stage 2	-	-	-	-	725
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1340	-	498
Mov Cap-2 Maneuver	-	-	-	-	498
Stage 1	-	-	-	-	817
Stage 2	-	-	-	-	718

Approach	EB	WB	NB	
HCM Control Delay, s	0	0.2	10.8	
HCM LOS			B	

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	625	-	-	1340	-
HCM Lane V/C Ratio	0.015	-	-	0.005	-
HCM Control Delay (s)	10.8	-	-	7.7	0
HCM Lane LOS	B	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

## Intersection

Int Delay, s/veh 3.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	0	0	36	1	5	31	19	1174	69	33	1183	40
Future Vol, veh/h	0	0	36	1	5	31	19	1174	69	33	1183	40
Conflicting Peds, #/hr	0	0	0	0	0	0	3	0	8	8	0	3
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	0	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	5	0	1	2	0	1	0
Mvmt Flow	0	0	39	1	5	34	21	1276	75	36	1286	43

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	-	-	668	2079	2768	684	1332	0	0	1359	0	0
Stage 1	-	-	-	1364	1364	-	-	-	-	-	-	-
Stage 2	-	-	-	715	1404	-	-	-	-	-	-	-
Critical Hdwy	-	-	6.9	7.5	6.5	7	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	-	-	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	-	-	3.3	3.5	4	3.35	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	0	0	405	32	20	384	525	-	-	512	-	-
Stage 1	0	0	-	158	218	-	-	-	-	-	-	-
Stage 2	0	0	-	392	208	-	-	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	404	20	12	381	524	-	-	508	-	-
Mov Cap-2 Maneuver	-	-	-	20	12	-	-	-	-	-	-	-
Stage 1	-	-	-	131	180	-	-	-	-	-	-	-
Stage 2	-	-	-	256	150	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	14.9	116.1			1.1			1.9		
HCM LOS	B	F								
<hr/>										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR		
Capacity (veh/h)	524	-	-	404	68	508	-	-		
HCM Lane V/C Ratio	0.039	-	-	0.097	0.591	0.071	-	-		
HCM Control Delay (s)	12.2	1	-	14.9	116.1	12.6	1.7	-		
HCM Lane LOS	B	A	-	B	F	B	A	-		
HCM 95th %tile Q(veh)	0.1	-	-	0.3	2.5	0.2	-	-		

## Intersection

Int Delay, s/veh

1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	5	112	1	4	49	3	2	0	2	2	3	5
Future Vol, veh/h	5	112	1	4	49	3	2	0	2	2	3	5
Conflicting Peds, #/hr	4	0	11	11	0	4	6	0	5	5	0	6
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	2	0	0	3	0	0	0	0	0	0	20
Mvmt Flow	5	122	1	4	53	3	2	0	2	2	3	5

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	60	0	0	134	0	0	217	212	139	206	211	65
Stage 1	-	-	-	-	-	-	144	144	-	67	67	-
Stage 2	-	-	-	-	-	-	73	68	-	139	144	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.4
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.48
Pot Cap-1 Maneuver	1556	-	-	1463	-	-	744	689	915	756	690	951
Stage 1	-	-	-	-	-	-	864	782	-	948	843	-
Stage 2	-	-	-	-	-	-	942	842	-	869	782	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1550	-	-	1448	-	-	722	675	901	745	676	942
Mov Cap-2 Maneuver	-	-	-	-	-	-	722	675	-	745	676	-
Stage 1	-	-	-	-	-	-	853	772	-	941	837	-
Stage 2	-	-	-	-	-	-	925	836	-	860	772	-

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.3	0.5		9.5		9.5		
HCM LOS				A		A		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	802	1550	-	-	1448	-	-	804
HCM Lane V/C Ratio	0.005	0.004	-	-	0.003	-	-	0.014
HCM Control Delay (s)	9.5	7.3	0	-	7.5	0	-	9.5
HCM Lane LOS	A	A	A	-	A	A	-	A
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0

Intersection												
Int Delay, s/veh	5.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	+	+	+	+	+	+	+	+	+	+	+	+
Traffic Vol, veh/h	75	19	22	17	22	22	8	120	7	16	144	26
Future Vol, veh/h	75	19	22	17	22	22	8	120	7	16	144	26
Conflicting Peds, #/hr	0	0	20	20	0	0	59	0	44	44	0	59
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	0	0	2	0	12	0	0	0	0	0	0	0
Mvmt Flow	84	21	25	19	25	25	9	135	8	18	162	29
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	454	477	256	457	487	183	250	0	0	187	0	0
Stage 1	272	272	-	201	201	-	-	-	-	-	-	-
Stage 2	182	205	-	256	286	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.22	7.1	6.62	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.62	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.62	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.318	3.5	4.108	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	520	490	783	517	466	865	1327	-	-	1399	-	-
Stage 1	738	688	-	805	717	-	-	-	-	-	-	-
Stage 2	824	736	-	753	657	-	-	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	447	433	725	444	412	829	1252	-	-	1340	-	-
Mov Cap-2 Maneuver	447	433	-	444	412	-	-	-	-	-	-	-
Stage 1	691	640	-	765	681	-	-	-	-	-	-	-
Stage 2	764	699	-	679	611	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	15.3		13		0.5		0.7					
HCM LOS	C		B									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR				
Capacity (veh/h)	1252	-	-	479	516	1340	-	-				
HCM Lane V/C Ratio	0.007	-	-	0.272	0.133	0.013	-	-				
HCM Control Delay (s)	7.9	0	-	15.3	13	7.7	0	-				
HCM Lane LOS	A	A	-	C	B	A	A	-				
HCM 95th %tile Q(veh)	0	-	-	1.1	0.5	0	-	-				

## Intersection

Int Delay, s/veh 2.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	7	245	4	21	218	16	5	19	27	9	17	9
Future Vol, veh/h	7	245	4	21	218	16	5	19	27	9	17	9
Conflicting Peds, #/hr	18	0	5	5	0	18	7	0	11	11	0	7
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	87	87	87	87	87	87	87	87	87	87	87	87
Heavy Vehicles, %	0	18	0	0	1	0	0	0	0	0	1	0
Mvmt Flow	8	282	5	24	251	18	6	22	31	10	20	10

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	287	0	0	292	0	0	636	641	301	664	634	285
Stage 1	-	-	-	-	-	-	306	306	-	326	326	-
Stage 2	-	-	-	-	-	-	330	335	-	338	308	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.51	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.51	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.51	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4.009	3.3
Pot Cap-1 Maneuver	1287	-	-	1281	-	-	393	395	743	377	398	759
Stage 1	-	-	-	-	-	-	708	665	-	691	650	-
Stage 2	-	-	-	-	-	-	687	646	-	681	662	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1265	-	-	1275	-	-	360	375	732	328	378	741
Mov Cap-2 Maneuver	-	-	-	-	-	-	360	375	-	328	378	-
Stage 1	-	-	-	-	-	-	699	656	-	674	625	-
Stage 2	-	-	-	-	-	-	638	621	-	619	653	-

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.2	0.6		13.1		14.6		
HCM LOS				B		B		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	503	1265	-	-	1275	-	-	414
HCM Lane V/C Ratio	0.117	0.006	-	-	0.019	-	-	0.097
HCM Control Delay (s)	13.1	7.9	0	-	7.9	0	-	14.6
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.4	0	-	-	0.1	-	-	0.3

## Intersection

Int Delay, s/veh 0.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	3	275	3	1	247	1	1	0	1	0	1	7
Future Vol, veh/h	3	275	3	1	247	1	1	0	1	0	1	7
Conflicting Peds, #/hr	19	0	10	10	0	19	4	0	1	1	0	4
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	4	324	4	1	291	1	1	0	1	0	1	8

Major/Minor	Major1	Major2		Minor1		Minor2		
Conflicting Flow All	311	0	0	338	0	0	646	657
Stage 1	-	-	-	-	-	-	344	344
Stage 2	-	-	-	-	-	-	302	313
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4
Pot Cap-1 Maneuver	1261	-	-	1232	-	-	387	387
Stage 1	-	-	-	-	-	-	676	640
Stage 2	-	-	-	-	-	-	712	661
Platoon blocked, %	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1238	-	-	1220	-	-	375	374
Mov Cap-2 Maneuver	-	-	-	-	-	-	703	377
Stage 1	-	-	-	-	-	-	667	631
Stage 2	-	-	-	-	-	-	699	648

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.1	0		12.4		10.7		
HCM LOS				B		B		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBC	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	489	1238	-	-	1220	-	-	641
HCM Lane V/C Ratio	0.005	0.003	-	-	0.001	-	-	0.015
HCM Control Delay (s)	12.4	7.9	0	-	8	0	-	10.7
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0

Lanes, Volumes, Timings  
1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑↑			↑↑
Traffic Volume (vph)	147	83	1252	19	0	1446
Future Volume (vph)	147	83	1252	19	0	1446
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	2000
Storage Length (ft)	110	0		0	0	
Storage Lanes	1	1		0	0	
Taper Length (ft)	145			25		
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Ped Bike Factor	0.92	0.97	1.00			
Fr <sub>t</sub>		0.850	0.998			
Flt Protected	0.950					
Satd. Flow (prot)	1752	1225	3362	0	0	3623
Flt Permitted	0.950					
Satd. Flow (perm)	1605	1185	3362	0	0	3623
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	30		30			30
Link Distance (ft)	465		176			249
Travel Time (s)	10.6		4.0			5.7
Confl. Peds. (#/hr)	56	13		29	29	
Confl. Bikes (#/hr)		2				
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	14%	7%	7%	0%	3%
Bus Blockages (#/hr)	0	0	0	0	0	9
Parking (#/hr)		7				
Shared Lane Traffic (%)						
Lane Group Flow (vph)	160	90	1382	0	0	1572
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2			6
Permitted Phases		8				
Detector Phase	8	8	2			6
Switch Phase						
Minimum Initial (s)	8.0	8.0	15.0			15.0
Minimum Split (s)	31.0	31.0	28.0			29.0
Total Split (s)	38.0	38.0	87.0			87.0
Total Split (%)	30.4%	30.4%	69.6%			69.6%
Yellow Time (s)	4.0	4.0	4.0			4.0
All-Red Time (s)	2.0	2.0	2.0			2.0
Lost Time Adjust (s)	0.0	0.0	0.0			0.0
Total Lost Time (s)	6.0	6.0	6.0			6.0
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Min			C-Min
Act Effct Green (s)	17.6	17.6	95.4			95.4
Actuated g/C Ratio	0.14	0.14	0.76			0.76
v/c Ratio	0.65	0.54	0.54			0.57
Control Delay	62.4	60.9	13.7			7.7
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	62.4	60.9	13.7			7.7

Lanes, Volumes, Timings  
1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
LOS	E	E	B			A
Approach Delay	61.8		13.7			7.7
Approach LOS	E		B			A
Queue Length 50th (ft)	124	69	471			241
Queue Length 95th (ft)	188	120	611			359
Internal Link Dist (ft)	385		96			169
Turn Bay Length (ft)	110					
Base Capacity (vph)	448	303	2564			2763
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.36	0.30	0.54			0.57

Intersection Summary

Area Type: Other

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 103 (82%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.65

Intersection Signal Delay: 14.5

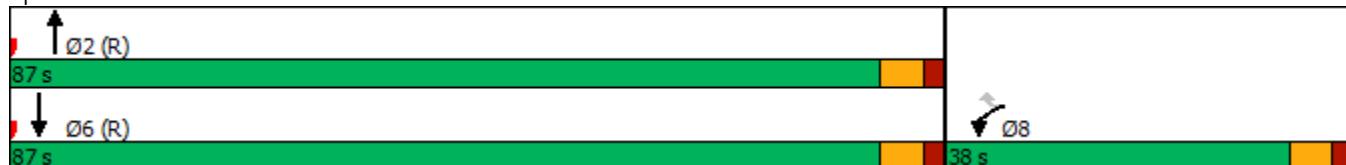
Intersection LOS: B

Intersection Capacity Utilization 60.6%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: Harlem Avenue & South Boulevard



## Lanes, Volumes, Timings

## 2: Harlem Avenue &amp; Randolph Street

10/07/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑↑		↑	↑↑	
Traffic Volume (vph)	130	149	119	75	111	24	83	1168	24	32	1369	82
Future Volume (vph)	130	149	119	75	111	24	83	1168	24	32	1369	82
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	2000	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)	0%			0%			0%			0%		
Storage Length (ft)	40			0	50		0	165		0	85	0
Storage Lanes	1			0	1		0	1		0	1	0
Taper Length (ft)	90				75			120			90	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	0.98	0.99		0.99	0.99			1.00			1.00	
Fr <sub>t</sub>	0.933			0.973			0.997			0.992		
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1787	1722	0	1805	1806	0	1736	3539	0	1736	3581	0
Flt Permitted	0.523			0.312			0.063			0.153		
Satd. Flow (perm)	968	1722	0	587	1806	0	115	3539	0	280	3581	0
Right Turn on Red	No			No			No			No		
Satd. Flow (RTOR)												
Link Speed (mph)	30			30			30			30		
Link Distance (ft)	454			257			242			813		
Travel Time (s)	10.3			5.8			5.5			18.5		
Confl. Peds. (#/hr)	14	12		12	14		7	7		7	7	
Confl. Bikes (#/hr)	7											
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	2%	1%	0%	2%	0%	4%	7%	5%	4%	3%	6%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	9	0
Parking (#/hr)												
Mid-Block Traffic (%)	0%			0%			0%			0%		
Shared Lane Traffic (%)												
Lane Group Flow (vph)	135	279	0	78	141	0	86	1242	0	33	1511	0
Turn Type	pm+pt	NA										
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	3.0	8.0		3.0	8.0		3.0	15.0		3.0	15.0	
Minimum Split (s)	9.5	30.0		9.5	32.0		9.5	29.0		10.5	33.0	
Total Split (s)	14.0	30.0		14.0	30.0		17.0	67.0		14.0	64.0	
Total Split (%)	11.2%	24.0%		11.2%	24.0%		13.6%	53.6%		11.2%	51.2%	
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0		3.5	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	
Lead/Lag	Lead	Lag										
Lead-Lag Optimize?	Yes	Yes										
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effct Green (s)	34.9	24.0		32.3	20.8		80.3	72.3		75.7	66.8	
Actuated g/C Ratio	0.28	0.19		0.26	0.17		0.64	0.58		0.61	0.53	

Lanes, Volumes, Timings  
2: Harlem Avenue & Randolph Street

10/07/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.40	0.85		0.33	0.47		0.48	0.61		0.14	0.79	
Control Delay	36.2	71.7		34.7	51.4		21.9	20.5		11.9	34.7	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	36.2	71.7		34.7	51.4		21.9	20.5		11.9	34.7	
LOS	D	E		C	D		C	C		B	C	
Approach Delay		60.1			45.4			20.6			34.2	
Approach LOS		E			D			C			C	
Queue Length 50th (ft)	80	217		44	101		26	376		14	617	
Queue Length 95th (ft)	133	#366		83	166		66	464		m21	717	
Internal Link Dist (ft)		374			177			162			733	
Turn Bay Length (ft)	40		50			165				85		
Base Capacity (vph)	339	334		261	346		249	2046		300	1914	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.40	0.84		0.30	0.41		0.35	0.61		0.11	0.79	

Intersection Summary

Area Type: Other

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 51 (41%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 32.8

Intersection LOS: C

Intersection Capacity Utilization 81.0%

ICU Level of Service D

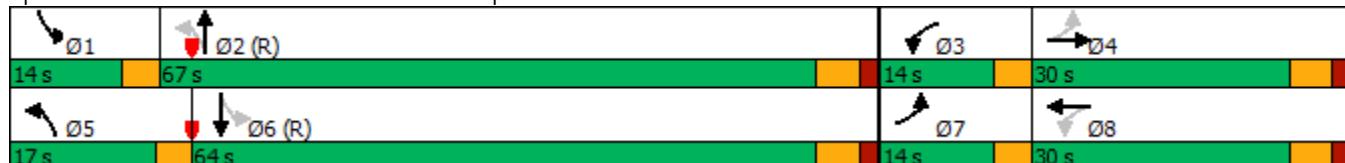
Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Harlem Avenue & Randolph Street



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Intersection

Intersection Delay, s/veh 7.6

Intersection LOS A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	9	65	14	8	19	0	16	7	18	2	2	2
Future Vol, veh/h	9	65	14	8	19	0	16	7	18	2	2	2
Peak Hour Factor	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67
Heavy Vehicles, %	0	0	0	0	0	0	0	25	12	0	0	14
Mvmt Flow	13	97	21	12	28	0	24	10	27	3	3	3
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	7.7			7.4			7.4			7.2		
HCM LOS	A			A			A			A		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	39%	10%	30%	33%
Vol Thru, %	17%	74%	70%	33%
Vol Right, %	44%	16%	0%	33%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	41	88	27	6
LT Vol	16	9	8	2
Through Vol	7	65	19	2
RT Vol	18	14	0	2
Lane Flow Rate	61	131	40	9
Geometry Grp	1	1	1	1
Degree of Util (X)	0.068	0.145	0.047	0.01
Departure Headway (Hd)	4.018	3.977	4.181	4.112
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	880	898	850	857
Service Time	2.095	2.018	2.238	2.2
HCM Lane V/C Ratio	0.069	0.146	0.047	0.011
HCM Control Delay	7.4	7.7	7.4	7.2
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	0.2	0.5	0.1	0

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Intersection

Intersection Delay, s/veh 9.2

Intersection LOS A

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	26	160	24	6	167	12	17	43	4	5	63	36
Future Vol, veh/h	26	160	24	6	167	12	17	43	4	5	63	36
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	29	178	27	7	186	13	19	48	4	6	70	40
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	9.5			9.3			8.7			8.7		
HCM LOS	A			A			A			A		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	27%	12%	3%	5%
Vol Thru, %	67%	76%	90%	61%
Vol Right, %	6%	11%	6%	35%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	64	210	185	104
LT Vol	17	26	6	5
Through Vol	43	160	167	63
RT Vol	4	24	12	36
Lane Flow Rate	71	233	206	116
Geometry Grp	1	1	1	1
Degree of Util (X)	0.1	0.295	0.262	0.154
Departure Headway (Hd)	5.074	4.552	4.594	4.8
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	703	786	779	743
Service Time	3.131	2.594	2.637	2.853
HCM Lane V/C Ratio	0.101	0.296	0.264	0.156
HCM Control Delay	8.7	9.5	9.3	8.7
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	0.3	1.2	1	0.5

**Intersection**

Int Delay, s/veh 0.2

Movement	EBL	EBC	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	18	0	1271	1469	124
Future Vol, veh/h	0	18	0	1271	1469	124
Conflicting Peds, #/hr	13	56	91	0	0	91
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	14	0	7	3	2
Mvmt Flow	0	20	0	1382	1597	135

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	-	1013	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	7.18	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	3.44	-
Pot Cap-1 Maneuver	0	217	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	188	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

**Approach**

EB NB SB

HCM Control Delay, s 26.4 0 0

HCM LOS D

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	188	-	-
HCM Lane V/C Ratio	-	0.104	-	-
HCM Control Delay (s)	-	26.4	-	-
HCM Lane LOS	-	D	-	-
HCM 95th %tile Q(veh)	-	0.3	-	-

**Intersection**

Int Delay, s/veh 0.1

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	25	0	1	222	0	2
Future Vol, veh/h	25	0	1	222	0	2
Conflicting Peds, #/hr	0	24	24	0	1	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	96	96	96	96	96	96
Heavy Vehicles, %	8	0	0	9	0	0
Mvmt Flow	26	0	1	231	0	2

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	50	0	284 50
Stage 1	-	-	-	-	50 -
Stage 2	-	-	-	-	234 -
Critical Hdwy	-	-	4.1	-	6.4 6.2
Critical Hdwy Stg 1	-	-	-	-	5.4 -
Critical Hdwy Stg 2	-	-	-	-	5.4 -
Follow-up Hdwy	-	-	2.2	-	3.5 3.3
Pot Cap-1 Maneuver	-	-	1570	-	710 1024
Stage 1	-	-	-	-	978 -
Stage 2	-	-	-	-	810 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1534	-	692 1001
Mov Cap-2 Maneuver	-	-	-	-	692 -
Stage 1	-	-	-	-	955 -
Stage 2	-	-	-	-	809 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0	8.6
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	1001	-	-	1534	-
HCM Lane V/C Ratio	0.002	-	-	0.001	-
HCM Control Delay (s)	8.6	-	-	7.3	0
HCM Lane LOS	A	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

## Intersection

Int Delay, s/veh 19.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	0	0	49	6	5	26	34	1245	43	45	1428	14
Future Vol, veh/h	0	0	49	6	5	26	34	1245	43	45	1428	14
Conflicting Peds, #/hr	1	0	0	0	0	1	22	0	6	6	0	22
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	0	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	0	0	3	0	0	6	0	7	0	0	4	0
Mvmt Flow	0	0	52	6	5	27	36	1311	45	47	1503	15

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	-	-	781	2258	3046	685	1540	0	0	1362	0	0
Stage 1	-	-	-	1412	1412	-	-	-	-	-	-	-
Stage 2	-	-	-	846	1634	-	-	-	-	-	-	-
Critical Hdwy	-	-	6.96	7.5	6.5	7.02	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	-	-	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	-	-	3.33	3.5	4	3.36	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	0	0	335	23	13	381	437	-	-	511	-	-
Stage 1	0	0	-	148	206	-	-	-	-	-	-	-
Stage 2	0	0	-	328	161	-	-	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	328	8	~3	378	428	-	-	508	-	-
Mov Cap-2 Maneuver	-	-	-	8	~3	-	-	-	-	-	-	-
Stage 1	-	-	-	96	133	-	-	-	-	-	-	-
Stage 2	-	-	-	111	64	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	18	\$ 1244.1			2.5			3.8		
HCM LOS	C	F								
<hr/>										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR		
Capacity (veh/h)	428	-	-	328	15	508	-	-		
HCM Lane V/C Ratio	0.084	-	-	0.157	2.596	0.093	-	-		
HCM Control Delay (s)	14.2	2.3	-	\$ 1244.1	12.8	3.6	-	-		
HCM Lane LOS	B	A	-	C	F	B	A	-		
HCM 95th %tile Q(veh)	0.3	-	-	0.6	5.6	0.3	-	-		

## Notes

~: Volume exceeds capacity    \$: Delay exceeds 300s    +: Computation Not Defined    \*: All major volume in platoon

## Intersection

Int Delay, s/veh 1.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	2	81	2	8	23	0	3	0	2	0	0	1
Future Vol, veh/h	2	81	2	8	23	0	3	0	2	0	0	1
Conflicting Peds, #/hr	1	0	24	24	0	1	2	0	0	0	0	2
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	77	77	77	77	77	77	77	77	77	77	77	77
Heavy Vehicles, %	0	2	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	3	105	3	10	30	0	4	0	3	0	0	1

Major/Minor	Major1	Major2		Minor1		Minor2		
Conflicting Flow All	31	0	0	132	0	0	190	188
Stage 1	-	-	-	-	-	-	137	137
Stage 2	-	-	-	-	-	-	53	51
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4
Pot Cap-1 Maneuver	1595	-	-	1466	-	-	774	710
Stage 1	-	-	-	-	-	-	871	787
Stage 2	-	-	-	-	-	-	965	856
Platoon blocked, %	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1593	-	-	1432	-	-	748	687
Mov Cap-2 Maneuver	-	-	-	-	-	-	748	687
Stage 1	-	-	-	-	-	-	849	767
Stage 2	-	-	-	-	-	-	955	849

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.2	1.9		9.5		8.5		
HCM LOS				A		A		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	803	1593	-	-	1432	-	-	1043
HCM Lane V/C Ratio	0.008	0.002	-	-	0.007	-	-	0.001
HCM Control Delay (s)	9.5	7.3	0	-	7.5	0	-	8.5
HCM Lane LOS	A	A	A	-	A	A	-	A
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0

Intersection

Int Delay, s/veh 4.5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	55	14	14	2	23	3	1	73	7	4	88	7
Future Vol, veh/h	55	14	14	2	23	3	1	73	7	4	88	7
Conflicting Peds, #/hr	0	0	30	30	0	0	30	0	60	60	0	30
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	3	0	0	0	0	0	0	2	0	67	0	0
Mvmt Flow	64	16	16	2	27	3	1	85	8	5	102	8

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	252	301	166	313	301	149	140	0	0	153	0	0
Stage 1	146	146	-	151	151	-	-	-	-	-	-	-
Stage 2	106	155	-	162	150	-	-	-	-	-	-	-
Critical Hdwy	7.13	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.77	-	-
Critical Hdwy Stg 1	6.13	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.13	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.527	4	3.3	3.5	4	3.3	2.2	-	-	2.803	-	-
Pot Cap-1 Maneuver	699	615	884	643	615	903	1456	-	-	1112	-	-
Stage 1	854	780	-	856	776	-	-	-	-	-	-	-
Stage 2	897	773	-	845	777	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	649	560	834	562	560	851	1414	-	-	1048	-	-
Mov Cap-2 Maneuver	649	560	-	562	560	-	-	-	-	-	-	-
Stage 1	828	753	-	806	731	-	-	-	-	-	-	-
Stage 2	860	728	-	783	751	-	-	-	-	-	-	-

Approach	EB	WB			NB		SB	
HCM Control Delay, s	11.4	11.6			0.1		0.3	
HCM LOS	B	B						
<hr/>								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1414	-	-	656	581	1048	-	-
HCM Lane V/C Ratio	0.001	-	-	0.147	0.056	0.004	-	-
HCM Control Delay (s)	7.5	0	-	11.4	11.6	8.4	0	-
HCM Lane LOS	A	A	-	B	B	A	A	-
HCM 95th %tile Q(veh)	0	-	-	0.5	0.2	0	-	-

## Intersection

Int Delay, s/veh 1.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	17	183	5	9	198	18	3	6	23	5	10	9
Future Vol, veh/h	17	183	5	9	198	18	3	6	23	5	10	9
Conflicting Peds, #/hr	22	0	34	34	0	22	9	0	8	8	0	9
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	0	0	33	0	3	0	0	0	0	20	12	0
Mvmt Flow	20	213	6	10	230	21	3	7	27	6	12	10

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	273	0	0	253	0	0	571	583	258	564	576	272
Stage 1	-	-	-	-	-	-	290	290	-	283	283	-
Stage 2	-	-	-	-	-	-	281	293	-	281	293	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.3	6.62	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.3	5.62	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.3	5.62	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.68	4.108	3.3
Pot Cap-1 Maneuver	1302	-	-	1324	-	-	435	427	786	410	415	772
Stage 1	-	-	-	-	-	-	722	676	-	687	659	-
Stage 2	-	-	-	-	-	-	730	674	-	688	653	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1275	-	-	1281	-	-	394	394	755	371	383	749
Mov Cap-2 Maneuver	-	-	-	-	-	-	394	394	-	371	383	-
Stage 1	-	-	-	-	-	-	686	643	-	660	639	-
Stage 2	-	-	-	-	-	-	694	654	-	640	621	-

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.7	0.3		11.4		13.2		
HCM LOS				B		B		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	600	1275	-	-	1281	-	-	465
HCM Lane V/C Ratio	0.062	0.016	-	-	0.008	-	-	0.06
HCM Control Delay (s)	11.4	7.9	0	-	7.8	0	-	13.2
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.2	0	-	-	0	-	-	0.2

## Intersection

Int Delay, s/veh 0.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	3	205	3	0	220	0	2	2	0	5	2	3
Future Vol, veh/h	3	205	3	0	220	0	2	2	0	5	2	3
Conflicting Peds, #/hr	26	0	26	26	0	26	1	0	0	0	0	1
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	0	2	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	3	230	3	0	247	0	2	2	0	6	2	3

Major/Minor	Major1	Major2		Minor1		Minor2	
Conflicting Flow All	273	0	0	259	0	0	515 537 258 512 538 274
Stage 1	-	-	-	-	-	264 264	- 273 273 -
Stage 2	-	-	-	-	-	251 273	- 239 265 -
Critical Hdwy	4.1	-	-	4.1	-	-	7.1 6.5 6.2 7.1 6.5 6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1 5.5 - 6.1 5.5 -
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1 5.5 - 6.1 5.5 -
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5 4 3.3 3.5 4 3.3
Pot Cap-1 Maneuver	1302	-	-	1317	-	-	474 453 786 476 453 770
Stage 1	-	-	-	-	-	-	746 694 - 737 688 -
Stage 2	-	-	-	-	-	-	758 688 - 769 693 -
Platoon blocked, %	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1270	-	-	1284	-	-	457 429 767 461 429 750
Mov Cap-2 Maneuver	-	-	-	-	-	-	457 429 - 461 429 -
Stage 1	-	-	-	-	-	-	725 675 - 716 671 -
Stage 2	-	-	-	-	-	-	751 671 - 764 674 -

Approach	EB	WB		NB		SB	
HCM Control Delay, s	0.1	0		13.2		12.2	
HCM LOS		B		B		B	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	443	1270	-	-	1284	-	-	513
HCM Lane V/C Ratio	0.01	0.003	-	-	-	-	-	0.022
HCM Control Delay (s)	13.2	7.8	0	-	0	-	-	12.2
HCM Lane LOS	B	A	A	-	A	-	-	B
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0.1

## Lanes, Volumes, Timings

### 1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑↑			↑↑
Traffic Volume (vph)	210	154	1358	48	0	1486
Future Volume (vph)	210	154	1358	48	0	1486
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	2000
Storage Length (ft)	110	0		0	0	
Storage Lanes	1	1		0	0	
Taper Length (ft)	145			25		
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Ped Bike Factor	0.92	0.98	1.00			
Fr <sub>t</sub>		0.850	0.995			
Flt Protected	0.950					
Satd. Flow (prot)	1787	1318	3484	0	0	3623
Flt Permitted	0.950					
Satd. Flow (perm)	1648	1290	3484	0	0	3623
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	30		30		30	
Link Distance (ft)	465		176		249	
Travel Time (s)	10.6		4.0		5.7	
Confl. Peds. (#/hr)	52	6		14	14	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	1%	6%	3%	0%	0%	3%
Bus Blockages (#/hr)	0	0	0	0	0	9
Parking (#/hr)		7				
Shared Lane Traffic (%)						
Lane Group Flow (vph)	216	159	1449	0	0	1532
Turn Type	Prot	Perm	NA		NA	
Protected Phases	8		2		6	
Permitted Phases		8				
Detector Phase	8	8	2		6	
Switch Phase						
Minimum Initial (s)	8.0	8.0	15.0		15.0	
Minimum Split (s)	31.0	31.0	28.0		29.0	
Total Split (s)	38.0	38.0	87.0		87.0	
Total Split (%)	30.4%	30.4%	69.6%		69.6%	
Yellow Time (s)	4.0	4.0	4.0		4.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0		6.0	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Min		C-Min	
Act Effct Green (s)	22.0	22.0	91.0		91.0	
Actuated g/C Ratio	0.18	0.18	0.73		0.73	
v/c Ratio	0.69	0.70	0.57		0.58	
Control Delay	59.1	64.2	9.8		9.8	
Queue Delay	0.0	0.0	0.0		0.0	
Total Delay	59.1	64.2	9.8		9.8	
LOS	E	E	A		A	

Lanes, Volumes, Timings  
1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Approach Delay	61.3		9.8			9.8
Approach LOS		E		A		A
Queue Length 50th (ft)	166	123	465		267	
Queue Length 95th (ft)	232	184	628		421	
Internal Link Dist (ft)	385		96		169	
Turn Bay Length (ft)	110					
Base Capacity (vph)	457	330	2537		2638	
Starvation Cap Reductn	0	0	0		0	
Spillback Cap Reductn	0	0	0		0	
Storage Cap Reductn	0	0	0		0	
Reduced v/c Ratio	0.47	0.48	0.57		0.58	

Intersection Summary

Area Type: Other

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 104 (83%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.70

Intersection Signal Delay: 15.6

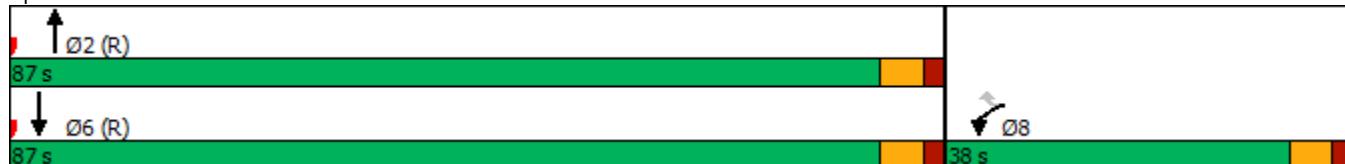
Intersection LOS: B

Intersection Capacity Utilization 62.4%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: Harlem Avenue & South Boulevard



## Lanes, Volumes, Timings

## 2: Harlem Avenue &amp; Randolph Street

10/07/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑↑		↑	↑↑	
Traffic Volume (vph)	173	266	122	81	183	39	126	1213	62	86	1285	84
Future Volume (vph)	173	266	122	81	183	39	126	1213	62	86	1285	84
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	2000	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	40		0	50		0	165		0	85		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	90			75			120			90		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	0.98	0.99		1.00	0.99			1.00			0.99	
Fr <sub>t</sub>		0.953			0.973			0.993			0.991	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1798	0	1805	1824	0	1703	3662	0	1805	3575	0
Flt Permitted	0.367			0.166			0.068			0.101		
Satd. Flow (perm)	684	1798	0	315	1824	0	122	3662	0	192	3575	0
Right Turn on Red			No			No			No		No	
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		454			257			242			813	
Travel Time (s)		10.3			5.8			5.5			18.5	
Confl. Peds. (#/hr)	20		6	6		20	35		7	7		35
Confl. Bikes (#/hr)			1			2						
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	3%	6%	3%	0%	0%	3%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	9	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	180	404	0	84	232	0	131	1329	0	90	1427	0
Turn Type	pm+pt	NA										
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	3.0	8.0		3.0	8.0		3.0	15.0		3.0	15.0	
Minimum Split (s)	9.5	30.0		9.5	30.0		9.5	29.0		10.5	33.0	
Total Split (s)	14.0	30.0		14.0	30.0		14.0	67.0		14.0	67.0	
Total Split (%)	11.2%	24.0%		11.2%	24.0%		11.2%	53.6%		11.2%	53.6%	
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0		3.5	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	
Lead/Lag	Lead	Lag										
Lead-Lag Optimize?	Yes	Yes										
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effct Green (s)	38.3	25.4		35.7	24.1		75.4	63.5		72.6	62.1	
Actuated g/C Ratio	0.31	0.20		0.29	0.19		0.60	0.51		0.58	0.50	

## Lanes, Volumes, Timings

### 2: Harlem Avenue & Randolph Street

10/07/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.60	1.10		0.43	0.66		0.68	0.71		0.42	0.80	
Control Delay	41.5	124.5		37.0	56.7		38.8	26.7		18.0	38.8	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	41.5	124.5		37.0	56.7		38.8	26.7		18.0	38.8	
LOS	D	F		D	E		D	C		B	D	
Approach Delay		98.9			51.5			27.8			37.6	
Approach LOS		F			D			C			D	
Queue Length 50th (ft)	109	~378		48	175		46	422		40	568	
Queue Length 95th (ft)	172	#589		89	265		#121	525		m58	658	
Internal Link Dist (ft)		374			177			162			733	
Turn Bay Length (ft)	40			50			165			85		
Base Capacity (vph)	303	366		218	352		207	1860		251	1775	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.59	1.10		0.39	0.66		0.63	0.71		0.36	0.80	

#### Intersection Summary

Area Type: Other

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 56 (45%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.10

Intersection Signal Delay: 44.3

Intersection LOS: D

Intersection Capacity Utilization 86.2%

ICU Level of Service E

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

#### Splits and Phases: 2: Harlem Avenue & Randolph Street



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Intersection

Intersection Delay, s/veh 7.6

Intersection LOS A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	5	79	6	31	30	9	12	5	26	3	4	8
Future Vol, veh/h	5	79	6	31	30	9	12	5	26	3	4	8
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles, %	0	0	0	0	0	0	0	0	12	0	0	12
Mvmt Flow	6	103	8	40	39	12	16	6	34	4	5	10
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	7.8			7.7			7.3			7.2		
HCM LOS	A			A			A			A		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	28%	6%	44%	20%
Vol Thru, %	12%	88%	43%	27%
Vol Right, %	60%	7%	13%	53%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	43	90	70	15
LT Vol	12	5	31	3
Through Vol	5	79	30	4
RT Vol	26	6	9	8
Lane Flow Rate	56	117	91	19
Geometry Grp	1	1	1	1
Degree of Util (X)	0.063	0.132	0.104	0.022
Departure Headway (Hd)	4.066	4.072	4.132	4.13
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	886	874	860	872
Service Time	2.066	2.127	2.192	2.132
HCM Lane V/C Ratio	0.063	0.134	0.106	0.022
HCM Control Delay	7.3	7.8	7.7	7.2
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	0.2	0.5	0.3	0.1

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Intersection

Intersection Delay, s/veh 19.3

Intersection LOS C

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	79	305	46	10	232	16	26	81	8	35	154	62
Future Vol, veh/h	79	305	46	10	232	16	26	81	8	35	154	62
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	1	0
Mvmt Flow	88	339	51	11	258	18	29	90	9	39	171	69
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	25.9			15.1			12.2			15.5		
HCM LOS	D			C			B			C		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	23%	18%	4%	14%
Vol Thru, %	70%	71%	90%	61%
Vol Right, %	7%	11%	6%	25%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	115	430	258	251
LT Vol	26	79	10	35
Through Vol	81	305	232	154
RT Vol	8	46	16	62
Lane Flow Rate	128	478	287	279
Geometry Grp	1	1	1	1
Degree of Util (X)	0.245	0.772	0.491	0.493
Departure Headway (Hd)	6.908	5.814	6.16	6.363
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	517	621	585	566
Service Time	4.979	3.859	4.214	4.419
HCM Lane V/C Ratio	0.248	0.77	0.491	0.493
HCM Control Delay	12.2	25.9	15.1	15.5
HCM Lane LOS	B	D	C	C
HCM 95th-tile Q	1	7.2	2.7	2.7

**Intersection**

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	30	0	1406	1437	259
Future Vol, veh/h	0	30	0	1406	1437	259
Conflicting Peds, #/hr	6	52	52	0	0	52
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	0	0	3	3	1
Mvmt Flow	0	31	0	1449	1481	267

Major/Minor	Minor2	Major1	Major2	
Conflicting Flow All	-	978	-	0
Stage 1	-	-	-	-
Stage 2	-	-	-	-
Critical Hdwy	-	6.9	-	-
Critical Hdwy Stg 1	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-
Follow-up Hdwy	-	3.3	-	-
Pot Cap-1 Maneuver	0	253	0	-
Stage 1	0	-	0	-
Stage 2	0	-	0	-
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	-	229	-	-
Mov Cap-2 Maneuver	-	-	-	-
Stage 1	-	-	-	-
Stage 2	-	-	-	-

**Approach**

EB NB SB

HCM Control Delay, s 23.2 0 0

HCM LOS C

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	229	-	-
HCM Lane V/C Ratio	-	0.135	-	-
HCM Control Delay (s)	-	23.2	-	-
HCM Lane LOS	-	C	-	-
HCM 95th %tile Q(veh)	-	0.5	-	-

**Intersection**

Int Delay, s/veh 0.5

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑		↔	↔		
Traffic Vol, veh/h	48	1	17	358	4	4
Future Vol, veh/h	48	1	17	358	4	4
Conflicting Peds, #/hr	0	56	56	0	19	6
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	0	0	0	3	0	0
Mvmt Flow	55	1	20	411	5	5

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	112	0	582
Stage 1	-	-	-	-	112
Stage 2	-	-	-	-	470
Critical Hdwy	-	-	4.1	-	6.4
Critical Hdwy Stg 1	-	-	-	-	5.4
Critical Hdwy Stg 2	-	-	-	-	5.4
Follow-up Hdwy	-	-	2.2	-	3.5
Pot Cap-1 Maneuver	-	-	1490	-	479
Stage 1	-	-	-	-	918
Stage 2	-	-	-	-	633
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1411	-	437
Mov Cap-2 Maneuver	-	-	-	-	884
Stage 1	-	-	-	-	437
Stage 2	-	-	-	-	854

Approach	EB	WB	NB
HCM Control Delay, s	0	0.3	11.3
HCM LOS		B	

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	585	-	-	1411	-
HCM Lane V/C Ratio	0.016	-	-	0.014	-
HCM Control Delay (s)	11.3	-	-	7.6	0
HCM Lane LOS	B	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

## Intersection

Int Delay, s/veh 8.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	0	0	51	4	1	45	18	1361	46	44	1400	23
Future Vol, veh/h	0	0	51	4	1	45	18	1361	46	44	1400	23
Conflicting Peds, #/hr	2	0	1	1	0	2	24	0	22	22	0	24
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	0	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96	96	96	96	96	96
Heavy Vehicles, %	0	0	0	0	0	3	6	3	0	0	3	0
Mvmt Flow	0	0	53	4	1	47	19	1418	48	46	1458	24

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	-	-	766	2324	3100	757	1506	0	0	1488	0	0
Stage 1	-	-	-	1502	1502	-	-	-	-	-	-	
Stage 2	-	-	-	822	1598	-	-	-	-	-	-	
Critical Hdwy	-	-	6.9	7.5	6.5	6.96	4.22	-	-	4.1	-	-
Critical Hdwy Stg 1	-	-	-	6.5	5.5	-	-	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	6.5	5.5	-	-	-	-	-	-	
Follow-up Hdwy	-	-	3.3	3.5	4	3.33	2.26	-	-	2.2	-	-
Pot Cap-1 Maneuver	0	0	350	20	12	348	421	-	-	458	-	-
Stage 1	0	0	-	130	187	-	-	-	-	-	-	
Stage 2	0	0	-	339	167	-	-	-	-	-	-	
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	
Mov Cap-1 Maneuver	-	-	342	7	3	340	411	-	-	448	-	-
Mov Cap-2 Maneuver	-	-	-	7	3	-	-	-	-	-	-	
Stage 1	-	-	-	95	136	-	-	-	-	-	-	
Stage 2	-	-	-	117	67	-	-	-	-	-	-	

Approach	EB	WB			NB			SB		
HCM Control Delay, s	17.5	291.5			1.9			4.1		
HCM LOS	C	F								
<hr/>										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR		
Capacity (veh/h)	411	-	-	342	48	448	-	-		
HCM Lane V/C Ratio	0.046	-	-	0.155	1.085	0.102	-	-		
HCM Control Delay (s)	14.2	1.8	-	17.5	291.5	13.9	3.9	-		
HCM Lane LOS	B	A	-	C	F	B	A	-		
HCM 95th %tile Q(veh)	0.1	-	-	0.5	4.7	0.3	-	-		

## Intersection

Int Delay, s/veh 1.9

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	4	103	1	8	61	1	2	3	7	5	6	7
Future Vol, veh/h	4	103	1	8	61	1	2	3	7	5	6	7
Conflicting Peds, #/hr	0	0	19	19	0	0	8	0	0	0	0	8
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	81	81	81	81	81	81	81	81	81	81	81	81
Heavy Vehicles, %	0	3	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	5	127	1	10	75	1	2	4	9	6	7	9

Major/Minor	Major1	Major2			Minor1			Minor2				
Conflicting Flow All	76	0	0	147	0	0	269	253	147	240	253	84
Stage 1	-	-	-	-	-	-	157	157	-	96	96	-
Stage 2	-	-	-	-	-	-	112	96	-	144	157	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1536	-	-	1447	-	-	688	654	905	718	654	981
Stage 1	-	-	-	-	-	-	850	772	-	916	819	-
Stage 2	-	-	-	-	-	-	898	819	-	864	772	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1536	-	-	1421	-	-	653	635	889	702	635	974
Mov Cap-2 Maneuver	-	-	-	-	-	-	653	635	-	702	635	-
Stage 1	-	-	-	-	-	-	831	755	-	912	813	-
Stage 2	-	-	-	-	-	-	869	813	-	848	755	-

Approach	EB	WB			NB			SB			
HCM Control Delay, s	0.3	0.9			9.8			9.9			
HCM LOS					A			A			
<hr/>											
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBTn1	SBRn1	SBRn2
Capacity (veh/h)	766	1536	-	-	1421	-	-	758	-	-	-
HCM Lane V/C Ratio	0.019	0.003	-	-	0.007	-	-	0.029	-	-	-
HCM Control Delay (s)	9.8	7.4	0	-	7.6	0	-	9.9	-	-	-
HCM Lane LOS	A	A	A	-	A	A	-	A	-	-	-
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1	-	-	-

Intersection

Int Delay, s/veh 6.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	70	11	34	10	33	25	15	152	9	17	207	22
Future Vol, veh/h	70	11	34	10	33	25	15	152	9	17	207	22
Conflicting Peds, #/hr	0	0	19	19	0	0	53	0	66	66	0	53
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	80	80	80	80	80	80	80	80	80	80	80
Heavy Vehicles, %	0	0	8	0	0	0	0	0	0	0	0	0
Mvmt Flow	88	14	43	13	41	31	19	190	11	21	259	28

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	638	673	345	663	682	262	340	0	0	267	0	0
Stage 1	368	368	-	300	300	-	-	-	-	-	-	-
Stage 2	270	305	-	363	382	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.28	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.372	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	392	379	684	377	375	782	1230	-	-	1308	-	-
Stage 1	656	625	-	713	669	-	-	-	-	-	-	-
Stage 2	740	666	-	660	616	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	312	325	638	304	321	733	1168	-	-	1226	-	-
Mov Cap-2 Maneuver	312	325	-	304	321	-	-	-	-	-	-	-
Stage 1	611	582	-	656	615	-	-	-	-	-	-	-
Stage 2	649	613	-	579	573	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	20.8	16.4			0.7			0.6				
HCM LOS	C	C										
<hr/>												
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR				
Capacity (veh/h)	1168	-	-	369	400	1226	-	-				
HCM Lane V/C Ratio	0.016	-	-	0.39	0.213	0.017	-	-				
HCM Control Delay (s)	8.1	0	-	20.8	16.4	8	0	-				
HCM Lane LOS	A	A	-	C	C	A	A	-				
HCM 95th %tile Q(veh)	0	-	-	1.8	0.8	0.1	-	-				

## Intersection

Int Delay, s/veh 2.6

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	13	391	10	25	274	25	11	5	30	14	9	18
Future Vol, veh/h	13	391	10	25	274	25	11	5	30	14	9	18
Conflicting Peds, #/hr	22	0	49	49	0	22	17	0	7	7	0	17
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	79	79	79
Heavy Vehicles, %	5	0	0	12	1	0	0	0	0	0	1	10
Mvmt Flow	16	495	13	32	347	32	14	6	38	18	11	23

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	401	0	0	557	0	0	1044	1048	558	1012	1038	402
Stage 1	-	-	-	-	-	-	583	583	-	449	449	-
Stage 2	-	-	-	-	-	-	461	465	-	563	589	-
Critical Hdwy	4.15	-	-	4.22	-	-	7.1	6.5	6.2	7.1	6.51	6.3
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.51	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.51	-
Follow-up Hdwy	2.245	-	-	2.308	-	-	3.5	4	3.3	3.5	4.009	3.39
Pot Cap-1 Maneuver	1142	-	-	965	-	-	209	230	533	220	232	631
Stage 1	-	-	-	-	-	-	502	502	-	593	574	-
Stage 2	-	-	-	-	-	-	584	566	-	514	497	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1118	-	-	920	-	-	172	201	505	184	203	608
Mov Cap-2 Maneuver	-	-	-	-	-	-	172	201	-	184	203	-
Stage 1	-	-	-	-	-	-	469	469	-	569	537	-
Stage 2	-	-	-	-	-	-	517	530	-	456	464	-

Approach	EB	WB		NB		SB						
HCM Control Delay, s	0.3	0.7		19.3		21.3						
HCM LOS				C		C						
<hr/>												
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	310	1118	-	-	920	-	-	273				
HCM Lane V/C Ratio	0.188	0.015	-	-	0.034	-	-	0.19				
HCM Control Delay (s)	19.3	8.3	0	-	9.1	0	-	21.3				
HCM Lane LOS	C	A	A	-	A	A	-	C				
HCM 95th %tile Q(veh)	0.7	0	-	-	0.1	-	-	0.7				

## Intersection

Int Delay, s/veh 0.6

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	4	422	9	6	308	6	6	2	3	5	0	10
Future Vol, veh/h	4	422	9	6	308	6	6	2	3	5	0	10
Conflicting Peds, #/hr	8	0	10	10	0	8	1	0	5	5	0	1
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	4	469	10	7	342	7	7	2	3	6	0	11

Major/Minor	Major1	Major2		Minor1		Minor2		
Conflicting Flow All	357	0	0	489	0	0	858	863
Stage 1	-	-	-	-	-	-	492	492
Stage 2	-	-	-	-	-	-	366	371
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4
Pot Cap-1 Maneuver	1213	-	-	1085	-	-	279	295
Stage 1	-	-	-	-	-	-	562	551
Stage 2	-	-	-	-	-	-	657	623
Platoon blocked, %	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1204	-	-	1075	-	-	269	286
Mov Cap-2 Maneuver	-	-	-	-	-	-	269	286
Stage 1	-	-	-	-	-	-	554	543
Stage 2	-	-	-	-	-	-	641	613

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.1	0.2		16.7		13.2		
HCM LOS				C		B		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBC	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	319	1204	-	-	1075	-	-	454
HCM Lane V/C Ratio	0.038	0.004	-	-	0.006	-	-	0.037
HCM Control Delay (s)	16.7	8	0	-	8.4	0	-	13.2
HCM Lane LOS	C	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1

## Lanes, Volumes, Timings

### 1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑↑			↑↑
Traffic Volume (vph)	143	153	1182	52	0	1270
Future Volume (vph)	143	153	1182	52	0	1270
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	2000
Storage Length (ft)	110	0		0	0	
Storage Lanes	1	1		0	0	
Taper Length (ft)	145			25		
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Ped Bike Factor	0.88	0.98	1.00			
Fr <sub>t</sub>		0.850	0.994			
Flt Protected	0.950					
Satd. Flow (prot)	1752	1356	3504	0	0	3695
Flt Permitted	0.950					
Satd. Flow (perm)	1551	1327	3504	0	0	3695
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	30		30		30	
Link Distance (ft)	465		176		249	
Travel Time (s)	10.6		4.0		5.7	
Confl. Peds. (#/hr)	80	7		35	35	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	3%	3%	2%	0%	0%	1%
Bus Blockages (#/hr)	0	0	0	0	0	9
Parking (#/hr)		7				
Shared Lane Traffic (%)						
Lane Group Flow (vph)	147	158	1273	0	0	1309
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2		6	
Permitted Phases		8				
Detector Phase	8	8	2		6	
Switch Phase						
Minimum Initial (s)	8.0	8.0	15.0		15.0	
Minimum Split (s)	31.0	31.0	28.0		29.0	
Total Split (s)	36.0	36.0	84.0		84.0	
Total Split (%)	30.0%	30.0%	70.0%		70.0%	
Yellow Time (s)	4.0	4.0	4.0		4.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0		6.0	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Min		C-Min	
Act Effct Green (s)	20.4	20.4	87.6		87.6	
Actuated g/C Ratio	0.17	0.17	0.73		0.73	
v/c Ratio	0.49	0.70	0.50		0.49	
Control Delay	49.7	62.7	9.3		8.1	
Queue Delay	0.0	0.0	0.0		0.0	
Total Delay	49.7	62.7	9.3		8.1	
LOS	D	E	A		A	

Lanes, Volumes, Timings  
1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Approach Delay	56.4		9.3			8.1
Approach LOS		E		A		A
Queue Length 50th (ft)	104	117	355		195	
Queue Length 95th (ft)	158	179	482		305	
Internal Link Dist (ft)	385		96		169	
Turn Bay Length (ft)	110					
Base Capacity (vph)	438	331	2557		2697	
Starvation Cap Reductn	0	0	0		0	
Spillback Cap Reductn	0	0	0		0	
Storage Cap Reductn	0	0	0		0	
Reduced v/c Ratio	0.34	0.48	0.50		0.49	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 101 (84%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.70

Intersection Signal Delay: 13.8

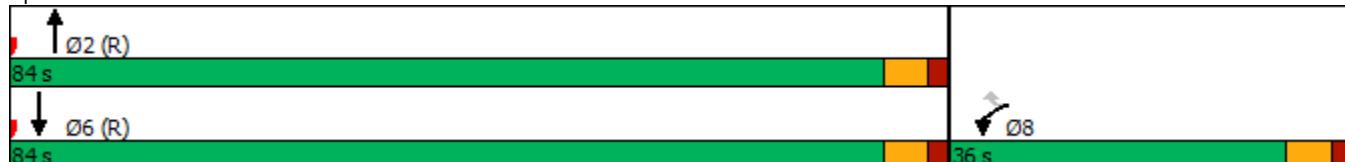
Intersection LOS: B

Intersection Capacity Utilization 56.9%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: Harlem Avenue & South Boulevard



## Lanes, Volumes, Timings

## 2: Harlem Avenue &amp; Randolph Street

10/07/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations	↑	↑		↑	↑		↑	↑↑		↑	↑↑			
Traffic Volume (vph)	155	158	74	65	138	34	86	1102	35	68	1090	89		
Future Volume (vph)	155	158	74	65	138	34	86	1102	35	68	1090	89		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	2000	1900	1900	2000	1900		
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12		
Grade (%)	0%			0%			0%			0%				
Storage Length (ft)	40	0		50	0		165	0		85	0			
Storage Lanes	1	0		1	0		1	0		1	0			
Taper Length (ft)	90	75			120			90						
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95		
Ped Bike Factor	0.99	0.99			0.99	0.99	1.00		1.00		1.00			
Fr <sub>t</sub>	0.952			0.970			0.995			0.989				
Flt Protected	0.950	0.950			0.950			0.950			0.950			
Satd. Flow (prot)	1787	1771	0	1805	1831	0	1805	3705	0	1805	3645	0		
Flt Permitted	0.433	0.407			0.128			0.153						
Satd. Flow (perm)	803	1771	0	767	1831	0	243	3705	0	291	3645	0		
Right Turn on Red	No			No			No			No				
Satd. Flow (RTOR)														
Link Speed (mph)	30			30			30			30				
Link Distance (ft)	454			257			242			813				
Travel Time (s)	10.3			5.8			5.5			18.5				
Confl. Peds. (#/hr)	14	10		10	14		10	6		6	6			
Confl. Bikes (#/hr)	1													
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95		
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%		
Heavy Vehicles (%)	1%	0%	4%	0%	0%	0%	0%	2%	0%	0%	1%	0%		
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	9	0		
Parking (#/hr)														
Mid-Block Traffic (%)	0%			0%			0%			0%				
Shared Lane Traffic (%)														
Lane Group Flow (vph)	163	244	0	68	181	0	91	1197	0	72	1241	0		
Turn Type	pm+pt	NA	pm+pt		NA	pm+pt		NA	pm+pt		NA			
Protected Phases	7	4	3		8	5		2	1		6			
Permitted Phases	4	8		2		6								
Detector Phase	7	4	3		8	5		2	1		6			
Switch Phase														
Minimum Initial (s)	3.0	8.0	3.0		8.0	3.0		15.0	3.0		15.0			
Minimum Split (s)	9.5	30.0	9.5		30.0	9.5		29.0	10.5		33.0			
Total Split (s)	13.0	30.0	13.0		30.0	13.0		64.0	13.0		64.0			
Total Split (%)	10.8%	25.0%	10.8%		25.0%	10.8%		53.3%	10.8%		53.3%			
Yellow Time (s)	3.5	4.0	3.5		4.0	3.5		4.0	3.5		4.0			
All-Red Time (s)	0.0	2.0	0.0		2.0	0.0		2.0	0.0		2.0			
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0		0.0	0.0		0.0			
Total Lost Time (s)	3.5	6.0	3.5		6.0	3.5		6.0	3.5		6.0			
Lead/Lag	Lead	Lag	Lead		Lag	Lead		Lag	Lead		Lag			
Lead-Lag Optimize?	Yes	Yes	Yes		Yes	Yes		Yes	Yes		Yes			
Recall Mode	None	None	None		None	C-Min		None	C-Min					
Act Effct Green (s)	33.1	23.0	30.6		19.9	75.2		66.1	73.7		63.9			
Actuated g/C Ratio	0.28	0.19	0.26		0.17	0.63		0.55	0.61		0.53			

Lanes, Volumes, Timings  
2: Harlem Avenue & Randolph Street

10/07/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.55	0.72		0.25	0.60		0.36	0.59		0.27	0.64	
Control Delay	39.7	58.5		32.0	54.0		12.9	20.9		14.8	30.9	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	39.7	58.5		32.0	54.0		12.9	20.9		14.8	30.9	
LOS	D	E		C	D		B	C		B	C	
Approach Delay		51.0			48.0			20.3			30.0	
Approach LOS		D			D			C			C	
Queue Length 50th (ft)	96	180		38	129		25	333		30	447	
Queue Length 95th (ft)	150	270		71	200		49	435		60	551	
Internal Link Dist (ft)		374			177			162			733	
Turn Bay Length (ft)	40		50			165				85		
Base Capacity (vph)	299	357		286	366		278	2041		303	1939	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.55	0.68		0.24	0.49		0.33	0.59		0.24	0.64	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 56 (47%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.72

Intersection Signal Delay: 30.2

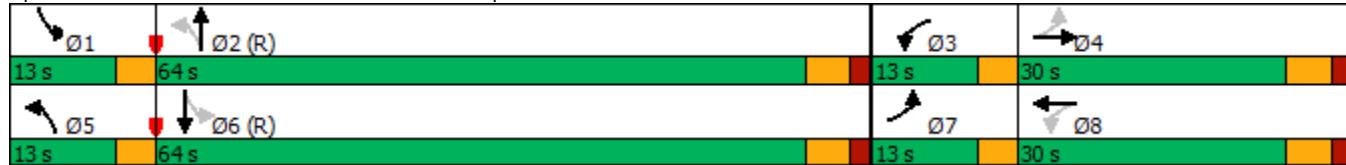
Intersection LOS: C

Intersection Capacity Utilization 74.9%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 2: Harlem Avenue & Randolph Street



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Intersection

Intersection Delay, s/veh 7.6

Intersection LOS A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Vol, veh/h	9	88	7	22	19	16	11	5	26	6	6	8
Future Vol, veh/h	9	88	7	22	19	16	11	5	26	6	6	8
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Heavy Vehicles, %	0	2	0	0	7	17	0	0	0	17	17	0
Mvmt Flow	11	111	9	28	24	20	14	6	33	8	8	10
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	7.8			7.5			7.3			7.7		
HCM LOS	A			A			A			A		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	26%	9%	39%	30%
Vol Thru, %	12%	85%	33%	30%
Vol Right, %	62%	7%	28%	40%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	42	104	57	20
LT Vol	11	9	22	6
Through Vol	5	88	19	6
RT Vol	26	7	16	8
Lane Flow Rate	53	132	72	25
Geometry Grp	1	1	1	1
Degree of Util (X)	0.06	0.149	0.081	0.032
Departure Headway (Hd)	4.051	4.069	4.046	4.507
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	889	874	875	799
Service Time	2.052	2.127	2.118	2.509
HCM Lane V/C Ratio	0.06	0.151	0.082	0.031
HCM Control Delay	7.3	7.8	7.5	7.7
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	0.2	0.5	0.3	0.1

## Intersection

Intersection Delay, s/veh 11.6

Intersection LOS B

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	60	198	23	8	199	5	11	76	13	43	105	44
Future Vol, veh/h	60	198	23	8	199	5	11	76	13	43	105	44
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	3	0	0
Mvmt Flow	67	222	26	9	224	6	12	85	15	48	118	49
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	12.6			11.2			10			11.3		
HCM LOS	B			B			A			B		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	11%	21%	4%	22%
Vol Thru, %	76%	70%	94%	55%
Vol Right, %	13%	8%	2%	23%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	100	281	212	192
LT Vol	11	60	8	43
Through Vol	76	198	199	105
RT Vol	13	23	5	44
Lane Flow Rate	112	316	238	216
Geometry Grp	1	1	1	1
Degree of Util (X)	0.178	0.456	0.352	0.331
Departure Headway (Hd)	5.708	5.203	5.315	5.526
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	627	691	676	649
Service Time	3.759	3.242	3.357	3.569
HCM Lane V/C Ratio	0.179	0.457	0.352	0.333
HCM Control Delay	10	12.6	11.2	11.3
HCM Lane LOS	A	B	B	B
HCM 95th-tile Q	0.6	2.4	1.6	1.4

**Intersection**

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	31	0	1234	1253	160
Future Vol, veh/h	0	31	0	1234	1253	160
Conflicting Peds, #/hr	7	80	58	0	0	58
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	0	0	2	1	0
Mvmt Flow	0	32	0	1272	1292	165

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	-	867	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	6.9	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	3.3	-
Pot Cap-1 Maneuver	0	300	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	262	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

**Approach**

EB NB SB

HCM Control Delay, s 20.6 0 0

HCM LOS C

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	262	-	-
HCM Lane V/C Ratio	-	0.122	-	-
HCM Control Delay (s)	-	20.6	-	-
HCM Lane LOS	-	C	-	-
HCM 95th %tile Q(veh)	-	0.4	-	-

## Intersection

Int Delay, s/veh 0.4

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑		↔	↔		
Traffic Vol, veh/h	51	4	6	289	4	4
Future Vol, veh/h	51	4	6	289	4	4
Conflicting Peds, #/hr	0	102	102	0	13	2
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	88	88	88	88	88	88
Heavy Vehicles, %	0	0	0	3	0	0
Mvmt Flow	58	5	7	328	5	5

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	165	0	518 165
Stage 1	-	-	-	-	163 -
Stage 2	-	-	-	-	355 -
Critical Hdwy	-	-	4.1	-	6.4 6.2
Critical Hdwy Stg 1	-	-	-	-	5.4 -
Critical Hdwy Stg 2	-	-	-	-	5.4 -
Follow-up Hdwy	-	-	2.2	-	3.5 3.3
Pot Cap-1 Maneuver	-	-	1426	-	521 885
Stage 1	-	-	-	-	871 -
Stage 2	-	-	-	-	714 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1287	-	462 798
Mov Cap-2 Maneuver	-	-	-	-	462 -
Stage 1	-	-	-	-	781 -
Stage 2	-	-	-	-	705 -

Approach	EB	WB	NB
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HCM Control Delay, s 0 0.2 11.3

HCM LOS B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	585	-	-	1287	-
HCM Lane V/C Ratio	0.016	-	-	0.005	-
HCM Control Delay (s)	11.3	-	-	7.8	0
HCM Lane LOS	B	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

## Intersection

Int Delay, s/veh 4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	0	0	37	1	5	32	19	1202	70	34	1209	41
Future Vol, veh/h	0	0	37	1	5	32	19	1202	70	34	1209	41
Conflicting Peds, #/hr	0	0	0	0	0	0	4	0	10	10	0	4
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	0	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	5	0	1	2	0	1	0
Mvmt Flow	0	0	40	1	5	35	21	1307	76	37	1314	45

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	-	-	684	2128	2834	702	1363	0	0	1393	0	0
Stage 1	-	-	-	1397	1397	-	-	-	-	-	-	
Stage 2	-	-	-	731	1437	-	-	-	-	-	-	
Critical Hdwy	-	-	6.9	7.5	6.5	7	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	-	-	-	6.5	5.5	-	-	-	-	-	-	
Critical Hdwy Stg 2	-	-	-	6.5	5.5	-	-	-	-	-	-	
Follow-up Hdwy	-	-	3.3	3.5	4	3.35	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	0	0	396	29	18	374	511	-	-	497	-	-
Stage 1	0	0	-	151	210	-	-	-	-	-	-	
Stage 2	0	0	-	384	201	-	-	-	-	-	-	
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	
Mov Cap-1 Maneuver	-	-	394	17	10	370	509	-	-	492	-	-
Mov Cap-2 Maneuver	-	-	-	17	10	-	-	-	-	-	-	
Stage 1	-	-	-	122	169	-	-	-	-	-	-	
Stage 2	-	-	-	237	137	-	-	-	-	-	-	

Approach	EB	WB			NB			SB		
HCM Control Delay, s	15.2	152.7			1.2			2.1		
HCM LOS	C	F								
<hr/>										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR		
Capacity (veh/h)	509	-	-	394	59	492	-	-		
HCM Lane V/C Ratio	0.041	-	-	0.102	0.7	0.075	-	-		
HCM Control Delay (s)	12.4	1.1	-	15.2	152.7	12.9	1.9	-		
HCM Lane LOS	B	A	-	C	F	B	A	-		
HCM 95th %tile Q(veh)	0.1	-	-	0.3	3	0.2	-	-		

## Intersection

Int Delay, s/veh

1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	5	114	1	4	50	3	2	0	2	2	3	5
Future Vol, veh/h	5	114	1	4	50	3	2	0	2	2	3	5
Conflicting Peds, #/hr	5	0	13	13	0	5	7	0	6	6	0	7
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	2	0	0	3	0	0	0	0	0	0	20
Mvmt Flow	5	124	1	4	54	3	2	0	2	2	3	5

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	62	0	0	138	0	0	223	218	144	211	217	68
Stage 1	-	-	-	-	-	-	148	148	-	69	69	-
Stage 2	-	-	-	-	-	-	75	70	-	142	148	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.4
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.48
Pot Cap-1 Maneuver	1554	-	-	1458	-	-	737	684	909	750	685	947
Stage 1	-	-	-	-	-	-	859	779	-	946	841	-
Stage 2	-	-	-	-	-	-	939	841	-	866	779	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1547	-	-	1440	-	-	713	668	893	737	669	936
Mov Cap-2 Maneuver	-	-	-	-	-	-	713	668	-	737	669	-
Stage 1	-	-	-	-	-	-	846	767	-	938	834	-
Stage 2	-	-	-	-	-	-	921	834	-	856	767	-

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.3	0.5		9.6		9.6		
HCM LOS				A		A		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	793	1547	-	-	1440	-	-	797
HCM Lane V/C Ratio	0.005	0.004	-	-	0.003	-	-	0.014
HCM Control Delay (s)	9.6	7.3	0	-	7.5	0	-	9.6
HCM Lane LOS	A	A	A	-	A	A	-	A
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0

Intersection

Int Delay, s/veh 5.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	76	20	22	17	23	22	8	126	7	16	153	26
Future Vol, veh/h	76	20	22	17	23	22	8	126	7	16	153	26
Conflicting Peds, #/hr	0	0	24	24	0	0	71	0	53	53	0	71
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	0	0	2	0	12	0	0	0	0	0	0	0
Mvmt Flow	85	22	25	19	26	25	9	142	8	18	172	29

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	484	515	282	487	525	199	272	0	0	203	0	0
Stage 1	294	294	-	217	217	-	-	-	-	-	-	-
Stage 2	190	221	-	270	308	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.22	7.1	6.62	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.62	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.62	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.318	3.5	4.108	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	496	466	757	494	444	847	1303	-	-	1381	-	-
Stage 1	719	673	-	790	705	-	-	-	-	-	-	-
Stage 2	816	724	-	740	643	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	418	403	690	415	384	804	1215	-	-	1311	-	-
Mov Cap-2 Maneuver	418	403	-	415	384	-	-	-	-	-	-	-
Stage 1	665	617	-	744	664	-	-	-	-	-	-	-
Stage 2	754	682	-	661	590	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	16.4	13.7			0.5			0.6				
HCM LOS	C	B										
<hr/>												
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR				
Capacity (veh/h)	1215	-	-	448	484	1311	-	-				
HCM Lane V/C Ratio	0.007	-	-	0.296	0.144	0.014	-	-				
HCM Control Delay (s)	8	0	-	16.4	13.7	7.8	0	-				
HCM Lane LOS	A	A	-	C	B	A	A	-				
HCM 95th %tile Q(veh)	0	-	-	1.2	0.5	0	-	-				

## Intersection

Int Delay, s/veh 2.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	7	250	4	21	223	16	5	19	27	9	17	9
Future Vol, veh/h	7	250	4	21	223	16	5	19	27	9	17	9
Conflicting Peds, #/hr	10	0	6	6	0	10	8	0	13	13	0	8
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	87	87	87	87	87	87	87	87	87	87	87	87
Heavy Vehicles, %	0	18	0	0	1	0	0	0	0	0	1	0
Mvmt Flow	8	287	5	24	256	18	6	22	31	10	20	10

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	284	0	0	298	0	0	648	644	309	668	637	283
Stage 1	-	-	-	-	-	-	312	312	-	323	323	-
Stage 2	-	-	-	-	-	-	336	332	-	345	314	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.51	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.51	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.51	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4.009	3.3
Pot Cap-1 Maneuver	1290	-	-	1275	-	-	386	394	736	375	396	761
Stage 1	-	-	-	-	-	-	703	661	-	693	652	-
Stage 2	-	-	-	-	-	-	682	648	-	675	658	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1278	-	-	1268	-	-	353	377	723	328	379	748
Mov Cap-2 Maneuver	-	-	-	-	-	-	353	377	-	328	379	-
Stage 1	-	-	-	-	-	-	694	652	-	682	631	-
Stage 2	-	-	-	-	-	-	633	627	-	612	649	-

Approach	EB	WB		NB		SB	
HCM Control Delay, s	0.2	0.6		13.2		14.6	
HCM LOS				B		B	

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	500	1278	-	-	1268	-	-	415
HCM Lane V/C Ratio	0.117	0.006	-	-	0.019	-	-	0.097
HCM Control Delay (s)	13.2	7.8	0	-	7.9	0	-	14.6
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.4	0	-	-	0.1	-	-	0.3

## Intersection

Int Delay, s/veh 0.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	3	280	3	1	252	1	1	0	1	0	1	7
Future Vol, veh/h	3	280	3	1	252	1	1	0	1	0	1	7
Conflicting Peds, #/hr	23	0	12	12	0	23	5	0	1	1	0	5
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	4	329	4	1	296	1	1	0	1	0	1	8

Major/Minor	Major1	Major2		Minor1		Minor2		
Conflicting Flow All	320	0	0	345	0	0	659	673
Stage 1	-	-	-	-	-	-	351	351
Stage 2	-	-	-	-	-	-	308	322
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4
Pot Cap-1 Maneuver	1251	-	-	1225	-	-	380	379
Stage 1	-	-	-	-	-	-	670	636
Stage 2	-	-	-	-	-	-	706	655
Platoon blocked, %	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1224	-	-	1211	-	-	367	365
Mov Cap-2 Maneuver	-	-	-	-	-	-	367	365
Stage 1	-	-	-	-	-	-	660	626
Stage 2	-	-	-	-	-	-	692	640

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.1	0		12.5		10.8		
HCM LOS				B		B		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBC	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	480	1224	-	-	1211	-	-	629
HCM Lane V/C Ratio	0.005	0.003	-	-	0.001	-	-	0.015
HCM Control Delay (s)	12.5	8	0	-	8	0	-	10.8
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0

Lanes, Volumes, Timings  
1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑↑			↑↑
Traffic Volume (vph)	147	83	1260	19	0	1463
Future Volume (vph)	147	83	1260	19	0	1463
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	2000
Storage Length (ft)	110	0		0	0	
Storage Lanes	1	1		0	0	
Taper Length (ft)	145			25		
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Ped Bike Factor	0.92	0.97	1.00			
Fr <sub>t</sub>		0.850	0.998			
Flt Protected	0.950					
Satd. Flow (prot)	1752	1225	3361	0	0	3623
Flt Permitted	0.950					
Satd. Flow (perm)	1605	1185	3361	0	0	3623
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	30		30			30
Link Distance (ft)	465		176			249
Travel Time (s)	10.6		4.0			5.7
Confl. Peds. (#/hr)	56	13		39	39	
Confl. Bikes (#/hr)		2				
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	14%	7%	7%	0%	3%
Bus Blockages (#/hr)	0	0	0	0	0	9
Parking (#/hr)		7				
Shared Lane Traffic (%)						
Lane Group Flow (vph)	160	90	1391	0	0	1590
Turn Type	Prot	Perm	NA			NA
Protected Phases	8		2			6
Permitted Phases		8				
Detector Phase	8	8	2			6
Switch Phase						
Minimum Initial (s)	8.0	8.0	15.0			15.0
Minimum Split (s)	31.0	31.0	28.0			29.0
Total Split (s)	38.0	38.0	87.0			87.0
Total Split (%)	30.4%	30.4%	69.6%			69.6%
Yellow Time (s)	4.0	4.0	4.0			4.0
All-Red Time (s)	2.0	2.0	2.0			2.0
Lost Time Adjust (s)	0.0	0.0	0.0			0.0
Total Lost Time (s)	6.0	6.0	6.0			6.0
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Min			C-Min
Act Effct Green (s)	17.6	17.6	95.4			95.4
Actuated g/C Ratio	0.14	0.14	0.76			0.76
v/c Ratio	0.65	0.54	0.54			0.58
Control Delay	62.4	60.9	14.5			7.8
Queue Delay	0.0	0.0	0.0			0.0
Total Delay	62.4	60.9	14.5			7.8

Lanes, Volumes, Timings  
1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
LOS	E	E	B			A
Approach Delay	61.8		14.5			7.8
Approach LOS	E		B			A
Queue Length 50th (ft)	124	69	474			246
Queue Length 95th (ft)	188	120	613			366
Internal Link Dist (ft)	385		96			169
Turn Bay Length (ft)	110					
Base Capacity (vph)	448	303	2564			2763
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.36	0.30	0.54			0.58

Intersection Summary

Area Type: Other

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 103 (82%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.65

Intersection Signal Delay: 14.8

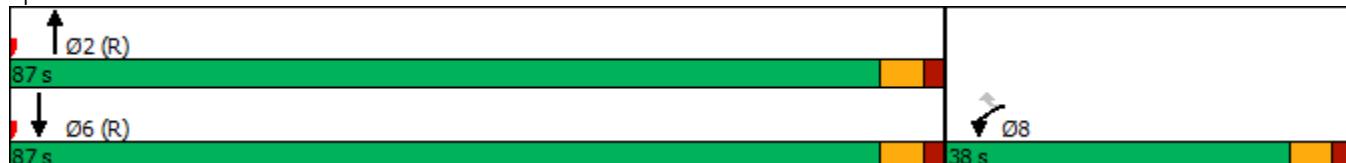
Intersection LOS: B

Intersection Capacity Utilization 61.0%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: Harlem Avenue & South Boulevard



## Lanes, Volumes, Timings

## 2: Harlem Avenue &amp; Randolph Street

01/28/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑↑		↑	↑↑	
Traffic Volume (vph)	130	151	119	94	114	24	83	1169	38	32	1369	82
Future Volume (vph)	130	151	119	94	114	24	83	1169	38	32	1369	82
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	40		0	50		0	165		0	85		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	90			75			120			90		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	0.98	0.99		0.99	0.99			1.00			1.00	
Fr <sub>t</sub>		0.934			0.974			0.995			0.992	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1787	1724	0	1805	1808	0	1736	3531	0	1736	3581	0
Flt Permitted	0.562			0.272			0.059			0.145		
Satd. Flow (perm)	1041	1724	0	512	1808	0	108	3531	0	265	3581	0
Right Turn on Red			No			No			No		No	
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		454			257			242			813	
Travel Time (s)		10.3			5.8			5.5			18.5	
Confl. Peds. (#/hr)	14		12	12		14	7		7	7		7
Confl. Bikes (#/hr)					7							
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	1%	2%	1%	0%	2%	0%	4%	7%	5%	4%	3%	6%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	9	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	135	281	0	98	144	0	86	1258	0	33	1511	0
Turn Type	pm+pt	NA										
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	3.0	8.0		3.0	8.0		3.0	15.0		3.0	15.0	
Minimum Split (s)	9.5	30.0		9.5	32.0		9.5	29.0		10.5	33.0	
Total Split (s)	14.0	30.0		14.0	30.0		17.0	67.0		14.0	64.0	
Total Split (%)	11.2%	24.0%		11.2%	24.0%		13.6%	53.6%		11.2%	51.2%	
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0		3.5	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	
Lead/Lag	Lead	Lag										
Lead-Lag Optimize?	Yes	Yes										
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effct Green (s)	35.8	23.3		34.6	22.6		78.6	70.6		74.0	65.1	
Actuated g/C Ratio	0.29	0.19		0.28	0.18		0.63	0.56		0.59	0.52	
v/c Ratio	0.38	0.88		0.41	0.44		0.49	0.63		0.14	0.81	
Control Delay	35.1	76.7		36.3	49.8		24.3	21.5		12.4	36.6	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	35.1	76.7		36.3	49.8		24.3	21.5		12.4	36.6	
LOS	D	E		D	D		C	C		B	D	

Lanes, Volumes, Timings  
2: Harlem Avenue & Randolph Street

01/28/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		63.2			44.3			21.7			36.1	
Approach LOS		E			D			C			D	
Queue Length 50th (ft)	80	221		57	103		26	385		14	617	
Queue Length 95th (ft)	133	#371		100	170		69	474	m23	716		
Internal Link Dist (ft)		374			177			162			733	
Turn Bay Length (ft)	40			50			165			85		
Base Capacity (vph)	364	332		254	347		244	1992		288	1864	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.37	0.85		0.39	0.41		0.35	0.63		0.11	0.81	

Intersection Summary

Area Type: Other

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 51 (41%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 95

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.88

Intersection Signal Delay: 34.4

Intersection LOS: C

Intersection Capacity Utilization 82.1%

ICU Level of Service E

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Harlem Avenue & Randolph Street



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Intersection

Intersection Delay, s/veh 7.7

Intersection LOS A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	9	83	14	8	27	0	16	7	18	2	2	2
Future Vol, veh/h	9	83	14	8	27	0	16	7	18	2	2	2
Peak Hour Factor	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67	0.67
Heavy Vehicles, %	0	0	0	0	0	0	0	25	12	0	0	14
Mvmt Flow	13	124	21	12	40	0	24	10	27	3	3	3
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	7.9			7.5			7.5			7.3		
HCM LOS	A			A			A			A		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	39%	8%	23%	33%
Vol Thru, %	17%	78%	77%	33%
Vol Right, %	44%	13%	0%	33%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	41	106	35	6
LT Vol	16	9	8	2
Through Vol	7	83	27	2
RT Vol	18	14	0	2
Lane Flow Rate	61	158	52	9
Geometry Grp	1	1	1	1
Degree of Util (X)	0.069	0.176	0.061	0.011
Departure Headway (Hd)	4.083	3.998	4.188	4.288
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	862	892	847	840
Service Time	2.181	2.047	2.255	2.288
HCM Lane V/C Ratio	0.071	0.177	0.061	0.011
HCM Control Delay	7.5	7.9	7.5	7.3
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	0.2	0.6	0.2	0

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Intersection

Intersection Delay, s/veh 9.6

Intersection LOS A

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	40	160	27	6	167	14	18	51	4	8	71	50
Future Vol, veh/h	40	160	27	6	167	14	18	51	4	8	71	50
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	0	1	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	44	178	30	7	186	16	20	57	4	9	79	56
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	10			9.6			9			9.1		
HCM LOS	A			A			A			A		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	25%	18%	3%	6%
Vol Thru, %	70%	70%	89%	55%
Vol Right, %	5%	12%	7%	39%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	73	227	187	129
LT Vol	18	40	6	8
Through Vol	51	160	167	71
RT Vol	4	27	14	50
Lane Flow Rate	81	252	208	143
Geometry Grp	1	1	1	1
Degree of Util (X)	0.117	0.327	0.272	0.193
Departure Headway (Hd)	5.181	4.668	4.717	4.86
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	686	767	756	732
Service Time	3.257	2.725	2.777	2.928
HCM Lane V/C Ratio	0.118	0.329	0.275	0.195
HCM Control Delay	9	10	9.6	9.1
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	0.4	1.4	1.1	0.7

**Intersection**

Int Delay, s/veh 0.2

Movement	EBL	EBC	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	18	0	1279	1486	124
Future Vol, veh/h	0	18	0	1279	1486	124
Conflicting Peds, #/hr	13	56	91	0	0	91
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	14	0	7	3	2
Mvmt Flow	0	20	0	1390	1615	135

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	-	1022	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	7.18	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	3.44	-
Pot Cap-1 Maneuver	0	214	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	185	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

**Approach**

EB NB SB

HCM Control Delay, s 26.7 0 0

HCM LOS D

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	185	-	-
HCM Lane V/C Ratio	-	0.106	-	-
HCM Control Delay (s)	-	26.7	-	-
HCM Lane LOS	-	D	-	-
HCM 95th %tile Q(veh)	-	0.3	-	-

**Intersection**

Int Delay, s/veh 0.1

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑		↔	↔		
Traffic Vol, veh/h	25	0	1	222	0	2
Future Vol, veh/h	25	0	1	222	0	2
Conflicting Peds, #/hr	0	24	24	0	1	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	96	96	96	96	96	96
Heavy Vehicles, %	8	0	0	9	0	0
Mvmt Flow	26	0	1	231	0	2

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	50	0	284 50
Stage 1	-	-	-	-	50 -
Stage 2	-	-	-	-	234 -
Critical Hdwy	-	-	4.1	-	6.4 6.2
Critical Hdwy Stg 1	-	-	-	-	5.4 -
Critical Hdwy Stg 2	-	-	-	-	5.4 -
Follow-up Hdwy	-	-	2.2	-	3.5 3.3
Pot Cap-1 Maneuver	-	-	1570	-	710 1024
Stage 1	-	-	-	-	978 -
Stage 2	-	-	-	-	810 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1534	-	692 1001
Mov Cap-2 Maneuver	-	-	-	-	692 -
Stage 1	-	-	-	-	956 -
Stage 2	-	-	-	-	808 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0	8.6
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	1001	-	-	1534	-
HCM Lane V/C Ratio	0.002	-	-	0.001	-
HCM Control Delay (s)	8.6	-	-	7.3	0
HCM Lane LOS	A	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

## Intersection

Int Delay, s/veh 59.9

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
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## Lane Configurations

Traffic Vol, veh/h	0	0	49	6	5	34	34	1245	44	62	1428	14
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Future Vol, veh/h	0	0	49	6	5	34	34	1245	44	62	1428	14
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Conflicting Peds, #/hr	1	0	0	0	0	1	22	0	6	6	0	22
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Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
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RT Channelized	-	-	None									
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Storage Length	-	-	0	-	-	-	-	-	-	-	-	-
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Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
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Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
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Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
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Heavy Vehicles, %	0	0	3	0	0	6	0	7	0	0	4	0
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Mvmt Flow	0	0	52	6	5	36	36	1311	46	65	1503	15
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Major/Minor	Minor2	Minor1			Major1			Major2		
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Conflicting Flow All	-	-	781	2294	3082	686	1540	0	0	1363	0	0
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Stage 1	-	-	-	1412	1412	-	-	-	-	-	-	-
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Stage 2	-	-	-	882	1670	-	-	-	-	-	-	-
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Critical Hdwy	-	-	6.96	7.5	6.5	7.02	4.1	-	-	4.1	-	-
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Critical Hdwy Stg 1	-	-	-	6.5	5.5	-	-	-	-	-	-	-
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Critical Hdwy Stg 2	-	-	-	6.5	5.5	-	-	-	-	-	-	-
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Follow-up Hdwy	-	-	3.33	3.5	4	3.36	2.2	-	-	2.2	-	-
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Pot Cap-1 Maneuver	0	0	335	22	12	381	437	-	-	511	-	-
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Stage 1	0	0	-	148	206	-	-	-	-	-	-	-
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Stage 2	0	0	-	312	154	-	-	-	-	-	-	-
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Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
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Mov Cap-1 Maneuver	-	-	328	~ 4	~ 1	378	428	-	-	508	-	-
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Mov Cap-2 Maneuver	-	-	-	~ 4	~ 1	-	-	-	-	-	-	-
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Stage 1	-	-	-	96	133	-	-	-	-	-	-	-
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Stage 2	-	-	-	46	26	-	-	-	-	-	-	-
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Approach	EB	WB			NB			SB		
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HCM Control Delay, s	18	\$ 3619.5			2.5			5.3		
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HCM LOS	C	F								
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Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
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Capacity (veh/h)	428	-	-	328	7	508	-	-
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HCM Lane V/C Ratio	0.084	-	-	0.157	6.767	0.128	-	-
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HCM Control Delay (s)	14.2	2.3	-	\$ 3619.5	13.1	5	-	-
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HCM Lane LOS	B	A	-	C	F	B	A	-
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HCM 95th %tile Q(veh)	0.3	-	-	0.6	7.4	0.4	-	-
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## Notes

~: Volume exceeds capacity    \$: Delay exceeds 300s    +: Computation Not Defined    \*: All major volume in platoon

## Intersection

Int Delay, s/veh 1.9

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	2	95	6	10	23	0	11	0	9	0	0	1
Future Vol, veh/h	2	95	6	10	23	0	11	0	9	0	0	1
Conflicting Peds, #/hr	6	0	26	26	0	6	2	0	0	0	0	2
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	77	77	77	77	77	77	77	77	77	77	77	77
Heavy Vehicles, %	0	2	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	3	123	8	13	30	0	14	0	12	0	0	1

Major/Minor	Major1	Major2		Minor1		Minor2		
Conflicting Flow All	36	0	0	157	0	0	218	221
Stage 1	-	-	-	-	-	-	159	159
Stage 2	-	-	-	-	-	-	59	62
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4
Pot Cap-1 Maneuver	1588	-	-	1435	-	-	743	681
Stage 1	-	-	-	-	-	-	848	770
Stage 2	-	-	-	-	-	-	958	847
Platoon blocked, %	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1579	-	-	1399	-	-	716	653
Mov Cap-2 Maneuver	-	-	-	-	-	-	716	653
Stage 1	-	-	-	-	-	-	825	749
Stage 2	-	-	-	-	-	-	946	834

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.1	2.3		9.8		8.5		
HCM LOS				A		A		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	780	1579	-	-	1399	-	-	1032
HCM Lane V/C Ratio	0.033	0.002	-	-	0.009	-	-	0.001
HCM Control Delay (s)	9.8	7.3	0	-	7.6	0	-	8.5
HCM Lane LOS	A	A	A	-	A	A	-	A
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0

Intersection

Int Delay, s/veh 5.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	59	15	30	4	23	3	2	94	9	4	95	8
Future Vol, veh/h	59	15	30	4	23	3	2	94	9	4	95	8
Conflicting Peds, #/hr	0	0	92	30	0	0	46	0	117	117	0	46
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	3	0	0	0	0	0	0	2	0	67	0	0
Mvmt Flow	69	17	35	5	27	3	2	109	10	5	110	9

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	304	411	253	478	410	231	165	0	0	236	0	0
Stage 1	171	171	-	235	235	-	-	-	-	-	-	-
Stage 2	133	240	-	243	175	-	-	-	-	-	-	-
Critical Hdwy	7.13	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.77	-	-
Critical Hdwy Stg 1	6.13	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.13	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.527	4	3.3	3.5	4	3.3	2.2	-	-	2.803	-	-
Pot Cap-1 Maneuver	646	534	791	501	534	813	1426	-	-	1028	-	-
Stage 1	829	761	-	773	714	-	-	-	-	-	-	-
Stage 2	868	711	-	765	758	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	583	450	690	372	450	722	1364	-	-	913	-	-
Mov Cap-2 Maneuver	583	450	-	372	450	-	-	-	-	-	-	-
Stage 1	791	723	-	686	633	-	-	-	-	-	-	-
Stage 2	826	631	-	643	720	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	12.8	13.6			0.1			0.3		
HCM LOS	B	B								
<hr/>										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR		
Capacity (veh/h)	1364	-	-	584	454	913	-	-		
HCM Lane V/C Ratio	0.002	-	-	0.207	0.077	0.005	-	-		
HCM Control Delay (s)	7.6	0	-	12.8	13.6	9	0	-		
HCM Lane LOS	A	A	-	B	B	A	A	-		
HCM 95th %tile Q(veh)	0	-	-	0.8	0.2	0	-	-		

## Intersection

Int Delay, s/veh 1.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	17	199	5	9	220	18	3	6	23	5	10	9
Future Vol, veh/h	17	199	5	9	220	18	3	6	23	5	10	9
Conflicting Peds, #/hr	22	0	34	34	0	22	9	0	8	8	0	9
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	0	0	33	0	3	0	0	0	0	20	12	0
Mvmt Flow	20	231	6	10	256	21	3	7	27	6	12	10

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	299	0	0	271	0	0	615	627	276	608	620	298
Stage 1	-	-	-	-	-	-	308	308	-	309	309	-
Stage 2	-	-	-	-	-	-	307	319	-	299	311	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.3	6.62	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.3	5.62	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.3	5.62	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.68	4.108	3.3
Pot Cap-1 Maneuver	1274	-	-	1304	-	-	406	403	768	383	391	746
Stage 1	-	-	-	-	-	-	706	664	-	664	642	-
Stage 2	-	-	-	-	-	-	707	657	-	673	641	-
Platoon blocked, %	-	-	-	-	-	-						
Mov Cap-1 Maneuver	1247	-	-	1262	-	-	367	372	737	346	361	724
Mov Cap-2 Maneuver	-	-	-	-	-	-	367	372	-	346	361	-
Stage 1	-	-	-	-	-	-	671	631	-	638	623	-
Stage 2	-	-	-	-	-	-	672	637	-	625	610	-

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.6	0.3		11.7		13.7		
HCM LOS				B		B		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	576	1247	-	-	1262	-	-	440
HCM Lane V/C Ratio	0.065	0.016	-	-	0.008	-	-	0.063
HCM Control Delay (s)	11.7	7.9	0	-	7.9	0	-	13.7
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.2	0	-	-	0	-	-	0.2

## Intersection

Int Delay, s/veh 0.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	5	219	3	0	234	1	2	2	0	8	2	11
Future Vol, veh/h	5	219	3	0	234	1	2	2	0	8	2	11
Conflicting Peds, #/hr	26	0	26	26	0	26	1	0	0	0	0	1
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	0	2	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	6	246	3	0	263	1	2	2	0	9	2	12

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	290	0	0	275	0	0	558	576	274	551	577	291
Stage 1	-	-	-	-	-	-	286	286	-	290	290	-
Stage 2	-	-	-	-	-	-	272	290	-	261	287	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1283	-	-	1300	-	-	443	431	770	448	430	753
Stage 1	-	-	-	-	-	-	726	679	-	722	676	-
Stage 2	-	-	-	-	-	-	738	676	-	748	678	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1251	-	-	1268	-	-	420	407	751	433	406	734
Mov Cap-2 Maneuver	-	-	-	-	-	-	420	407	-	433	406	-
Stage 1	-	-	-	-	-	-	703	658	-	700	659	-
Stage 2	-	-	-	-	-	-	722	659	-	741	657	-

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.2	0		13.8		11.9		
HCM LOS				B		B		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	413	1251	-	-	1268	-	-	547
HCM Lane V/C Ratio	0.011	0.004	-	-	-	-	-	0.043
HCM Control Delay (s)	13.8	7.9	0	-	0	-	-	11.9
HCM Lane LOS	B	A	A	-	A	-	-	B
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0.1

**Intersection**

Int Delay, s/veh 5.3

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	B	B			
Traffic Vol, veh/h	11	15	5	3	6	10
Future Vol, veh/h	11	15	5	3	6	10
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	12	16	5	3	6	11

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	30	7	0	0	8
Stage 1	7	-	-	-	-
Stage 2	23	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1
Critical Hdwy Stg 1	5.4	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2
Pot Cap-1 Maneuver	989	1081	-	-	1625
Stage 1	1021	-	-	-	-
Stage 2	1005	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	985	1081	-	-	1625
Mov Cap-2 Maneuver	985	-	-	-	-
Stage 1	1021	-	-	-	-
Stage 2	1001	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	8.6	0	2.7
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	1038	1625	-
HCM Lane V/C Ratio	-	-	0.026	0.004	-
HCM Control Delay (s)	-	-	8.6	7.2	0
HCM Lane LOS	-	-	A	A	A
HCM 95th %tile Q(veh)	-	-	0.1	0	-

## Lanes, Volumes, Timings

### 1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑↑			↑↑
Traffic Volume (vph)	210	154	1363	48	0	1498
Future Volume (vph)	210	154	1363	48	0	1498
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	2000
Storage Length (ft)	110	0		0	0	
Storage Lanes	1	1		0	0	
Taper Length (ft)	145			25		
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Ped Bike Factor	0.92	0.98	1.00			
Fr <sub>t</sub>		0.850	0.995			
Flt Protected	0.950					
Satd. Flow (prot)	1787	1318	3482	0	0	3623
Flt Permitted	0.950					
Satd. Flow (perm)	1648	1290	3482	0	0	3623
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	30		30		30	
Link Distance (ft)	465		176		249	
Travel Time (s)	10.6		4.0		5.7	
Confl. Peds. (#/hr)	52	6		21	21	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	1%	6%	3%	0%	0%	3%
Bus Blockages (#/hr)	0	0	0	0	0	9
Parking (#/hr)		7				
Shared Lane Traffic (%)						
Lane Group Flow (vph)	216	159	1454	0	0	1544
Turn Type	Prot	Perm	NA		NA	
Protected Phases	8		2		6	
Permitted Phases		8				
Detector Phase	8	8	2		6	
Switch Phase						
Minimum Initial (s)	8.0	8.0	15.0		15.0	
Minimum Split (s)	31.0	31.0	28.0		29.0	
Total Split (s)	38.0	38.0	87.0		87.0	
Total Split (%)	30.4%	30.4%	69.6%		69.6%	
Yellow Time (s)	4.0	4.0	4.0		4.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0		6.0	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Min		C-Min	
Act Effct Green (s)	22.0	22.0	91.0		91.0	
Actuated g/C Ratio	0.18	0.18	0.73		0.73	
v/c Ratio	0.69	0.70	0.57		0.59	
Control Delay	59.1	64.2	9.7		9.9	
Queue Delay	0.0	0.0	0.0		0.0	
Total Delay	59.1	64.2	9.7		9.9	
LOS	E	E	A		A	

Lanes, Volumes, Timings  
1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Approach Delay	61.3		9.7			9.9
Approach LOS		E		A		A
Queue Length 50th (ft)	166	123	467		271	
Queue Length 95th (ft)	232	184	631		427	
Internal Link Dist (ft)	385		96		169	
Turn Bay Length (ft)	110					
Base Capacity (vph)	457	330	2536		2638	
Starvation Cap Reductn	0	0	0		0	
Spillback Cap Reductn	0	0	0		0	
Storage Cap Reductn	0	0	0		0	
Reduced v/c Ratio	0.47	0.48	0.57		0.59	

Intersection Summary

Area Type: Other

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 104 (83%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 65

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.70

Intersection Signal Delay: 15.5

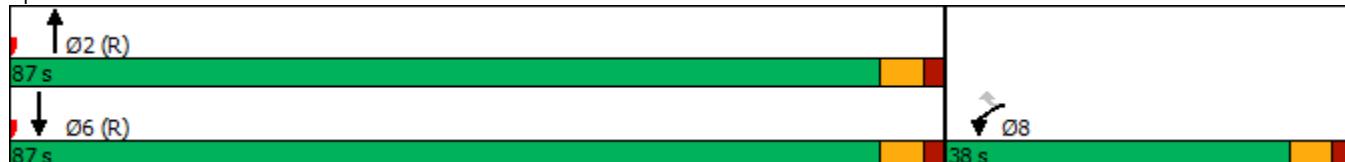
Intersection LOS: B

Intersection Capacity Utilization 62.6%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: Harlem Avenue & South Boulevard



## Lanes, Volumes, Timings

## 2: Harlem Avenue &amp; Randolph Street

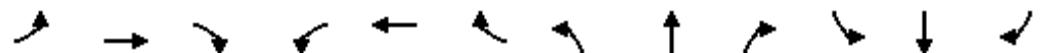
01/28/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑↑		↑	↑↑	
Traffic Volume (vph)	173	268	122	88	185	39	126	1216	69	86	1285	84
Future Volume (vph)	173	268	122	88	185	39	126	1216	69	86	1285	84
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	40		0	50		0	165		0	85		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	90			75			120			90		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	0.98	0.99		1.00	0.99			1.00			0.99	
Fr <sub>t</sub>		0.953			0.974			0.992			0.991	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1798	0	1805	1826	0	1703	3658	0	1805	3575	0
Flt Permitted	0.365			0.166			0.068			0.098		
Satd. Flow (perm)	681	1798	0	315	1826	0	122	3658	0	186	3575	0
Right Turn on Red			No			No			No		No	
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		454			257			242			813	
Travel Time (s)		10.3			5.8			5.5			18.5	
Confl. Peds. (#/hr)	20		6	6		20	35		7	7		35
Confl. Bikes (#/hr)			1			2						
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	0%	0%	0%	0%	0%	3%	6%	3%	0%	0%	3%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	9	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	180	406	0	92	234	0	131	1339	0	90	1427	0
Turn Type	pm+pt	NA										
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	3.0	8.0		3.0	8.0		3.0	15.0		3.0	15.0	
Minimum Split (s)	9.5	30.0		9.5	30.0		9.5	29.0		10.5	33.0	
Total Split (s)	14.0	30.0		14.0	30.0		14.0	67.0		14.0	67.0	
Total Split (%)	11.2%	24.0%		11.2%	24.0%		11.2%	53.6%		11.2%	53.6%	
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0		3.5	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	
Lead/Lag	Lead	Lag										
Lead-Lag Optimize?	Yes	Yes										
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effct Green (s)	38.1	25.2		35.9	24.1		75.4	63.5		72.6	62.1	
Actuated g/C Ratio	0.30	0.20		0.29	0.19		0.60	0.51		0.58	0.50	
v/c Ratio	0.60	1.12		0.46	0.66		0.68	0.72		0.43	0.80	
Control Delay	41.7	129.3		38.0	57.0		38.8	26.9		18.6	39.3	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	41.7	129.3		38.0	57.0		38.8	26.9		18.6	39.3	
LOS	D	F		D	E		D	C		B	D	

Lanes, Volumes, Timings  
2: Harlem Avenue & Randolph Street

01/28/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		102.4			51.6			28.0			38.1	
Approach LOS		F			D			C			D	
Queue Length 50th (ft)	109	~386		53	177		46	427		40	568	
Queue Length 95th (ft)	172	#594		95	268		#122	534		m65	660	
Internal Link Dist (ft)		374			177			162			733	
Turn Bay Length (ft)	40			50			165			85		
Base Capacity (vph)	302	363		218	352		207	1857		248	1775	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.60	1.12		0.42	0.66		0.63	0.72		0.36	0.80	

Intersection Summary

Area Type: Other

Cycle Length: 125

Actuated Cycle Length: 125

Offset: 56 (45%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.12

Intersection Signal Delay: 45.1

Intersection LOS: D

Intersection Capacity Utilization 86.7%

ICU Level of Service E

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

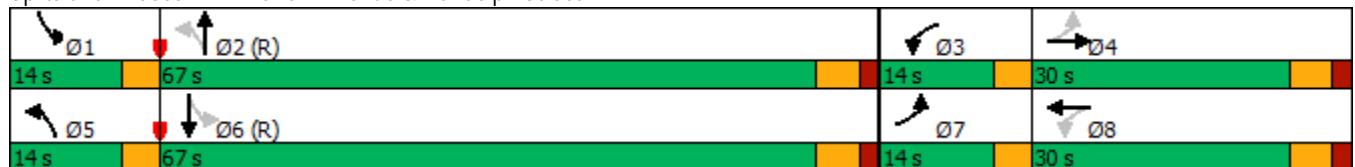
Queue shown is maximum after two cycles.

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Harlem Avenue & Randolph Street



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Intersection

Intersection Delay, s/veh 7.7

Intersection LOS A

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	5	94	6	31	35	9	12	5	26	3	4	8
Future Vol, veh/h	5	94	6	31	35	9	12	5	26	3	4	8
Peak Hour Factor	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77	0.77
Heavy Vehicles, %	0	0	0	0	0	0	0	0	12	0	0	12
Mvmt Flow	6	122	8	40	45	12	16	6	34	4	5	10
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	7.9			7.7			7.4			7.3		
HCM LOS	A			A			A			A		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	28%	5%	41%	20%
Vol Thru, %	12%	90%	47%	27%
Vol Right, %	60%	6%	12%	53%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	43	105	75	15
LT Vol	12	5	31	3
Through Vol	5	94	35	4
RT Vol	26	6	9	8
Lane Flow Rate	56	136	97	19
Geometry Grp	1	1	1	1
Degree of Util (X)	0.064	0.155	0.112	0.023
Departure Headway (Hd)	4.124	4.081	4.146	4.19
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	873	871	855	859
Service Time	2.125	2.142	2.215	2.192
HCM Lane V/C Ratio	0.064	0.156	0.113	0.022
HCM Control Delay	7.4	7.9	7.7	7.3
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	0.2	0.5	0.4	0.1

**Intersection**

Intersection Delay, s/veh 20.5

Intersection LOS C

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	83	305	48	10	232	18	27	87	8	37	156	66
Future Vol, veh/h	83	305	48	10	232	18	27	87	8	37	156	66
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	1	0
Mvmt Flow	92	339	53	11	258	20	30	97	9	41	173	73
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	28.1			15.6			12.6			16.2		
HCM LOS	D			C			B			C		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	22%	19%	4%	14%
Vol Thru, %	71%	70%	89%	60%
Vol Right, %	7%	11%	7%	25%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	122	436	260	259
LT Vol	27	83	10	37
Through Vol	87	305	232	156
RT Vol	8	48	18	66
Lane Flow Rate	136	484	289	288
Geometry Grp	1	1	1	1
Degree of Util (X)	0.264	0.795	0.503	0.516
Departure Headway (Hd)	7.016	5.908	6.272	6.45
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	510	610	572	558
Service Time	5.093	3.962	4.337	4.511
HCM Lane V/C Ratio	0.267	0.793	0.505	0.516
HCM Control Delay	12.6	28.1	15.6	16.2
HCM Lane LOS	B	D	C	C
HCM 95th-tile Q	1.1	7.8	2.8	2.9

**Intersection**

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	30	0	1411	1449	259
Future Vol, veh/h	0	30	0	1411	1449	259
Conflicting Peds, #/hr	6	52	52	0	0	52
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	0	0	3	3	1
Mvmt Flow	0	31	0	1455	1494	267

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	-	985	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	6.9	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	3.3	-
Pot Cap-1 Maneuver	0	251	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	227	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

**Approach**

EB NB SB

HCM Control Delay, s 23.3 0 0

HCM LOS C

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	227	-	-
HCM Lane V/C Ratio	-	0.136	-	-
HCM Control Delay (s)	-	23.3	-	-
HCM Lane LOS	-	C	-	-
HCM 95th %tile Q(veh)	-	0.5	-	-

**Intersection**

Int Delay, s/veh 0.5

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	48	1	17	358	4	4
Future Vol, veh/h	48	1	17	358	4	4
Conflicting Peds, #/hr	0	56	56	0	19	6
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	0	0	0	3	0	0
Mvmt Flow	55	1	20	411	5	5

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	112	0	582
Stage 1	-	-	-	-	112
Stage 2	-	-	-	-	470
Critical Hdwy	-	-	4.1	-	6.4
Critical Hdwy Stg 1	-	-	-	-	5.4
Critical Hdwy Stg 2	-	-	-	-	5.4
Follow-up Hdwy	-	-	2.2	-	3.5
Pot Cap-1 Maneuver	-	-	1490	-	479
Stage 1	-	-	-	-	918
Stage 2	-	-	-	-	633
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1411	-	437
Mov Cap-2 Maneuver	-	-	-	-	884
Stage 1	-	-	-	-	437
Stage 2	-	-	-	-	869

Approach	EB	WB	NB
HCM Control Delay, s	0	0.3	11.3
HCM LOS		B	

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	585	-	-	1411	-
HCM Lane V/C Ratio	0.016	-	-	0.014	-
HCM Control Delay (s)	11.3	-	-	7.6	0
HCM Lane LOS	B	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

## Intersection

Int Delay, s/veh 12.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	0	0	51	4	1	50	18	1361	49	56	1400	23
Future Vol, veh/h	0	0	51	4	1	50	18	1361	49	56	1400	23
Conflicting Peds, #/hr	2	0	1	1	0	2	24	0	22	22	0	24
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	0	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	96	96	96	96	96	96	96	96	96	96	96	96
Heavy Vehicles, %	0	0	0	0	0	3	6	3	0	0	3	0
Mvmt Flow	0	0	53	4	1	52	19	1418	51	58	1458	24

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	-	-	766	2350	3126	759	1506	0	0	1491	0	0
Stage 1	-	-	-	1504	1504	-	-	-	-	-	-	-
Stage 2	-	-	-	846	1622	-	-	-	-	-	-	-
Critical Hdwy	-	-	6.9	7.5	6.5	6.96	4.22	-	-	4.1	-	-
Critical Hdwy Stg 1	-	-	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	-	-	3.3	3.5	4	3.33	2.26	-	-	2.2	-	-
Pot Cap-1 Maneuver	0	0	350	20	11	347	421	-	-	456	-	-
Stage 1	0	0	-	130	186	-	-	-	-	-	-	-
Stage 2	0	0	-	328	163	-	-	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	342	5	2	339	411	-	-	446	-	-
Mov Cap-2 Maneuver	-	-	-	5	2	-	-	-	-	-	-	-
Stage 1	-	-	-	94	135	-	-	-	-	-	-	-
Stage 2	-	-	-	70	40	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	17.5	\$ 492			1.9			5.2		
HCM LOS	C	F								
<hr/>										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR		
Capacity (veh/h)	411	-	-	342	38	446	-	-		
HCM Lane V/C Ratio	0.046	-	-	0.155	1.508	0.131	-	-		
HCM Control Delay (s)	14.2	1.8	-	17.5	\$ 492	14.3	4.9	-		
HCM Lane LOS	B	A	-	C	F	B	A	-		
HCM 95th %tile Q(veh)	0.1	-	-	0.5	6	0.4	-	-		

## Notes

~: Volume exceeds capacity    \$: Delay exceeds 300s    +: Computation Not Defined    \*: All major volume in platoon

## Intersection

Int Delay, s/veh 2.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	4	107	12	17	61	1	7	3	12	5	6	7
Future Vol, veh/h	4	107	12	17	61	1	7	3	12	5	6	7
Conflicting Peds, #/hr	3	0	23	23	0	3	8	0	0	0	0	8
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	81	81	81	81	81	81	81	81	81	81	81	81
Heavy Vehicles, %	0	3	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	5	132	15	21	75	1	9	4	15	6	7	9

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	79	0	0	170	0	0	307	294	163	280	301	87
Stage 1	-	-	-	-	-	-	173	173	-	121	121	-
Stage 2	-	-	-	-	-	-	134	121	-	159	180	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1532	-	-	1420	-	-	649	620	887	676	615	977
Stage 1	-	-	-	-	-	-	834	760	-	888	800	-
Stage 2	-	-	-	-	-	-	874	800	-	848	754	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1528	-	-	1389	-	-	609	593	868	650	588	967
Mov Cap-2 Maneuver	-	-	-	-	-	-	609	593	-	650	588	-
Stage 1	-	-	-	-	-	-	812	740	-	882	785	-
Stage 2	-	-	-	-	-	-	838	785	-	826	734	-

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.2	1.6		10.2		10.2		
HCM LOS				B		B		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	724	1528	-	-	1389	-	-	716
HCM Lane V/C Ratio	0.038	0.003	-	-	0.015	-	-	0.031
HCM Control Delay (s)	10.2	7.4	0	-	7.6	0	-	10.2
HCM Lane LOS	B	A	A	-	A	A	-	B
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1

Intersection

Int Delay, s/veh 8.9

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	73	12	39	11	34	25	19	159	10	17	209	26
Future Vol, veh/h	73	12	39	11	34	25	19	159	10	17	209	26
Conflicting Peds, #/hr	0	0	52	19	0	0	118	0	99	99	0	118
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	80	80	80	80	80	80	80	80	80	80	80	80
Heavy Vehicles, %	0	0	8	0	0	0	0	0	0	0	0	0
Mvmt Flow	91	15	49	14	43	31	24	199	13	21	261	33

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	729	797	448	757	807	305	412	0	0	311	0	0
Stage 1	438	438	-	353	353	-	-	-	-	-	-	-
Stage 2	291	359	-	404	454	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.28	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.372	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	341	322	598	327	317	740	1158	-	-	1261	-	-
Stage 1	601	582	-	668	634	-	-	-	-	-	-	-
Stage 2	721	631	-	627	573	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	241	247	505	234	243	670	1028	-	-	1142	-	-
Mov Cap-2 Maneuver	241	247	-	234	243	-	-	-	-	-	-	-
Stage 1	520	505	-	589	559	-	-	-	-	-	-	-
Stage 2	619	557	-	511	497	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	31	21			0.9			0.6				
HCM LOS	D	C										
<hr/>												
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR				
Capacity (veh/h)	1028	-	-	289	312	1142	-	-				
HCM Lane V/C Ratio	0.023	-	-	0.536	0.28	0.019	-	-				
HCM Control Delay (s)	8.6	0	-	31	21	8.2	0	-				
HCM Lane LOS	A	A	-	D	C	A	A	-				
HCM 95th %tile Q(veh)	0.1	-	-	3	1.1	0.1	-	-				

## Intersection

Int Delay, s/veh 2.6

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	13	400	10	25	283	25	11	5	30	14	9	18
Future Vol, veh/h	13	400	10	25	283	25	11	5	30	14	9	18
Conflicting Peds, #/hr	22	0	49	49	0	22	17	0	7	7	0	17
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	79	79	79	79	79	79	79	79	79	79	79	79
Heavy Vehicles, %	5	0	0	12	1	0	0	0	0	0	1	10
Mvmt Flow	16	506	13	32	358	32	14	6	38	18	11	23

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	412	0	0	568	0	0	1066	1070	569	1034	1060	413
Stage 1	-	-	-	-	-	-	594	594	-	460	460	-
Stage 2	-	-	-	-	-	-	472	476	-	574	600	-
Critical Hdwy	4.15	-	-	4.22	-	-	7.1	6.5	6.2	7.1	6.51	6.3
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.51	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.51	-
Follow-up Hdwy	2.245	-	-	2.308	-	-	3.5	4	3.3	3.5	4.009	3.39
Pot Cap-1 Maneuver	1131	-	-	956	-	-	202	223	525	212	225	622
Stage 1	-	-	-	-	-	-	495	496	-	585	568	-
Stage 2	-	-	-	-	-	-	576	560	-	507	491	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1107	-	-	911	-	-	166	195	497	177	196	599
Mov Cap-2 Maneuver	-	-	-	-	-	-	166	195	-	177	196	-
Stage 1	-	-	-	-	-	-	462	463	-	561	531	-
Stage 2	-	-	-	-	-	-	509	524	-	450	459	-

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.3	0.7		19.7		21.9		
HCM LOS				C		C		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	302	1107	-	-	911	-	-	264
HCM Lane V/C Ratio	0.193	0.015	-	-	0.035	-	-	0.197
HCM Control Delay (s)	19.7	8.3	0	-	9.1	0	-	21.9
HCM Lane LOS	C	A	A	-	A	A	-	C
HCM 95th %tile Q(veh)	0.7	0	-	-	0.1	-	-	0.7

## Intersection

Int Delay, s/veh 0.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	9	426	9	6	312	7	6	2	3	7	0	15
Future Vol, veh/h	9	426	9	6	312	7	6	2	3	7	0	15
Conflicting Peds, #/hr	8	0	10	10	0	8	1	0	5	5	0	1
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	0	1	0	0	0	0	0	0	0
Mvmt Flow	10	473	10	7	347	8	7	2	3	8	0	17

Major/Minor	Major1	Major2			Minor1			Minor2				
Conflicting Flow All	363	0	0	493	0	0	883	885	493	879	886	360
Stage 1	-	-	-	-	-	-	508	508	-	373	373	-
Stage 2	-	-	-	-	-	-	375	377	-	506	513	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1207	-	-	1081	-	-	269	286	580	270	286	689
Stage 1	-	-	-	-	-	-	551	542	-	652	622	-
Stage 2	-	-	-	-	-	-	650	619	-	552	539	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1198	-	-	1071	-	-	256	276	572	260	276	683
Mov Cap-2 Maneuver	-	-	-	-	-	-	256	276	-	260	276	-
Stage 1	-	-	-	-	-	-	540	531	-	640	612	-
Stage 2	-	-	-	-	-	-	628	609	-	538	528	-

Approach	EB	WB			NB		SB				
HCM Control Delay, s	0.2	0.2			17.3		13.5				
HCM LOS					C		B				
<hr/>											
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)	306	1198	-	-	1071	-	-	450			
HCM Lane V/C Ratio	0.04	0.008	-	-	0.006	-	-	0.054			
HCM Control Delay (s)	17.3	8	0	-	8.4	0	-	13.5			
HCM Lane LOS	C	A	A	-	A	A	-	B			
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.2			

**Intersection**

Int Delay, s/veh 4.2

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	B	B			
Traffic Vol, veh/h	7	10	12	6	20	15
Future Vol, veh/h	7	10	12	6	20	15
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	7	11	13	6	21	16

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	74	16	0	0	19
Stage 1	16	-	-	-	-
Stage 2	58	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1
Critical Hdwy Stg 1	5.4	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2
Pot Cap-1 Maneuver	935	1069	-	-	1611
Stage 1	1012	-	-	-	-
Stage 2	970	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	923	1069	-	-	1611
Mov Cap-2 Maneuver	923	-	-	-	-
Stage 1	1012	-	-	-	-
Stage 2	957	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	8.7	0	4.2
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	1004	1611	-
HCM Lane V/C Ratio	-	-	0.018	0.013	-
HCM Control Delay (s)	-	-	8.7	7.3	0
HCM Lane LOS	-	-	A	A	A
HCM 95th %tile Q(veh)	-	-	0.1	0	-

## Lanes, Volumes, Timings

## 1: Harlem Avenue &amp; South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↑	↑	↑↑			↑↑
Traffic Volume (vph)	143	153	1189	52	0	1285
Future Volume (vph)	143	153	1189	52	0	1285
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	2000
Storage Length (ft)	110	0		0	0	
Storage Lanes	1	1		0	0	
Taper Length (ft)	145			25		
Lane Util. Factor	1.00	1.00	0.95	0.95	1.00	0.95
Ped Bike Factor	0.88	0.98	0.99			
Fr <sub>t</sub>		0.850	0.994			
Flt Protected	0.950					
Satd. Flow (prot)	1752	1356	3501	0	0	3695
Flt Permitted	0.950					
Satd. Flow (perm)	1551	1327	3501	0	0	3695
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	30		30		30	
Link Distance (ft)	465		176		249	
Travel Time (s)	10.6		4.0		5.7	
Confl. Peds. (#/hr)	80	7		42	42	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Heavy Vehicles (%)	3%	3%	2%	0%	0%	1%
Bus Blockages (#/hr)	0	0	0	0	0	9
Parking (#/hr)		7				
Shared Lane Traffic (%)						
Lane Group Flow (vph)	147	158	1280	0	0	1325
Turn Type	Prot	Perm	NA		NA	
Protected Phases	8		2		6	
Permitted Phases		8				
Detector Phase	8	8	2		6	
Switch Phase						
Minimum Initial (s)	8.0	8.0	15.0		15.0	
Minimum Split (s)	31.0	31.0	28.0		29.0	
Total Split (s)	36.0	36.0	84.0		84.0	
Total Split (%)	30.0%	30.0%	70.0%		70.0%	
Yellow Time (s)	4.0	4.0	4.0		4.0	
All-Red Time (s)	2.0	2.0	2.0		2.0	
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	
Total Lost Time (s)	6.0	6.0	6.0		6.0	
Lead/Lag						
Lead-Lag Optimize?						
Recall Mode	None	None	C-Min		C-Min	
Act Effct Green (s)	20.4	20.4	87.6		87.6	
Actuated g/C Ratio	0.17	0.17	0.73		0.73	
v/c Ratio	0.49	0.70	0.50		0.49	
Control Delay	49.7	62.7	9.2		8.2	
Queue Delay	0.0	0.0	0.0		0.0	
Total Delay	49.7	62.7	9.2		8.2	
LOS	D	E	A		A	

Lanes, Volumes, Timings  
1: Harlem Avenue & South Boulevard

01/28/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Approach Delay	56.4		9.2			8.2
Approach LOS		E		A		A
Queue Length 50th (ft)	104	117	358			200
Queue Length 95th (ft)	158	179	486			311
Internal Link Dist (ft)	385		96			169
Turn Bay Length (ft)	110					
Base Capacity (vph)	438	331	2555			2697
Starvation Cap Reductn	0	0	0			0
Spillback Cap Reductn	0	0	0			0
Storage Cap Reductn	0	0	0			0
Reduced v/c Ratio	0.34	0.48	0.50			0.49

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 101 (84%), Referenced to phase 2:NBT and 6:SBT, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.70

Intersection Signal Delay: 13.7

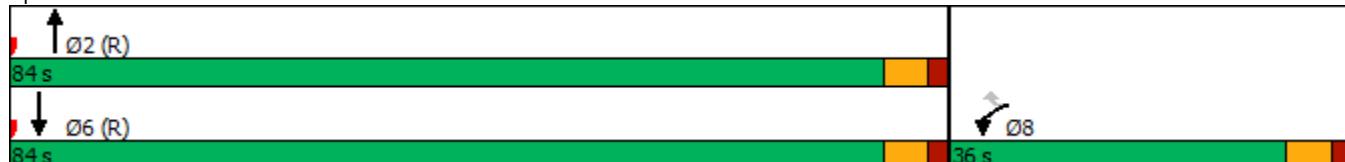
Intersection LOS: B

Intersection Capacity Utilization 57.1%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: Harlem Avenue & South Boulevard



## Lanes, Volumes, Timings

## 2: Harlem Avenue &amp; Randolph Street

01/28/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑		↑	↑↑		↑	↑↑	
Traffic Volume (vph)	155	160	74	78	140	34	86	1104	46	68	1090	89
Future Volume (vph)	155	160	74	78	140	34	86	1104	46	68	1090	89
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	40		0	50		0	165		0	85		0
Storage Lanes	1		0	1		0	1		0	1		0
Taper Length (ft)	90			75			120			90		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	0.95
Ped Bike Factor	0.99	0.99		0.99	0.99			1.00			1.00	
Fr <sub>t</sub>		0.952			0.970			0.994			0.989	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1787	1771	0	1805	1831	0	1805	3701	0	1805	3645	0
Flt Permitted	0.436			0.399			0.127			0.148		
Satd. Flow (perm)	808	1771	0	752	1831	0	241	3701	0	281	3645	0
Right Turn on Red			No			No			No		No	
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		454			257			242			813	
Travel Time (s)		10.3			5.8			5.5			18.5	
Confl. Peds. (#/hr)	14		10	10		14	10		6	6		10
Confl. Bikes (#/hr)					1							
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	1%	0%	4%	0%	0%	0%	0%	2%	0%	0%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	9	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	163	246	0	82	183	0	91	1210	0	72	1241	0
Turn Type	pm+pt	NA										
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	7	4		3	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	3.0	8.0		3.0	8.0		3.0	15.0		3.0	15.0	
Minimum Split (s)	9.5	30.0		9.5	30.0		9.5	29.0		10.5	33.0	
Total Split (s)	13.0	30.0		13.0	30.0		13.0	64.0		13.0	64.0	
Total Split (%)	10.8%	25.0%		10.8%	25.0%		10.8%	53.3%		10.8%	53.3%	
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0		3.5	4.0	
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	
Lead/Lag	Lead	Lag										
Lead-Lag Optimize?	Yes	Yes										
Recall Mode	None	None		None	None		None	C-Min		None	C-Min	
Act Effct Green (s)	33.2	23.1		31.3	20.2		74.9	65.8		73.4	63.5	
Actuated g/C Ratio	0.28	0.19		0.26	0.17		0.62	0.55		0.61	0.53	
v/c Ratio	0.54	0.72		0.30	0.59		0.36	0.60		0.27	0.64	
Control Delay	39.3	58.7		32.7	53.6		13.1	21.3		15.3	31.5	
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	39.3	58.7		32.7	53.6		13.1	21.3		15.3	31.5	
LOS	D	E		C	D		B	C		B	C	

Lanes, Volumes, Timings  
2: Harlem Avenue & Randolph Street

01/28/2021



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Approach Delay		51.0			47.1			20.7			30.6	
Approach LOS			D			D			C			C
Queue Length 50th (ft)	95	181		45	129		26	343		30	452	
Queue Length 95th (ft)	150	271		83	202		49	442		60	551	
Internal Link Dist (ft)		374			177			162			733	
Turn Bay Length (ft)		40			50			165			85	
Base Capacity (vph)	300	357		284	366		276	2028		297	1929	
Starvation Cap Reductn	0	0		0	0		0	0		0	0	
Spillback Cap Reductn	0	0		0	0		0	0		0	0	
Storage Cap Reductn	0	0		0	0		0	0		0	0	
Reduced v/c Ratio	0.54	0.69		0.29	0.50		0.33	0.60		0.24	0.64	

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 56 (47%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.72

Intersection Signal Delay: 30.5

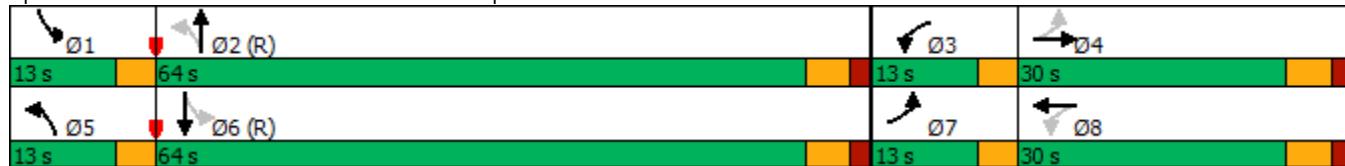
Intersection LOS: C

Intersection Capacity Utilization 75.0%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 2: Harlem Avenue & Randolph Street



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Intersection

Intersection Delay, s/veh 7.8

Intersection LOS A

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Vol, veh/h	9	105	7	22	26	16	11	5	26	6	6	8
Future Vol, veh/h	9	105	7	22	26	16	11	5	26	6	6	8
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Heavy Vehicles, %	0	2	0	0	7	17	0	0	0	17	17	0
Mvmt Flow	11	133	9	28	33	20	14	6	33	8	8	10
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	8			7.6			7.4			7.7		
HCM LOS	A			A			A			A		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	26%	7%	34%	30%
Vol Thru, %	12%	87%	41%	30%
Vol Right, %	62%	6%	25%	40%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	42	121	64	20
LT Vol	11	9	22	6
Through Vol	5	105	26	6
RT Vol	26	7	16	8
Lane Flow Rate	53	153	81	25
Geometry Grp	1	1	1	1
Degree of Util (X)	0.061	0.174	0.092	0.032
Departure Headway (Hd)	4.12	4.079	4.072	4.577
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	875	872	868	787
Service Time	2.121	2.142	2.152	2.579
HCM Lane V/C Ratio	0.061	0.175	0.093	0.032
HCM Control Delay	7.4	8	7.6	7.7
HCM Lane LOS	A	A	A	A
HCM 95th-tile Q	0.2	0.6	0.3	0.1

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Intersection

Intersection Delay, s/veh 12.1

Intersection LOS B

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↖			↖			↖			↖	
Traffic Vol, veh/h	68	198	26	8	199	7	12	82	13	45	111	52
Future Vol, veh/h	68	198	26	8	199	7	12	82	13	45	111	52
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	3	0	0
Mvmt Flow	76	222	29	9	224	8	13	92	15	51	125	58
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	13.3			11.6			10.3			11.8		
HCM LOS	B			B			B			B		

Lane	NBLn1	EBLn1	WBLn1	SBLn1
Vol Left, %	11%	23%	4%	22%
Vol Thru, %	77%	68%	93%	53%
Vol Right, %	12%	9%	3%	25%
Sign Control	Stop	Stop	Stop	Stop
Traffic Vol by Lane	107	292	214	208
LT Vol	12	68	8	45
Through Vol	82	198	199	111
RT Vol	13	26	7	52
Lane Flow Rate	120	328	240	234
Geometry Grp	1	1	1	1
Degree of Util (X)	0.194	0.484	0.363	0.363
Departure Headway (Hd)	5.822	5.306	5.434	5.594
Convergence, Y/N	Yes	Yes	Yes	Yes
Cap	613	679	661	640
Service Time	3.885	3.351	3.484	3.648
HCM Lane V/C Ratio	0.196	0.483	0.363	0.366
HCM Control Delay	10.3	13.3	11.6	11.8
HCM Lane LOS	B	B	B	B
HCM 95th-tile Q	0.7	2.7	1.7	1.7

**Intersection**

Int Delay, s/veh 0.2

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	31	0	1241	1268	160
Future Vol, veh/h	0	31	0	1241	1268	160
Conflicting Peds, #/hr	7	80	58	0	0	58
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	0	0	0	2	1	0
Mvmt Flow	0	32	0	1279	1307	165

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	-	874	-
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	6.9	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	3.3	-
Pot Cap-1 Maneuver	0	297	0
Stage 1	0	-	0
Stage 2	0	-	0
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	259	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

**Approach**

EB NB SB

HCM Control Delay, s 20.8 0 0

HCM LOS C

Minor Lane/Major Mvmt	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	-	259	-	-
HCM Lane V/C Ratio	-	0.123	-	-
HCM Control Delay (s)	-	20.8	-	-
HCM Lane LOS	-	C	-	-
HCM 95th %tile Q(veh)	-	0.4	-	-

## Intersection

Int Delay, s/veh 0.4

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑		↓	↔		
Traffic Vol, veh/h	51	4	6	289	4	4
Future Vol, veh/h	51	4	6	289	4	4
Conflicting Peds, #/hr	0	102	102	0	13	2
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	88	88	88	88	88	88
Heavy Vehicles, %	0	0	0	3	0	0
Mvmt Flow	58	5	7	328	5	5

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	165	0	518 165
Stage 1	-	-	-	-	163 -
Stage 2	-	-	-	-	355 -
Critical Hdwy	-	-	4.1	-	6.4 6.2
Critical Hdwy Stg 1	-	-	-	-	5.4 -
Critical Hdwy Stg 2	-	-	-	-	5.4 -
Follow-up Hdwy	-	-	2.2	-	3.5 3.3
Pot Cap-1 Maneuver	-	-	1426	-	521 885
Stage 1	-	-	-	-	871 -
Stage 2	-	-	-	-	714 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1287	-	462 798
Mov Cap-2 Maneuver	-	-	-	-	462 -
Stage 1	-	-	-	-	787 -
Stage 2	-	-	-	-	700 -

Approach	EB	WB	NB
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HCM Control Delay, s 0 0.2 11.3

HCM LOS B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	585	-	-	1287	-
HCM Lane V/C Ratio	0.016	-	-	0.005	-
HCM Control Delay (s)	11.3	-	-	7.8	0
HCM Lane LOS	B	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

## Intersection

Int Delay, s/veh 5.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	0	0	37	1	5	39	19	1202	72	49	1209	41
Future Vol, veh/h	0	0	37	1	5	39	19	1202	72	49	1209	41
Conflicting Peds, #/hr	0	0	0	0	0	0	4	0	10	10	0	4
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	0	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	5	0	1	2	0	1	0
Mvmt Flow	0	0	40	1	5	42	21	1307	78	53	1314	45

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	-	-	684	2161	2867	703	1363	0	0	1395	0	0
Stage 1	-	-	-	1398	1398	-	-	-	-	-	-	-
Stage 2	-	-	-	763	1469	-	-	-	-	-	-	-
Critical Hdwy	-	-	6.9	7.5	6.5	7	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	-	-	-	6.5	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	-	-	3.3	3.5	4	3.35	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	0	0	396	27	17	373	511	-	-	497	-	-
Stage 1	0	0	-	151	209	-	-	-	-	-	-	-
Stage 2	0	0	-	367	194	-	-	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	394	14	8	369	509	-	-	492	-	-
Mov Cap-2 Maneuver	-	-	-	14	8	-	-	-	-	-	-	-
Stage 1	-	-	-	122	168	-	-	-	-	-	-	-
Stage 2	-	-	-	182	107	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	15.2	202.3			1.2			3		
HCM LOS	C	F								
<b>Minor Lane/Major Mvmt</b>										
Capacity (veh/h)	509	-	-	394	56	492	-	-	-	-
HCM Lane V/C Ratio	0.041	-	-	0.102	0.873	0.108	-	-	-	-
HCM Control Delay (s)	12.4	1.1	-	15.2	202.3	13.2	2.7	-	-	-
HCM Lane LOS	B	A	-	C	F	B	A	-	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0.3	3.9	0.4	-	-	-	-

## Intersection

Int Delay, s/veh 1.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	5	122	10	11	50	3	9	0	8	2	3	5
Future Vol, veh/h	5	122	10	11	50	3	9	0	8	2	3	5
Conflicting Peds, #/hr	8	0	17	17	0	8	7	0	6	6	0	7
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	2	0	0	3	0	0	0	0	0	0	20
Mvmt Flow	5	133	11	12	54	3	10	0	9	2	3	5

Major/Minor	Major1	Major2		Minor1		Minor2						
Conflicting Flow All	65	0	0	161	0	0	257	255	162	247	259	71
Stage 1	-	-	-	-	-	-	166	166	-	88	88	-
Stage 2	-	-	-	-	-	-	91	89	-	159	171	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.4
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.48
Pot Cap-1 Maneuver	1550	-	-	1430	-	-	700	652	888	711	649	943
Stage 1	-	-	-	-	-	-	841	765	-	925	826	-
Stage 2	-	-	-	-	-	-	921	825	-	848	761	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1538	-	-	1407	-	-	671	628	869	688	625	930
Mov Cap-2 Maneuver	-	-	-	-	-	-	671	628	-	688	625	-
Stage 1	-	-	-	-	-	-	824	750	-	914	812	-
Stage 2	-	-	-	-	-	-	898	811	-	831	746	-

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.3	1.3		9.9		9.8		
HCM LOS				A		A		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	752	1538	-	-	1407	-	-	764
HCM Lane V/C Ratio	0.025	0.004	-	-	0.008	-	-	0.014
HCM Control Delay (s)	9.9	7.3	0	-	7.6	0	-	9.8
HCM Lane LOS	A	A	A	-	A	A	-	A
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0

Intersection												
Int Delay, s/veh	7.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	+	+	+	+	+	+	+	+	+	+	+	+
Traffic Vol, veh/h	79	21	32	18	24	22	11	138	8	16	158	29
Future Vol, veh/h	79	21	32	18	24	22	11	138	8	16	158	29
Conflicting Peds, #/hr	0	0	57	57	0	0	136	0	86	86	0	136
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	0	0	2	0	12	0	0	0	0	0	0	0
Mvmt Flow	89	24	36	20	27	25	12	155	9	18	178	33
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	577	641	388	588	653	246	347	0	0	250	0	0
Stage 1	367	367	-	270	270	-	-	-	-	-	-	-
Stage 2	210	274	-	318	383	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.22	7.1	6.62	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.62	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.62	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.318	3.5	4.108	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	431	395	660	423	374	798	1223	-	-	1327	-	-
Stage 1	657	626	-	740	668	-	-	-	-	-	-	-
Stage 2	797	687	-	698	595	-	-	-	-	-	-	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	329	307	543	316	290	733	1065	-	-	1218	-	-
Mov Cap-2 Maneuver	329	307	-	316	290	-	-	-	-	-	-	-
Stage 1	565	535	-	671	606	-	-	-	-	-	-	-
Stage 2	727	623	-	579	509	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	21.9		16.8		0.6		0.6					
HCM LOS	C		C		A		A					
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR				
Capacity (veh/h)	1065	-	-	359	377	1218	-	-				
HCM Lane V/C Ratio	0.012	-	-	0.413	0.191	0.015	-	-				
HCM Control Delay (s)	8.4	0	-	21.9	16.8	8	0	-				
HCM Lane LOS	A	A	-	C	C	A	A	-				
HCM 95th %tile Q(veh)	0	-	-	2	0.7	0	-	-				

## Intersection

Int Delay, s/veh 2.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	7	263	4	21	238	16	5	19	27	9	17	9
Future Vol, veh/h	7	263	4	21	238	16	5	19	27	9	17	9
Conflicting Peds, #/hr	10	0	6	6	0	10	8	0	13	13	0	8
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	87	87	87	87	87	87	87	87	87	87	87	87
Heavy Vehicles, %	0	18	0	0	1	0	0	0	0	0	1	0
Mvmt Flow	8	302	5	24	274	18	6	22	31	10	20	10

Major/Minor	Major1	Major2		Minor1		Minor2		
Conflicting Flow All	302	0	0	313	0	0	681	677
Stage 1	-	-	-	-	-	-	327	327
Stage 2	-	-	-	-	-	-	354	350
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.51
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4
Pot Cap-1 Maneuver	1270	-	-	1259	-	-	367	377
Stage 1	-	-	-	-	-	-	690	651
Stage 2	-	-	-	-	-	-	667	636
Platoon blocked, %	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1258	-	-	1252	-	-	334	360
Mov Cap-2 Maneuver	-	-	-	-	-	-	334	360
Stage 1	-	-	-	-	-	-	680	642
Stage 2	-	-	-	-	-	-	617	615

Approach	EB	WB		NB		SB		
HCM Control Delay, s	0.2	0.6		13.5		15.1		
HCM LOS				B		C		
<hr/>								
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	482	1258	-	-	1252	-	-	396
HCM Lane V/C Ratio	0.122	0.006	-	-	0.019	-	-	0.102
HCM Control Delay (s)	13.5	7.9	0	-	7.9	0	-	15.1
HCM Lane LOS	B	A	A	-	A	A	-	C
HCM 95th %tile Q(veh)	0.4	0	-	-	0.1	-	-	0.3

## Intersection

Int Delay, s/veh 0.5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
<b>Lane Configurations</b>												
Traffic Vol, veh/h	8	288	3	1	260	2	1	0	1	3	1	14
Future Vol, veh/h	8	288	3	1	260	2	1	0	1	3	1	14
Conflicting Peds, #/hr	23	0	12	12	0	23	5	0	1	1	0	5
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None									
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	0
Mvmt Flow	9	339	4	1	306	2	1	0	1	4	1	16

Major/Minor	Major1	Major2			Minor1			Minor2				
Conflicting Flow All	331	0	0	355	0	0	694	704	354	693	705	335
Stage 1	-	-	-	-	-	-	371	371	-	332	332	-
Stage 2	-	-	-	-	-	-	323	333	-	361	373	-
Critical Hdwy	4.1	-	-	4.1	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1240	-	-	1215	-	-	360	364	694	360	363	712
Stage 1	-	-	-	-	-	-	653	623	-	686	648	-
Stage 2	-	-	-	-	-	-	693	647	-	662	622	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1213	-	-	1201	-	-	342	349	685	348	348	693
Mov Cap-2 Maneuver	-	-	-	-	-	-	342	349	-	348	348	-
Stage 1	-	-	-	-	-	-	640	611	-	665	633	-
Stage 2	-	-	-	-	-	-	671	632	-	654	610	-

Approach	EB	WB			NB			SB			
HCM Control Delay, s	0.2	0			12.9			11.6			
HCM LOS					B			B			
<hr/>											
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBT	SBR	SBLn2
Capacity (veh/h)	456	1213	-	-	1201	-	-	568	-	-	-
HCM Lane V/C Ratio	0.005	0.008	-	-	0.001	-	-	0.037	-	-	-
HCM Control Delay (s)	12.9	8	0	-	8	0	-	11.6	-	-	-
HCM Lane LOS	B	A	A	-	A	A	-	B	-	-	-
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0.1	-	-	-

**Intersection**

Int Delay, s/veh 5.5

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W	B	B			
Traffic Vol, veh/h	10	13	4	6	16	8
Future Vol, veh/h	10	13	4	6	16	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	11	14	4	6	17	8

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	49	7	0	0	10
Stage 1	7	-	-	-	-
Stage 2	42	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1
Critical Hdwy Stg 1	5.4	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2
Pot Cap-1 Maneuver	965	1081	-	-	1623
Stage 1	1021	-	-	-	-
Stage 2	986	-	-	-	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	954	1081	-	-	1623
Mov Cap-2 Maneuver	954	-	-	-	-
Stage 1	1021	-	-	-	-
Stage 2	975	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	8.6	0	4.8
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	1022	1623	-
HCM Lane V/C Ratio	-	-	0.024	0.01	-
HCM Control Delay (s)	-	-	8.6	7.2	0
HCM Lane LOS	-	-	A	A	A
HCM 95th %tile Q(veh)	-	-	0.1	0	-

# Sight Distance Analyses

POSTED SPEED LIMIT = 25 MPH  
DESIGN SPEED = 30 MPH

IDOT BDE MANUAL - CHAPTER 31

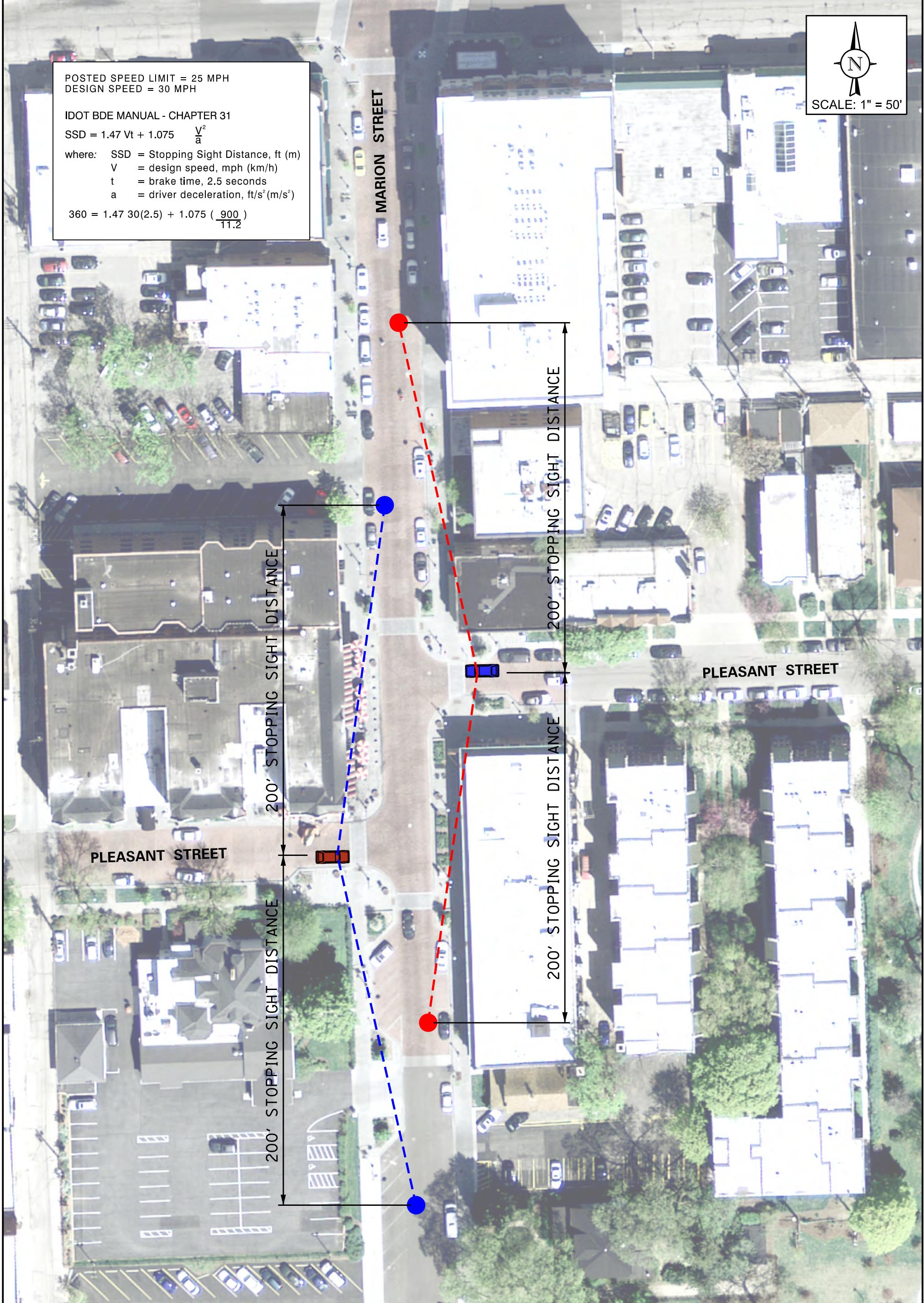
$$SSD = 1.47 Vt + 1.075 \frac{V^2}{a}$$

where: SSD = Stopping Sight Distance, ft (m)  
V = design speed, mph (km/h)  
t = brake time, 2.5 seconds  
a = driver deceleration, ft/s<sup>2</sup>(m/s<sup>2</sup>)

$$360 = 1.47 30(2.5) + 1.075 \frac{900}{11.2}$$



SCALE: 1" = 50'



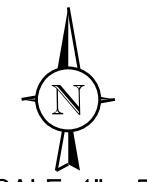
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IDOT BDE MANUAL - CHAPTER 31

$$SSD = 1.47 Vt + 1.075 \frac{V^2}{a}$$

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SCALE: 1" = 50'

